



RULES FOR BUILDING AND CLASSING

**STEEL VESSELS UNDER 90 METERS (295 FEET)
IN LENGTH
2006**

**PART 3
HULL CONSTRUCTION AND EQUIPMENT**

**American Bureau of Shipping
Incorporated by Act of Legislature of
the State of New York 1862**

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ABS Plaza
16855 Northchase Drive
Houston, TX 77060 USA**

Rule Change Notice (2006)

The effective date of each technical change since 1993 is shown in parenthesis at the end of the subsection/paragraph titles within the text of each Part. Unless a particular date and month are shown, the years in parentheses refer to the following effective dates:

(2000) and after	1 January 2000 (and subsequent years)	(1996)	9 May 1996
(1999)	12 May 1999	(1995)	15 May 1995
(1998)	13 May 1998	(1994)	9 May 1994
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Listing by Effective Dates of Changes from the 2001 Rules

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(based on the contract date for new construction between builder and Owner)

<i>Part/Para. No.</i>	<i>Title/Subject</i>	<i>Status/Remarks</i>
3-2-7/1.1	Openings and Penetrations	To specify the requirements for ventilation penetrations through watertight bulkheads
3-5-1/13.3.1	Operating Loads	For consistency with 4-5-1/5.1.1(a) of the <i>Rules for Building and Classing Steel Vessels</i> for windlass design with the cable stopper attached to the windlass structure.
3-7-1/Table 1	Testing Requirements for Tanks and Boundaries	To align the requirements with IACS UR S14 (Rev. 2) and to clarify that “top of hatch” means “top of hatch coaming” for the purpose of measuring hydrostatic testing head.

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CHAPTER **1 General**

SECTION **1 Definitions**

1 Application

The following definitions apply throughout these Rules.

3 Length

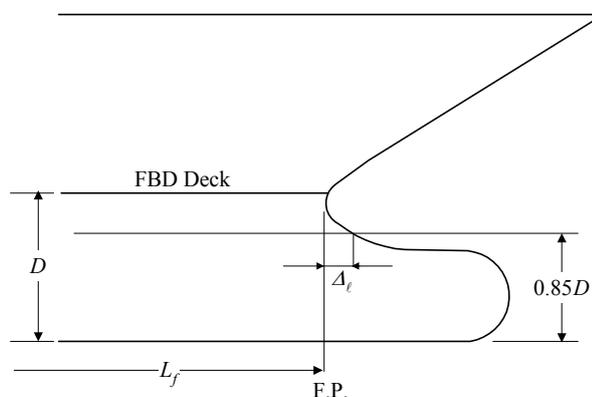
3.1 Scantling Length (L)

L is the distance in meters (feet) on the summer load line from the fore side of the stem to the centerline of the rudder stock. For use with the Rules, L is not to be less than 96% and need not be greater than 97% of the length on the summer load line. The forward end of L is to coincide with the fore side of the stem on the waterline on which L is measured.

3.3 Freeboard Length (L_f)

L_f is the distance in meters (feet) on a waterline at 85% of the least molded depth measured from the top of the keel from the fore side of the stem to the centerline of the rudder stock or 96% of the length on that waterline, whichever is greater. Where the stem is a fair concave curve above the waterline at 85% of the least molded depth and where the aftmost point of the stem is above the waterline, the forward end of the length, L_f , is to be taken at the aftmost point of the stem above that waterline. See 3-1-1/Figure 1.

FIGURE 1



5 Breadth (B)

B is the greatest molded breadth in meters (feet).

7 Depth

7.1 Molded Depth (D)

D is the molded depth at side in meters (feet) measured at the middle of L from the molded base line to the top of the freeboard-deck beams. In vessels having rounded gunwales, D is to be measured to the point of intersection of the molded lines of the deck and side shell plating. In cases where watertight bulkheads extend to a deck above the freeboard deck and are to be recorded in the *Record* as effective to that deck, D is to be measured to the bulkhead deck.

7.3 Scantling Depth (D_s)

The depth, D_s , for use with scantling requirements is measured to the strength deck, as defined in 3-1-1/13.5.

9 Draft for Scantlings (d)

d is the draft in meters (feet) measured at the middle of the length, L , from the molded keel or the rabbet line at its lowest point to the estimated summer load waterline, the design load waterline or $0.66D$, whichever is greater.

11 Molded Displacement and Block Coefficient

11.1 Molded Displacement (Δ)

Δ is the molded displacement of the vessel in metric tons (long tons), excluding appendages, taken at the summer load line.

11.3 Block Coefficient (C_b)

C_b is the block coefficient obtained from the following equation:

$$C_b = \Delta / 1.025LB_{wl}d \quad (\text{SI \& MKS units})$$

$$C_b = 35\Delta / LB_{wl}d \quad (\text{US units})$$

where

Δ = molded displacement, as defined in 3-1-1/11.1

L = scantling length, as defined in 3-1-1/3.1

d = draft, as defined in 3-1-1/9

B_{wl} = the greatest molded breadth at summer load line

13 Decks

13.1 Freeboard Deck

The freeboard deck is normally the uppermost continuous deck having permanent means for closing all openings in its weather portions, and below which all openings in the vessel's side are equipped with permanent means for watertight closure. In cases where a vessel is designed for a special draft considerably less than that corresponding to the least freeboard obtainable under the International Load Line Regulations, the freeboard deck for the purpose of the Rules may be taken as the lowest actual deck from which the draft can be obtained under those regulations.

13.3 Bulkhead Deck

The bulkhead deck is the highest deck to which watertight bulkheads extend and are made effective.

13.5 Strength Deck

The strength deck is the deck which forms the top of the effective hull girder at any part of its length. See Section 3-2-1.

13.7 Superstructure Deck

A superstructure deck is a deck above the freeboard deck to which the side shell plating extends. Except where otherwise specified, the term superstructure deck, where used in the Rules, refers to the first such deck above the freeboard deck.

13.9 Deckhouses

A deckhouse is an enclosed structure above the freeboard deck having side plating set inboard of the hull's side-shell plating more than 4% of the breadth, B , of the vessel.

15 Deadweight and Lightship Weight

For the purpose of these Rules, the deadweight, DWT , is the difference in metric tons (long tons) between the displacement of the vessel in water having a specific gravity of 1.025 at the summer load line and the lightship weight. For the purpose of these Rules, lightship weight is the displacement of a vessel in metric tons (long tons) without cargo, fuel, lubricating oil, ballast water, fresh water and feed water in tanks, consumable stores, and passengers and crews and their effects.

17 Gross Tonnage

17.1 International Tonnage

For the purpose of application of these Rules to vessels intended for unrestricted service (see 1-1-3/1 of the *ABS Rules for Conditions of Classification (Part 1)*), the referenced gross tonnage throughout the Rules is the measure of the internal volume of spaces within the vessel as determined in accordance with the provisions of the "International Convention on Tonnage Measurement of Ships, 1969".

17.3 National Tonnage

As an alternative to 3-1-1/17.1 above, requirements applicable on the basis of National Tonnage measurement and National Regulations will be considered for vessels whose operation is intended to be restricted exclusively to domestic service. (See 1-1-3/7 of the *ABS Rules for Conditions of Classification (Part 1)*).

19 Units

These Rules are written in three systems of units, i.e., SI units, MKS units and US customary units. Each system is to be used independently of any other system.

Unless indicated otherwise, the format of presentation in the Rules of the three systems of units is as follows:

SI units (MKS units, US customary units).

PART

3

CHAPTER 1 General

SECTION 2 General Requirements

1 Materials

1.1 General

These Rules are intended for vessels of welded construction using steels complying with the requirements in Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)*.

1.1.1 Steel

Use of steels other than Grade A and AH in Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)* and plate over 20 mm (0.79 in.) in important locations will be specially considered.

1.1.2 Aluminum Alloys

The use of aluminum alloys in hull structures will be considered upon submission of proposed specification for the alloy and the method of fabrication.

1.1.3 Design Consideration

Where scantlings are reduced in connection with the use of higher-strength steel or where aluminum alloys are used, adequate buckling strength is to be provided. Where it is intended to use material of cold flanging quality for important longitudinal strength members, this steel is to be indicated on the plans.

1.1.4 Guidance for Repair

Where special welding procedures are required for the special steels used in the construction, including any low temperature steel and those materials not in Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)*, a set of plans showing the following information for each steel should be placed aboard the vessel:

- Material Specification
- Welding Procedure
- Location and extent of application

These plans are in addition to those normally placed aboard which are to show all material applications.

1.3 Selection of Material Grade

For vessels 61 m (200 ft) and over in length, steel materials are not to be lower grades than those required by 3-1-2/Table 1 for the material class given in 3-1-2/Table 2 for the particular location.

1.5 Note for the Users

The attention of users is drawn to the fact that when fatigue loading is present, the effective strength of higher-strength steel in a welded construction may not be greater than that of ordinary-strength steel. Precautions against corrosion fatigue may also be necessary.

TABLE 1
Material Grades (2000)

Thickness <i>t</i> mm (in.)	Material Class	
	I	II
$t \leq 15$ ($t \leq 0.60$)	A ⁽¹⁾ , AH	A, AH
$15 < t \leq 20$ ($0.60 < t \leq 0.79$)	A, AH	A, AH
$20 < t \leq 25$ ($0.79 < t \leq 0.98$)	A, AH	B, AH
$25 < t \leq 30$ ($0.98 < t \leq 1.18$)	A, AH	D, DH
$30 < t \leq 35$ ($1.18 < t \leq 1.38$)	B, AH	D, DH
$35 < t \leq 40$ ($1.38 < t \leq 1.57$)	B, AH	D, DH
$40 < t \leq 51$ ($1.57 < t \leq 2.00$)	D, DH	E, EH

Note:

- 1 ASTM A36 steel otherwise tested and certified to the ABS satisfaction may be used in lieu of Grade A for a thickness up to and including 12.5 mm (0.5 in.) for plate and 15 mm (0.6 in.) for sections.

TABLE 2
Material Class of Structural Members (2000)

Structural Member	Material Class ⁽¹⁾	
	Within 0.4L Amidships	Outside 0.4L Amidships
Shell		
Bottom plating including keel plate	II	A ⁽⁴⁾ /AH
Bilge strake	II	A ⁽⁴⁾ /AH
Side plating	I	A ⁽⁴⁾ /AH
Sheer strake at strength deck ⁽²⁾	II	A ⁽⁴⁾ /AH
Decks		
Strength deck plating ⁽³⁾	II	A ⁽⁴⁾ /AH
Stringer plate in strength deck ⁽²⁾	II	A ⁽⁴⁾ /AH
Strength deck plating within line of hatches and exposed to weather, in general	I	A ⁽⁴⁾ /AH
Strength deck strake on tankers at longitudinal bulkhead	II	A ⁽⁴⁾ /AH
Longitudinal Bulkheads		
Lowest strake in single bottom vessels	I	A ⁽⁴⁾ /AH
Uppermost strake including that of the top wing tank	II	A ⁽⁴⁾ /AH
Other Structures in General		
External continuous longitudinal members and bilge keels	II	A ⁽⁴⁾ /AH
Stem frames, rudder horns, rudders, and shaft brackets	—	I
Strength members not referred to in above categories and above local structures	A ⁽⁴⁾ /AH	A ⁽⁴⁾ /AH

Notes:

- 1 Special consideration will be given to vessels in restricted service.
- 2 A radius gunwale plate may be considered to meet the requirements for both the stringer plate and the sheer strake, provided it extends suitable distances inboard and vertically. For formed material, see 2-4-1/3.13.
- 3 Plating at the corners of large hatch openings are to be specially considered.
- 4 ASTM A36 steel otherwise tested and certified to the ABS satisfaction may be used in lieu of Grade A for a thickness up to and including 12.5 mm (0.5 in.) for plate and 40 mm (1.57 in.) for sections.

3 Workmanship

All workmanship is to be of commercial marine quality and acceptable to the Surveyor. Welding is to be in accordance with the requirements of Chapter 4 of the *Rules for Materials and Welding (Part 2)* and Section 3-2-16 of these Rules. Plates which have been subjected to excessive furnacing are to undergo a satisfactory heat treatment before being worked into a hull.

5 Design

5.1 Continuity

Care is to be taken to provide structural continuity. Changes in scantlings are to be gradual. Strength members are not to change direction abruptly. Where major longitudinal members end at transverse structural members, tapering may be required forward or aft of the transverses. Stanchions and bulkheads are to be aligned to provide support and to minimize eccentric loading. Major appendages outside the hull and strength bulkheads in superstructures are to be aligned with major structural members within the hull.

5.3 Openings

In general, major openings such as doors, hatches, and large vent ducts are to be avoided in the sheer strake and stringer plate within the amidships three-fifths length. Corners of openings in strength structures are to have generous radii. Compensation may be required for openings.

5.5 Brackets

Where brackets are fitted having thicknesses as required by 3-1-2/Table 3 and faces at approximately 45 degrees with the bulkhead deck or shell and the bracket is supported by a bulkhead, deck or shell structural member, the length of each member, ℓ , may be measured at a point 25% of the extent of the bracket beyond the toe of the bracket, as shown in 3-1-2/Figure 1, **when a reduction of the span is so permitted in each section**. The minimum overlap of the bracket arm along the stiffener is not to be less than obtained from the following equation:

$$x = 1.4y + 30 \text{ mm}$$

$$x = 1.4y + 1.2 \text{ in.}$$

where

$$x = \text{length of overlap along stiffener, in mm (in.)}$$

$$y = \text{depth of stiffener, in mm (in.)}$$

Where a bracket laps a member, the amount of overlap generally is to be 25.5 mm (1 in.).

TABLE 3
Brackets

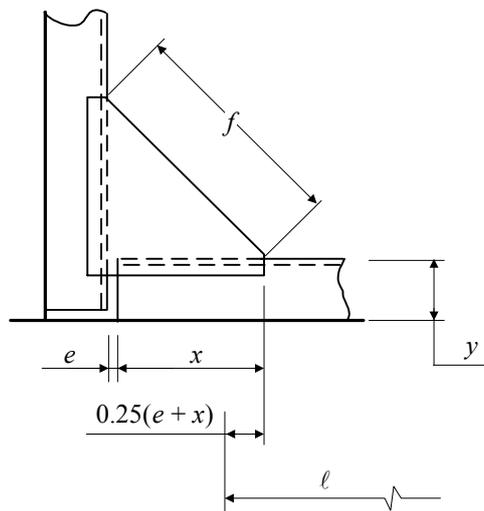
Metric

Length of Face <i>f</i> , mm	Thickness, mm		Width of Flange, mm
	Plain	Flanged	
Not exceeding 305	5.0	—	—
Over 305 to 455	6.5	5.0	38
Over 455 to 660	8.0	6.5	50
Over 660 to 915	9.5	8.0	63
Over 915 to 1370	11.0	9.5	75

Inch

Length of Face <i>f</i> , in.	Thickness, mm		Width of Flange, in.
	Plain	Flanged	
Not exceeding 12	$\frac{3}{16}$	—	—
Over 12 to 18	$\frac{1}{4}$	$\frac{3}{16}$	$1\frac{1}{2}$
Over 18 to 26	$\frac{5}{16}$	$\frac{1}{4}$	2
Over 26 to 36	$\frac{3}{8}$	$\frac{5}{16}$	$2\frac{1}{2}$
Over 36 to 54	$\frac{7}{16}$	$\frac{3}{8}$	3

FIGURE 1
Bracket



7 Structural Sections

The scantling requirements of these Rules are applicable to structural angles, channels, bars, rolled or built-up sections.

7.1 Deep Supporting Members

The required section modulus of members, such as girders, webs, etc., supporting frames, beams and stiffeners, is to be obtained on an effective width of plating basis, in accordance with this subsection. The section is to include the structural member in association with an effective width of plating not exceeding one-half the sum of spacing on each side of the member or 33% of the unsupported span ℓ , whichever is less. For girders and webs along hatch openings, an effective breadth of plating not exceeding one-half the spacing or 16.5% of the unsupported span, ℓ , whichever is less, is to be used. The section modulus of a shape, bar or fabricated section not attached to plating is that of the member only.

7.3 Frames, Beams and Stiffeners

7.3.1 Section Modulus

The required section modulus is assumed to be provided by the stiffener and a maximum of one frame space of the plating to which it is attached.

7.3.2 Web Thickness

The ratio of depth to thickness of the web portion of members is not to exceed the following:

Members with flange		$50C_1C_2$
Members without flange		$15C_1C_2$
C_1	=	0.95 (horizontal web within tank)
	=	1.0 (all other cases)
C_2	=	1.0 (ordinary strength steel)
	=	0.92 (HT32)
	=	0.90 (HT36)

7.5 Hold Frames of Single Side Skin Bulk Carriers (1998)

The hold frames of dry cargo vessels with typical bulk carrier configuration (sloping upper and lower wing tanks with a transversely framed side shell in way of the hold), in addition to the requirements of Section 3-2-5, are to comply insofar as is practical with the requirements for hold frames given in the following sections of the Rules.

- 5C-4-2/11.1, "Strength of Frame and Supporting Structure," of the *ABS Rules for Building and Classing Steel Vessels (Steel Vessel Rules)*
- 3-2-16/Table 1, "Welding Requirements," of these Rules

9 Structural Design Details

9.1 General

The designer is to give consideration to the following:

- i) The thickness of internals in locations susceptible to rapid corrosion.
- ii) The proportions of built-up members to comply with established standards for buckling strength.
- iii) The design of structural details such as noted below against the harmful effects of stress concentrations and notches:
 - Details of the ends, the intersections of members and associated brackets.
 - Shape and location of air, drainage or lightening holes.
 - Shape and reinforcement of slots or cut-outs for internals.
 - Elimination or closing of weld scallops in way of butts, “softening” of bracket toes, reducing abrupt changes of section or structural discontinuities.
- iv) Proportions and thickness of structural members to reduce fatigue response due to engine, propeller or wave-induced cyclic stresses, particularly for higher-strength steels.

Standard construction details based on the above considerations are to be indicated on the plans or in a booklet submitted for review and comment.

9.3 Termination of Structural Members (1998)

Unless permitted elsewhere in the Rules, structural members are to be effectively connected to the adjacent structure in such a manner as to avoid hard spots, notches and other harmful stress concentrations.

Where load bearing members are not required to be attached at their ends, special attention is to be given to the end taper by using a sniped end of not more than 30°.

The end brackets of large primary load-bearing members are to be soft-toed. Where any end bracket has a face bar, it is to be sniped and tapered not more than 30°.

Bracket toes and sniped end members are to be kept within 25 mm (1.0 in.) of the adjacent member, unless the bracket or member is supported by another member on the opposite side of the plating. The depth of toe or sniped end is generally not to exceed 15 mm (0.60 in.).

Where a strength deck or shell longitudinal terminates without end attachment, it is to extend into the adjacent transversely framed structure or stop at a local transverse member fitted at about one transverse frame space, see 3-2-5/1.3, beyond the last floor or web that supports the longitudinal.

The end attachments of non-load bearing members may, in general, be snipe ended. The snipe end is to be not more than 30° and is to be kept generally within 40 mm (1.57 in.) of the adjacent member unless it is supported by a member on the opposite side of the plating. The depth of the toe is generally not to exceed 15 mm (0.6 in.).

9.5 Fabrication (1998)

Structural fabrication is to be carried out in accordance with a recognized standard to the satisfaction of the attending surveyor. If a recognized national standard or an appropriate shipbuilding and repair standard is not available, the separate ABS publication, *Guide for Shipbuilding and Repair Quality Standard for Hull Structures During Construction*, may be used. These standards in the Guide are for conventional ship types and hull structures and they are not applicable to critical and highly stressed areas of the structure, which are to be reviewed and verified on an individual basis.

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PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **1 Longitudinal Strength**

1 General

Vessels are to have longitudinal hull girder section modulus in accordance with the requirements of this section. The equation in this section is, in general, valid for all vessels having breadths, B , which do not exceed two times their depths, D , as defined in Section 3-1-1. Vessels whose proportions exceed these limits will be subject to special consideration.

3 Longitudinal Hull Girder Strength

3.1 Minimum Section Modulus

The minimum required hull girder section modulus, SM , at amidships, is to be determined in accordance with the following equation:

$$SM = C_1 C_2 L^2 B (C_b + 0.7) \quad \text{m-cm}^2 \text{ (ft-in}^2\text{)}$$

where

C_1	=	$30.67 - 0.98L$	$12 \leq L < 18 \text{ m}$
	=	$22.40 - 0.52L$	$18 \leq L < 24 \text{ m}$
	=	$15.20 - 0.22L$	$24 \leq L < 35 \text{ m}$
	=	$11.35 - 0.11L$	$35 \leq L < 45 \text{ m}$
	=	6.40	$45 \leq L < 61 \text{ m}$
	=	$0.0451L + 3.65$	$61 \leq L < 90 \text{ m}$
C_1	=	$30.67 - 0.299L$	$40 \leq L < 59 \text{ ft}$
	=	$22.40 - 0.158L$	$59 \leq L < 79 \text{ ft}$
	=	$15.20 - 0.067L$	$79 \leq L < 115 \text{ ft}$
	=	$11.35 - 0.033L$	$115 \leq L < 150 \text{ ft}$
	=	6.40	$150 \leq L < 200 \text{ ft}$
	=	$0.0137L + 3.65$	$200 \leq L < 295 \text{ ft}$

- $C_2 = 0.01$ (0.01, 0.000144)
 $L =$ length of vessel, as defined in 3-1-1/3, in m (ft)
 $B =$ breadth of vessel, as defined in 3-1-1/5, in m (ft)
 $C_b =$ block coefficient at design draft, based on the length, L , measured on the design load waterline. C_b is not to be taken as less than 0.60.

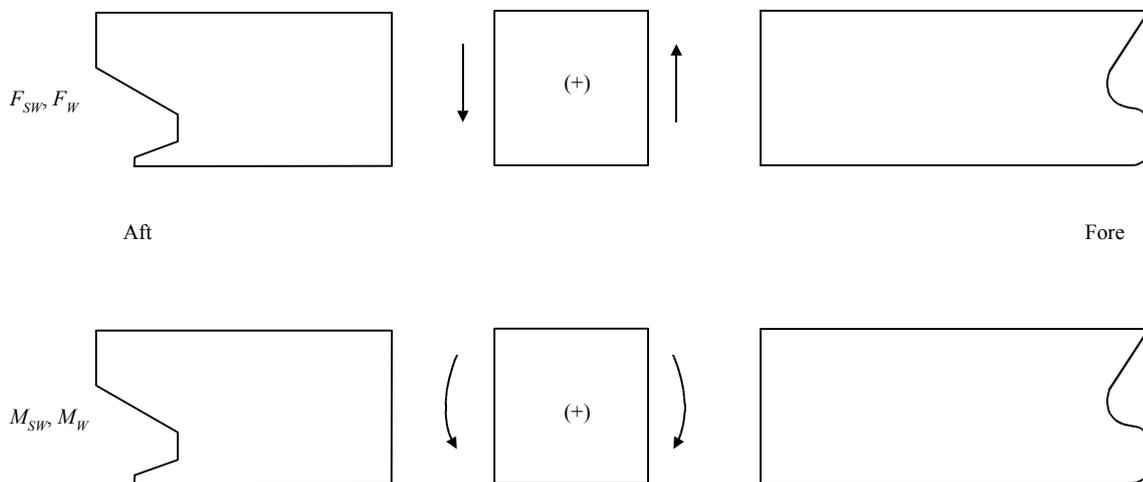
3.3 Vessels 61 m (200 ft) in Length and Over

In addition to meeting the above criteria in 3-2-1/3.1, vessels of 61 m (200 ft) in length or greater are to comply with the following requirements.

3.3.1 Sign Convention of Bending Moment and Shear

The sign convention bending moment and shear force is as shown in 3-2-1/Figure 1.

FIGURE 1
Sign Convention



3.3.2 Still-water Bending Moment and Shear Force

Still-water bending moment and shear force calculations showing hull girder shear force and bending moment values along the entire vessel length for the anticipated loaded, transitional and ballasted conditions are to be submitted together with the distribution of lightship weights.

3.3.3 Wave Loads

3.3.3(a) *Wave Bending Moment Amidships.* The wave bending moment, expressed in kN-m (tf-m, Ltf-ft), may be obtained from the following equations:

$$M_{ws} = -k_1 C_1 L^2 B (C_b + 0.7) \times 10^{-3} \quad \text{Sagging Moment}$$

$$M_{wh} = +k_2 C_1 L^2 B C_b \times 10^{-3} \quad \text{Hogging Moment}$$

where

$$k_1 = 110 \text{ (11.22, 1.026)}$$

$$k_2 = 190 \text{ (19.37, 1.772)}$$

$$C_1 = 0.044L + 3.75 \text{ SI/MKS units (0.0134L + 3.75 US customary units)}$$

- L = length of vessel, as defined in 3-1-1/3, in m (ft)
- B = breadth of vessel, as defined in 3-1-1/5, in m (ft)
- C_b = block coefficient at summer load waterline, based on L , as defined in 3-1-1/3

3.3.3(b) *Envelope Curve of Wave Bending Moment.* The wave bending moment along the length of the vessel L may be obtained by multiplying the midship value by the distribution factor M given in 3-2-1/Figure 2.

3.3.3(c) *Wave Shear Force.* The envelopes of maximum shearing forces induced by waves, F_w , as shown in 3-2-1/Figure 3 and 3-2-1/Figure 4, may be obtained from the following equations:

$$F_{wp} = +kF_1C_1LB(C_b + 0.7) \times 10^{-2} \quad \text{For positive shear force}$$

$$F_{wn} = -kF_2C_1LB(C_b + 0.7) \times 10^{-2} \quad \text{For negative shear force}$$

where

- F_{wp}, F_{wn} = maximum shearing force induced by wave, in kN (tf, Ltf)
- C_1 = as defined in 3-2-1/3.3.3(a)
- L = length of vessel, as defined in 3-1-1/3, in m (ft)
- B = breadth of vessel, as defined in 3-1-1/5, in m (ft)
- C_b = block coefficient at summer load waterline, based on L , as defined in 3-1-1/3
- k = 30 (3.059, 0.2797)
- F_1 = distribution factor, as shown in 3-2-1/Figure 3
- F_2 = distribution factor, as shown in 3-2-1/Figure 4

FIGURE 2
Distribution Factor M

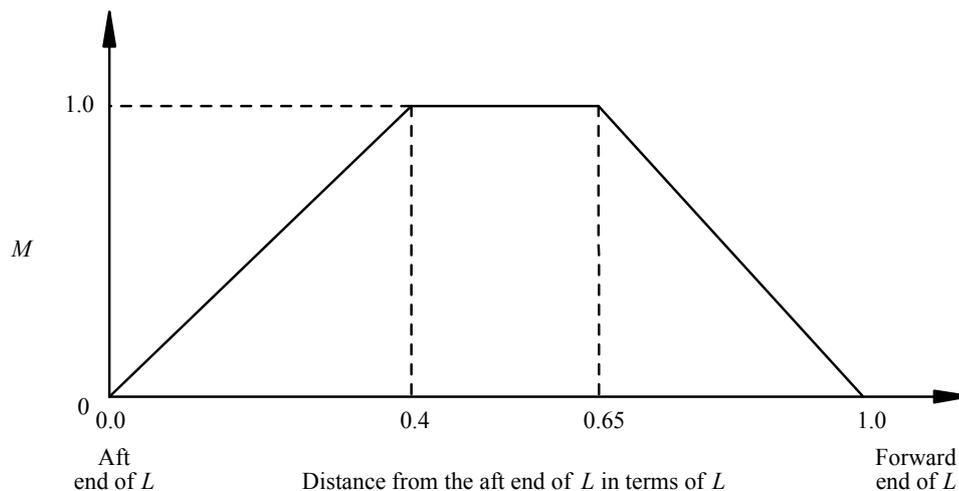


FIGURE 3
Distribution Factor F_1

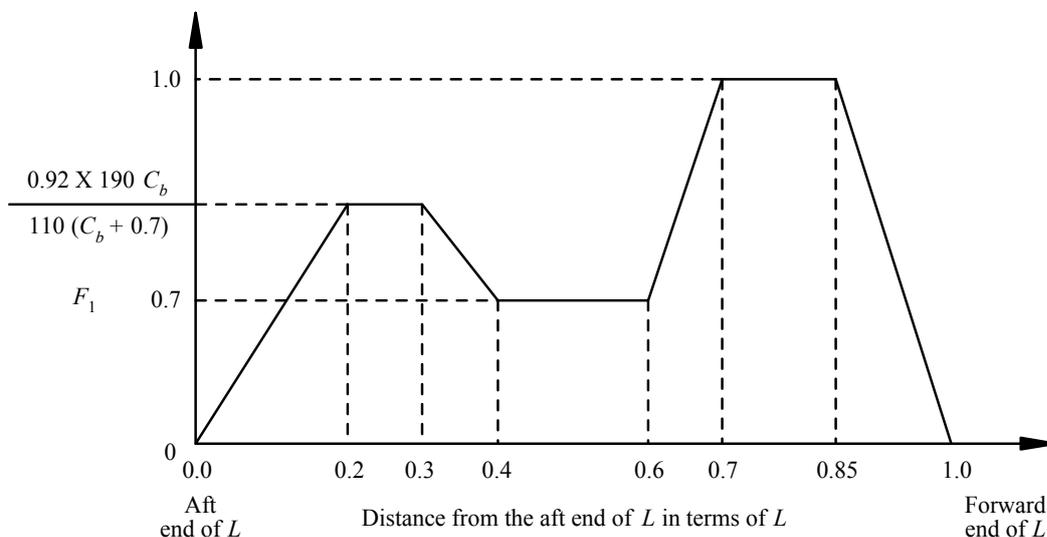
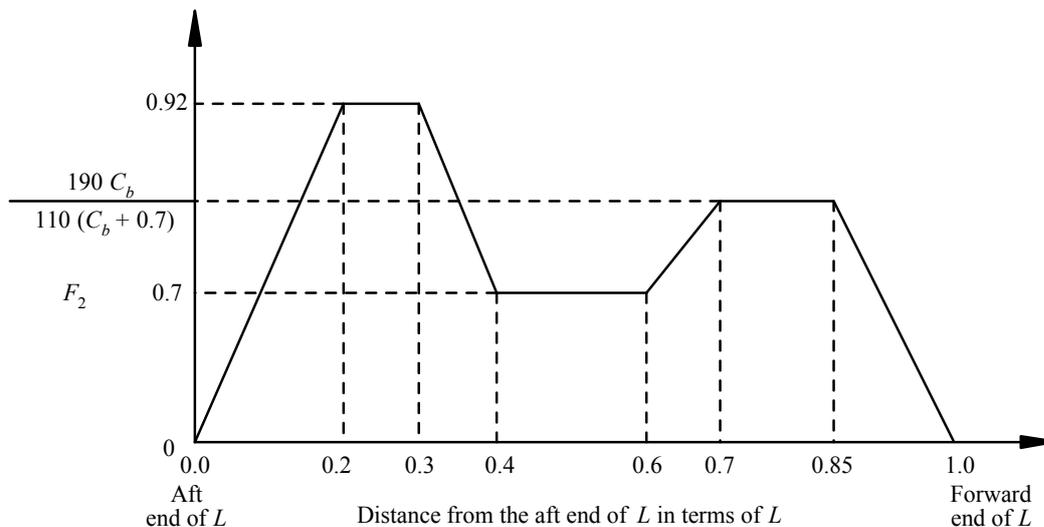


FIGURE 4
Distribution Factor F_2



3.3.4 Section Modulus

The required hull girder section modulus for $0.4L$ amidships is to be obtained from the following equation, or 3-2-1/3.1, whichever is greater.

$$SM = M_t f_p \quad \text{m-cm}^2 \text{ (ft-in}^2\text{)}$$

where

M_t = total bending moment to be obtained as the maximum algebraic sum (see sign convention in 3-2-1/3.3.1) of still-water bending moment and wave-induced bending moment, as follows.

$$M_t = M_{sw} + M_w$$

M_{sw} = still water bending moment, in accordance with 3-2-1/3.3.2

M_w = maximum wave-induced bending moment, in accordance with 3-2-1/3.3.3(a)

f_p = nominal permissible bending stress

= 17.5 kN/cm² (1.784 tf/cm², 11.33 Ltf/in²)

3.3.5 Shearing Strength

In calculating the nominal total shear stresses due to still-water and wave-induced loads, the maximum algebraic sum of the shearing force in still-water, F_{sw} , and that induced by wave, F_w , at the station examined is to be used. The thicknesses of the side shell and longitudinal bulkhead, where fitted, are to be such that the nominal total shear stresses, as obtained from 3-2-1/3.3.5(a) or 3-2-1/3.3.5(c), are not greater than 11.0 kN/cm² (1.122 tf/cm², 7.122 Ltf/in²). Where the side shell or longitudinal bulkhead is constructed of higher strength material, the permissible shear stresses may be increased by the factor, $1/Q$.

3.3.5(a) Shearing Strength for Vessels without Effective Longitudinal Bulkheads. For vessels without continuous longitudinal bulkheads, the nominal total shear stress, f_s , in the side shell plating may be obtained from the following equation:

$$f_s = (F_{sw} + F_w)m/2t_s I \quad \text{kN/cm}^2 \text{ (tf/cm}^2, \text{ Ltf/in}^2\text{)}$$

where

I = moment of inertia of the hull girder section, in cm⁴ (in⁴), at the section under consideration

m = first moment, in cm³ (in³), about the neutral axis, of the area of the effective longitudinal material between the horizontal level at which the shear stress is being determined and the vertical extremity of effective longitudinal material, taken at the section under consideration.

t_s = thickness of the side shell plating, in cm (in.), at the position under consideration.

F_{sw} = hull girder shearing force in still-water, in kN (tf, Ltf)

F_w = F_{wp} or F_{wn} , as specified by 3-2-1/3.3.3(c), depending upon loading

3.3.5(b) Modification of Hull Girder Shearing Force for Vessels Carrying Cargo in Alternate Holds or with Other Non-Uniform Loading. Where cargo is carried in alternate holds, the hull girder shearing force in still water, F_{sw} , to be used for calculating shear stresses in the side shell plating may be modified to account for the shear loads transmitted through the double bottom structure to the traverse bulkhead.

3.3.5(c) Shearing Strength for Vessels with Two or Three Longitudinal Bulkheads. For vessels having continuous longitudinal bulkheads, the total shear stresses in the side shell and longitudinal bulkhead plating are to be calculated by an acceptable method. In determining the still-water shear force, consideration is to be given to the effects of non-uniform athwartship distribution of loads. The method described in Appendix 3-2-A1 of the *Steel Vessel Rules* may be used as a guide in calculating the nominal total shear stress, f_s , related to the shear flow in the side shell or longitudinal bulkhead plating. Alternative methods of calculation will also be considered. One acceptable method is shown in Appendix 5C-2-A1 of the *Steel Vessel Rules*.

3.5 Hull Girder Moment of Inertia

The hull-girder moment of inertia of the vessel amidships, I , is to be not less than obtained from the following equation:

$$I = L(SM)/33.3 \quad \text{m}^2\text{-cm}^2 \text{ (ft}^2\text{-in}^2\text{)}$$

where

L = length of vessel, as defined in 3-1-1/3, in m (ft)

SM = hull girder section modulus required for the vessel in 3-2-1/3.1 or 3-2-1/3.3.4

5 Decks

5.1 Strength Decks

The uppermost deck to which the side shell plating extends for any part of the length of the vessel is to be considered the strength deck for that portion of the length, except in way of comparatively short superstructures. In such a case, the deck on which the superstructures are located is to be considered the strength deck in way of the superstructure. In general, the effective sectional area of the deck for use in calculating the section modulus is to exclude hatchways and other large openings through the deck but may include seam overlaps.

The deck sectional areas used in the section modulus calculations are to be maintained throughout the midship $0.4L$ in vessels. They may be reduced to one-half the normal requirement at $0.15L$ from the ends. In way of a superstructure beyond the midship $0.4L$, the strength deck area may be reduced to approximately 70% of the normal requirement at that location.

5.3 Effective Lower Decks

To be considered effective for use in calculating the hull girder section modulus, the thickness of the deck plating is to comply with the requirements of Section 3-2-3. The sectional areas of lower decks used in calculating the section modulus are to be obtained as described in 3-2-1/5.1. These areas are to be maintained throughout the midship $0.4L$ and may be gradually reduced to one-half their midship value at $0.15L$ from the ends.

7 Longitudinal Strength with Higher-Strength Materials

7.1 General

Vessels in which the effective longitudinal material of either the upper or lower flanges of the main hull girder, or both, are constructed of materials having mechanical properties greater than those of ordinary strength hull structural steel [see Section 2-1-2 of the *ABS Rules for Materials and Welding (Part 2)*], are to have longitudinal strength generally in accordance with the preceding paragraphs of this section, but the value of the hull girder section modulus may be modified as permitted by the following paragraphs. Applications of higher-strength material are to be continuous over the length of the vessel to locations where the stress levels will be suitable for the adjacent mild steel structure. Higher strength steel is to be extended to suitable locations below the strength deck and above the bottom, so that the stress levels will be satisfactory for the remaining ordinary strength steel structure. The strength deck and bottom structure are to be longitudinally framed. The longitudinal framing members are to be essentially of the same material as the plating they support and are to be continuous throughout the required extent of higher strength steel. Calculations showing that adequate strength has been provided against buckling are to be submitted for review and care is to be exercised against the adoption of reduced thicknesses of materials which may be subject to damage during normal operations.

7.3 Hull Girder Moment of Inertia

The hull-girder moment of inertia is to be not less than required by 3-2-1/3.5.

7.5 Hull Girder Section Modulus

When either the top or the bottom flange of the hull girder, or both, is constructed of higher-strength material, the section modulus, as obtained from 3-2-1/3.1 or 3-2-1/3.3.4, may be reduced by the factor Q .

$$SM_{hts} = Q(SM)$$

where

$$Q = 0.78 \text{ for Grade H32}$$

$$Q = 0.72 \text{ for Grade H36}$$

H32, H36 are as specified in Section 2-1-3 of the *ABS Rules for Materials and Welding (Part 2)*.

Q factor for steels having other yield point or yield strength will be specially considered.

9 Loading Guidance (1 July 1998)

9.1 Loading Manual and Loading Instrument (2009)

All vessels that are contracted for construction on or after July 1998 are to be provided with a loading manual. Loading instruments are not required by these Rules. However, when fitted, a loading instrument is to be in accordance with Appendix 3-2-A1. **9.3 Allowable Stresses**

9.3.1 At Sea

See 3-2-1/3.3.4 for bending stress and 3-2-1/3.3.5 for shear stress for vessels with ordinary strength steel material. For higher strength steel, the allowable stress may be increased by a factor of $1/Q$ where Q is as defined in 3-2-1/7.5.

9.3.2 In Port

The allowable in-port stress is 13.13 kN/cm² (1.34 tf/cm², 8.5 Ltf/in²) for bending and 10 kN/cm² (1.025 tf/cm², 6.5 Ltf/in²) for shear. For higher strength steel, the allowable stress may be increased by a factor of $1/Q$ where Q is as defined in 3-2-1/7.5.

11 Section Modulus Calculation

11.1 Items Included in the Calculation

In general, the following items may be included in the calculation of the section modulus, provided they are continuous or effectively developed within midship, $0.4L$, and gradually tapered beyond the midship, $0.4L$. Where the scantlings are based on the still-water bending moment envelope curves, items included in the hull girder section modulus amidships are to be extended as necessary to meet the hull girder section modulus required at the location being considered.

- Deck plating (strength deck and other effective decks)
- Shell and inner bottom plating
- Deck and bottom girders
- Plating and longitudinal stiffeners of longitudinal bulkheads
- All longitudinals of deck, sides, bottom and inner bottom

- Continuous longitudinal hatch coamings. See 3-2-1/13.

11.3 Effective Areas Included in the Calculation

In general, the net sectional areas of longitudinal strength members are to be used in the hull girder section modulus calculations, except that small isolated openings need not be deducted, provided the openings and the shadow area breadths of the other openings in any one transverse section do not reduce the section modulus by more than 3%. The breadth or depth of such openings is not to be greater than 1200 mm (47 in.) or 25% of the breadth or depth of the member in which it is located, whichever is less, with a maximum of 75 mm (3 in.) for scallops. The length of small isolated openings not required to be deducted is generally not to be greater than 2500 mm (100 in.) The shadow area of an opening is the area forward and aft of the opening enclosed by the lines tangential to the corners of the opening intersecting each other to form an included angle of 30 degrees.

11.5 Section Modulus to the Deck or Bottom

The section modulus to the deck or bottom is obtained by dividing the moment of inertia by the distance from the neutral axis to the molded deck at side amidships or baseline, respectively.

11.7 Section Modulus to the Top of Hatch Coamings

For continuous longitudinal hatch coamings, in accordance with 3-2-1/13, the section modulus to the top of the coaming is to be obtained by dividing the moment of inertia by the distance from the neutral axis to the deck at side plus the coaming height. This distance need not exceed y_t as given by the following equation, provided y_t is not less than the distance to the molded deck line at side.

$$y_t = y (0.9 + 0.2x/B) \quad \text{m (ft)}$$

where

- y = distance, in m (ft), from the neutral axis to the top of the continuous coaming
- x = distance, in m (ft), from the top of the continuous coaming to the centerline of the vessel
- B = breadth of the vessel, as defined in 3-1-1/5, in m (ft). x and y are to be measured to the point giving the largest value of y_t .

Section modulus to the top of longitudinal hatch coamings between multi-hatchways will be subject to special consideration.

13 Continuous Longitudinal Hatch Coamings and Above-Deck Girders

Where strength deck longitudinal coamings of length greater than $0.14L$ are effectively supported by longitudinal bulkheads or deep girders, the coamings are to be longitudinally stiffened, in accordance with 3-2-12/7.7. The section modulus amidships to the top of the coaming is to be as required by 3-2-1/3.1, 3-2-1/3.3, and 3-2-1/11.7, but the section modulus to the deck at side, excluding the coaming, need not be determined in way of such coaming.

Continuous longitudinal girders on top of the strength deck are to be similarly considered. Their scantlings are also to be in accordance with Section 3-2-6.

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CHAPTER **2 Hull Structures and Arrangements**

APPENDIX **1 Loading Manuals and Loading Instruments (1 July 1998)**

Note: These requirements are intended to satisfy Regulation 10(1) of the International Convention on Load Lines, 1966.

1 General

1.1 Application

The requirements in Appendix 3-2-A1 apply to all classed vessels 65 m (213 ft) and above in length (L_p) that are contracted for construction on or after 1 July 1998.

3 Definitions

3.1 Loading Guidance

Loading guidance is a generic term covering both loading manual and loading instrument as defined below.

3.1.1 Loading Manual

A loading manual is a document with sufficient information to enable the master of the vessel to arrange for the loading and ballasting of the vessel in such a way as to avoid the creation of any unacceptable stresses in the vessel's structure.

3.1.2 Loading Instrument

A loading instrument is an instrument by means of which it can be easily and quickly ascertained that the still water bending moments, shear forces and, where applicable, the still water torsional moments and lateral loads at the specified points along the length of the vessel will not exceed the specified values in any loaded or ballast condition.

3.3 Category I Vessels

Category I vessels are any one of the following vessels.

3.3.1

Vessels with large deck openings where combined stresses due to vertical and horizontal hull girder bending and torsional and lateral loads need to be considered.

3.3.2

Vessels designed for non-homogeneous loading where the cargo and/or ballast may be unevenly distributed, except those belonging to 3-2-A1/3.5.3.

3.3.3

Chemical carriers and gas carriers.

3.5 Category II Vessels

Category II vessels are any one of the following vessels.

3.5.1

Vessels with such arrangement as will result in small possibilities for variation in the distribution of cargo and ballast.

3.5.2

Vessels on regular and fixed trading pattern where the loading manual gives sufficient guidance.

3.5.3

Vessels of which the design takes into account the uneven distribution of cargo or ballast.

5 Required Loading Guidance (2003)

5.1 Loading Manual

All vessels are to be provided with a loading manual reviewed and stamped by ABS, in accordance with 3-2-A1/7, with the exception that a loading manual is not required for Category II vessels where the deadweight does not exceed 30% of the displacement at the summer load line.

5.3 Modifications

Where the modifications to the vessel or to the loading/trading pattern result in changes to the input information, a revised or new loading manual is to be submitted and a stamped copy to be placed aboard to replace the existing manual. The loading instrument is to be re-verified in accordance with 3-2-A1/9.3 or newly installed and verified in such cases.

Where the changes due to modification of the vessel are such that the still water bending moments and shear forces corresponding to the new loading conditions are within $\pm 2\%$ of the existing allowable values, the existing allowable values need not be modified.

7 Loading Manual

7.1 Required Information

The loading manual is to be based on the final data of the vessel and is to include at least the following information:

- i)* The loading conditions based on which the design of the vessel is approved.
- ii)* The results of the calculations of still water bending moments, shear forces.
- iii)* Permissible limits of still water bending moment and shear force and, where applicable, limitations due to torsional and lateral loads.
- iv)* Maximum allowable tank top loading.
- v)* If cargoes other than bulk cargoes are contemplated, such cargoes are to be listed together with any specific instructions for loading.
- vi)* Maximum allowable load on deck and hatch covers. If the vessel is not approved to carry load on deck or hatch covers, that fact is to be clearly stated in the loading manual.

7.3 Loading Conditions

The above information is to be based on the intended service conditions. See 3-2-A1/Table 1 for the selection of loading conditions.

7.5 Language

The loading manual is to be prepared in, or include, a language understood by the user. English may be considered as a language understood by the user.

9 Loading Instrument

9.1 Type

A loading instrument is to be digital. Single point loading instrument is not acceptable.

9.3 Required Verifications

Before a loading instrument is accepted for the vessel, all relevant aspects of the instrument, including but not limited to the following, are to be demonstrated to the Surveyor for the Surveyor's personal verification:

- i)* That the instrument is type approved, where applicable
- ii)* That the instrument is based on the final data of the vessel
- iii)* That the number and position of read-out points are satisfactory
- iv)* That the relevant limits for all read-out points are satisfactory
- v)* That the operation of the instrument after installation onboard, in accordance with the approved test conditions has been satisfactory
- vi)* That approved test conditions are available onboard
- vii)* That an operational manual, which does not require approval, is available onboard for the instrument

9.5 Language

The operation manual and the instrument output are to be prepared in, or include, a language understood by the user. English may be considered to be a language understood by the user.

11 Annual Surveys

The requirements in 7-3-2/1.1.5 of the *Rules for Survey After Construction (Part 7)* are to be complied with as follows: At each Annual Survey, loading manual is to be verified onboard and, where applicable, loading instrument is to be verified in working order. The operation manual for loading instrument is also to be verified onboard.

TABLE 1
Loading Conditions in the Loading Manual

1.	The loading manual is to include at least
1.1	full load conditions, subdivided into departure and arrival conditions,
1.2	ballast conditions, subdivided into departure and arrival conditions (see also 1.5)
1.3	critical loading conditions on which the design of the vessel is based.
1.4	in-port conditions (see also 1.5.3)
1.5	Intermediate conditions, including but not limited to
1.5.1	before and after any ballasting/deballasting during the voyage.
1.5.2	ballast exchange and its sequence, where intended,
1.5.3	during loading/unloading (for vessels in 2.1, 2.2 and, where applicable, 2.5)
2.	The following conditions are to be considered for the particular type of vessel. The list does not preclude any loading conditions that are necessary for the particular service intended:
2.1	Oil Carriers:
2.1.1	homogeneous cargo if consistent with the service of the vessel
2.1.2	cargoes of typical densities within the expected range
2.1.3	part loaded conditions
2.1.4	short voyages (e.g., half bunker)
2.1.5	tank cleaning conditions
2.1.6	docking conditions afloat
2.2	Bulk Carriers, Ore Carriers, Container Carriers, Dry Cargo Vessels, Other Specialized Carriers:
2.2.1	homogeneous cargo if consistent with the service of the vessel
2.2.2	cargoes of typical densities within the expected range
2.2.3	heavy cargo with empty holds or non-homogeneous conditions
2.2.4	short voyages (e.g., half bunker)
2.2.5	deck cargoes
2.2.6	docking conditions afloat
2.3	Liquefied Gas Carriers:
2.3.1	homogeneous loading for all approved cargoes
2.3.2	with empty or partially filled tank(s)
2.3.3	docking conditions afloat
2.4	Chemical Carriers:
2.4.1	conditions for oil carriers
2.4.2	all approved high density cargoes
2.5	Combination Carriers
2.5.1	conditions as specified in 2.1 and 2.2 above.

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PART

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CHAPTER **2 Hull Structures and Arrangements**

SECTION **2 Shell Plating**

1 General

Shell plating is to be of not less thickness than is required by the equations for thickness of side and bottom plating as required by this section, nor less than required by Section 3-2-1 for longitudinal strength and Section 3-2-8 for deep tank plating with h not less than the vertical distance to the freeboard deck at side.

3 Bottom Shell Plating

3.1 Extent of Bottom Plating

The term “bottom plating” refers to the plating from the keel to the upper turn of the bilge or upper chine.

3.3 Bottom Shell Plating

The thickness of the bottom shell plating throughout is not to be less than that obtained from the following equations:

3.3.1

$$t = \frac{s\sqrt{h}}{254} + 2.5 \text{ mm}$$

$$t = \frac{s\sqrt{h}}{460} + 0.10 \text{ in.}$$

where

- t = thickness of bottom shell plating, in mm (in.)
- s = frame spacing, in mm (in.)
- h = depth, D , in m (ft), as defined in 3-1-1/7.1, but not less than $0.1L$ or $1.18d$, whichever is greater
- d = draft for scantlings, as defined in 3-1-1/9, or $0.066L$, whichever is greater
- L = length of vessel, in m (ft), as defined in 3-1-1/3

3.3.2

$$t = \frac{s}{R} \sqrt{\frac{SM_R}{SM_A}} \cdot \frac{1}{\sqrt{Q}} \text{ mm (in.)}$$

where

t and s are as defined above.

- R = 45 with transverse framing
- = 55 with longitudinal framing
- SM_R = hull girder section modulus required by 3-2-1/3, in cm²-m (in²-ft)
- SM_A = bottom hull girder section modulus, in cm²-m (in²-ft)
- Q = as defined in 3-2-1/7.5

3.5 Bottom Forward

For vessels of 61 m (200 ft) in length and above, where the heavy weather ballast draft or operating draft forward is less than $0.04L$, the plating on the flat of bottom forward, forward of the location given in 3-2-4/Table 1 is to be not less than required by the following equation:

$$t = 0.0046s \sqrt{(0.005L^2 - 1.3d_f^2)/d_f} \text{ mm}$$

$$t = 0.0026s \sqrt{(0.005L^2 - 1.3d_f^2)/d_f} \text{ in.}$$

where

- s = frame spacing, in mm (in.)
- L = length of vessel, as defined in 3-1-1/3
- d_f = heavy weather ballast draft at the forward perpendicular, in m (ft)

5 Side Shell Plating

5.1 General (1998)

The side shell plating is not to be less in thickness than that obtained from the following equation:

$$t = \frac{s\sqrt{h}}{268} + 2.5 \text{ mm}$$

$$t = \frac{s\sqrt{h}}{485} + 0.10 \text{ in.}$$

where

- t = thickness, in mm (in.)
- s = spacing of transverse frames or longitudinals, in mm (in.)
- h = depth, in m (ft), as defined in 3-1-1/7, but not less than $0.1L$ or $1.18d$, whichever is greater
- d = draft for scantlings, as defined in 3-1-1/9, or $0.066L$, whichever is greater
- L = length of the vessel, as defined in 3-1-1/3

t is not to be taken less than 8.5 mm (0.33 in.) for offshore support vessels.

The side shell plating in way of hold frames of dry cargo vessels with typical bulk carrier configuration (sloping upper and lower wing tanks with a transversely framed side shell in way of the hold) is also not to be less than that obtained from the following equation:

$$t = \sqrt{L} \text{ mm}$$

$$t = 0.0218\sqrt{L} \text{ in.}$$

with L as defined above.

5.3 Recommendation for Vessels Subject to Impact

For vessels subject to impact loadings during routine operations, it is recommended that a side shell 25% greater in thickness than that obtained from the equation in 3-2-2/5.1 be provided.

5.5 Side Shell Plating at Ends

The minimum side shell plating thickness, t , at ends is to be obtained from the following equations and is not to extend for more than $0.1L$ from the ends. Between the midship $0.4L$ and the end $0.1L$, the thickness of the plating may be gradually tapered.

$$t = 0.0455L + 0.009s \text{ mm}$$

$$t = 0.000545L + 0.009s \text{ in.}$$

where

s = frame spacing, in mm (in.)

L = length of vessel, as defined in 3-1-1/3, in m (ft)

Where the strength deck at the ends is above the freeboard deck, the thickness of the side plating above the freeboard deck may be reduced to the thickness given for forecastle and poop sides at the forward and after ends, respectively.

5.7 Forecastle and Poop Side Plating

5.7.1 Forecastle Side Plating

The thickness, t , of the plating is to be not less than that obtained from the following equation:

$$t = 0.038(L + 30.8) + 0.006s \text{ mm}$$

$$t = 0.00045(L + 103.3) + 0.006s \text{ in.}$$

5.7.2 Poop Side Plating

The thickness, t , of the plating is to be not less than that obtained from the following equation:

$$t = 0.0296(L + 39.5) + 0.006s \text{ mm}$$

$$t = 0.00035(L + 132.9) + 0.006s \text{ in.}$$

where

s = spacing of frames, in mm (in.)

L = length of vessel, as defined in 3-1-1/3, in m (ft)

7 Bow and Stern Thruster Tunnels

The thickness of the tunnel plating is to be not less than that required by 3-2-2/5.5, nor is the thickness to be less than that obtained from the following equation:

$$t = 0.008d + 3.3 \text{ mm}$$

$$t = 0.008d + 0.13 \text{ in.}$$

where

$$d = \text{inside diameter of the tunnel, in mm (in.), but is to be taken as not less than 968 mm (38 in.)}$$

Where the outboard ends of the tunnel are provided with bars or grids, the bars or grids are to be effectively secured.

9 Local Strengthening for Research Vessels

In vessels used for research, wear plates or rollers are recommended at all places where research methods or gear will subject the shell plating to accelerated wear. Special strengthening may be required in areas where small boats are regularly launched, retrieved or stowed. Special strengthening may also be required in areas where the vessel makes contact with another vessel when pursuing, hauling, brailing, pumping, loading, unloading or running together.

11 Compensation

Compensation is to be provided for openings in the shell plating where required to maintain the longitudinal and transverse strength of the hull. All openings are to have well-rounded corners. Those in the upper side shell are to be located a suitable distance below the deck edge. Cargo and gangway openings are to be kept well clear of other discontinuities in the hull girder. Local provision is to be made to maintain the longitudinal and transverse strength of the hull.

Thick plating or doublers of sufficient breadth to prevent damage from the flukes of stockless anchors are to be fitted around the hawse pipes.

13 Breaks

Breaks in vessels having partial superstructures are to be specially strengthened to limit the local increases in stresses at these points. The stringer plates and sheer strakes at the lower level are to be increased in thickness well beyond the break in both directions. The thickness is to be increased 25% in way of breaks of superstructures. The side plating of the superstructure is to be increased in thickness and the side plating is to extend well beyond the end of the superstructure in such fashion as to provide a long gradual taper. Where the breaks of the forecastle or poop are appreciably beyond the midship $0.5L$, these requirements may be modified. Gangways, large freeing ports, side shell doors, and other openings in the shell or bulwarks are to be kept well clear of the breaks, and any holes which must unavoidably be cut in the plating are to be kept as small as possible and are to be circular or oval in form.

15 Bilge Keels

Bilge keels, where fitted, are to be attached to the shell by a doubler. In general, both the bilge keel and the doubler are to be continuous. The connection of the bilge keel to the doubler, and the doubler to the shell, are to be by double continuous fillet welds.

Butt welds in the bilge keel and doubler are to be full penetration and are to be kept clear of master erection butts. In general, shell butts are to be flush in way of the doubler. Doubler butts are to be flush in way of the bilge keel. In general, scallops and cutouts are not to be used. Where desired, a crack-arresting hole at least 25 mm (1 in.) in diameter may be drilled in the bilge keel butt weld as close as practicable to the doubler.

The ends of the bilge keel are to be suitably tapered and are to terminate on an internal stiffening member. The material tensile properties of bilge keels and doublers are to be as required for bottom shell plating.

17 Higher-strength Materials

17.1 General

In general, applications of higher-strength materials are to take into consideration the suitable extension of the higher-strength material above and below the bottom and deck, respectively, as required by 3-2-1/7.1. Care is to be taken against the adoption of reduced thickness of material that might be subject to damage during normal operation. The thickness of bottom and side-shell plating, where constructed of higher-strength materials, are to be not less than required for purposes of longitudinal hull girder strength; nor are they to be less than required by the foregoing paragraphs of this section when modified as indicated by the following paragraphs.

17.3 Bottom Plating of Higher-strength Material

Bottom shell plating, where constructed of higher-strength material and where longitudinally framed, is to be not less in thickness than that obtained from the following equation:

$$t_{hts} = (t_{ms} - C)\sqrt{Q} + C$$

where

- t_{hts} = thickness of higher-strength material, in mm (in.)
- t_{ms} = thickness, in mm (in.), of ordinary-strength steel, as required by preceding paragraphs of this section, or from the requirements of other sections of the Rules, appropriate to the vessel type.
- C = 4.3 mm (0.17 in.)
- Q = as defined in 3-2-1/7.5

17.5 Side Plating of Higher-strength Material

Side-shell plating, where constructed of higher-strength material, is to be not less in thickness than that obtained from the following equation:

$$t_{hts} = [t_{ms} - C][(Q + 2\sqrt{Q})/3] + C$$

where t_{hts} , t_{ms} , C and Q are as defined in 3-2-2/17.3 for bottom plating.

17.7 End Plating

End-plating thickness, including plating on the flat of bottom forward, where constructed of higher-strength materials, will be subject to special consideration.

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CHAPTER **2 Hull Structures and Arrangements**

SECTION **3 Deck Plating**

1 General

The thickness of the deck plating is not to be less than that required to obtain the hull-girder section modulus given in Section 3-2-1, nor less than required by this section.

3 Deck Plating

The thickness of plating on each deck is to be not less than the greater of those obtained from the following equations. The required thickness is not to be less than 5.0 mm (0.20 in.), except for platform decks in enclosed passenger spaces where the thickness is not to be less than 4.5 mm (0.18 in.). Thickness of strength deck inside line of openings may be reduced by 1.0 mm (0.04 in.) from t obtained by 3-2-3/3.3 below.

3.1 All Decks

$$t = \frac{s\sqrt{h}}{254} + 2.5 \text{ mm}$$

$$t = \frac{s\sqrt{h}}{460} + 0.10 \text{ in.}$$

where

- t = thickness, in mm (in.)
- s = beam or longitudinal spacing, in mm (in.)
- h = height, in m (ft), as follows:
- = *for a deck or portion of deck forming a tank top*, the greater of the following distances:
 - two-thirds of the distance from the tank top to the top of the overflow, or
 - two-thirds of the distance from the tank top to the bulkhead deck or freeboard deck.
 - = *for a lower deck on which cargo or stores are carried*, the tween-deck height at side; where the cargo weights are greater than normal [7010 N/m³ (715 kgf/m³, 45 lbf/ft³)], h is to be suitably adjusted.

= for an exposed deck on which cargo is carried, 3.66 m (12 ft). Where it is intended to carry deck cargoes in excess of 25850 N/m² (2636 kgf/m², 540 lbf/ft²), this head is to be increased in proportion to the added loads which will be imposed on the structure

Elsewhere, the value of h is to be not less than that obtained from the appropriate equation below, where L is the length of vessel in m (ft), as defined in 3-1-1/3.

3.1.1 Exposed Freeboard Deck Having No Deck Below

$$h = 0.028L + 1.08 \text{ m}$$

$$h = 0.028L + 3.57 \text{ ft}$$

3.1.2 Exposed Freeboard Deck Having a Deck Below, Forecastle Deck, Superstructure Deck Forward of Amidships 0.5L

$$h = 0.028L + 0.66 \text{ m}$$

$$h = 0.028L + 2.14 \text{ ft}$$

3.1.3 Freeboard Deck within Superstructure, Any Deck Below Freeboard Deck, Superstructure Deck Between 0.25L Forward of and 0.20L Aft of Amidships

$$h = 0.014L + 0.87 \text{ m}$$

$$h = 0.014L + 2.86 \text{ ft}$$

3.1.4 All Other Locations

$$h = 0.014L + 0.43 \text{ m}$$

$$h = 0.014L + 1.43 \text{ ft}$$

3.3 Strength Decks within the Midship 0.8L (2002)

For vessels of length equal to or greater than 61 meters, the strength deck plating within the midship 0.8L shall meet the following requirement:

$$t = 0.009s + 2.4 \text{ mm}$$

$$t = 0.009s + 0.095 \text{ in.}$$

where

$$s = \text{beam or longitudinal spacing, in mm (in.)}$$

3.5 All Strength Deck Plating Outside the Line of Openings and Other Effective Deck Plating (2002)

For vessels of length equal to or greater than 61 meters, the strength deck plating within the midship 0.8L shall meet the following requirement:

$$t = \frac{s}{R} \sqrt{\frac{SM_R}{SM_A}} \cdot \frac{1}{\sqrt{Q}} \text{ mm (in.)}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$s = \text{beam or longitudinal spacing, in mm (in.), not to be taken less than 610 mm (24 in.)}$$

$$R = 60 \text{ for longitudinal framing, } 45 \text{ for transverse framing}$$

$$SM_R = \text{hull girder section modulus required in 3-2-1/3, in cm}^2\text{-m (in}^2\text{-ft)}$$

- SM_A = hull girder section modulus, in $\text{cm}^2\text{-m}$ ($\text{in}^2\text{-ft}$), measured to the deck in question
 Q = material factor for the material used in determining SM_R , as defined in 3-2-1/7.5

5 Compensation

Compensation is to be provided for openings in the strength deck and other effective decks to maintain the longitudinal and transverse strength. Openings in the strength deck are to have a minimum corner radius of 0.125 times the width of the opening, but need not exceed a radius of 600 mm (24 in.). In other decks, the radius is to be 0.09375 times the width of the opening, but need not exceed radius of 450 mm (18 in.). Openings are to be a suitable distance from the deck edge, from cargo hatch covers, from superstructure breaks and from other areas of structural discontinuity.

7 Wheel Loading

Where provision is to be made for the operation or stowage of vehicles having rubber tires, and after all other requirements are met, the thickness of deck plating is to be not less than that obtained from the following equation:

$$t = kKn\sqrt{CW} \text{ mm (in.)}$$

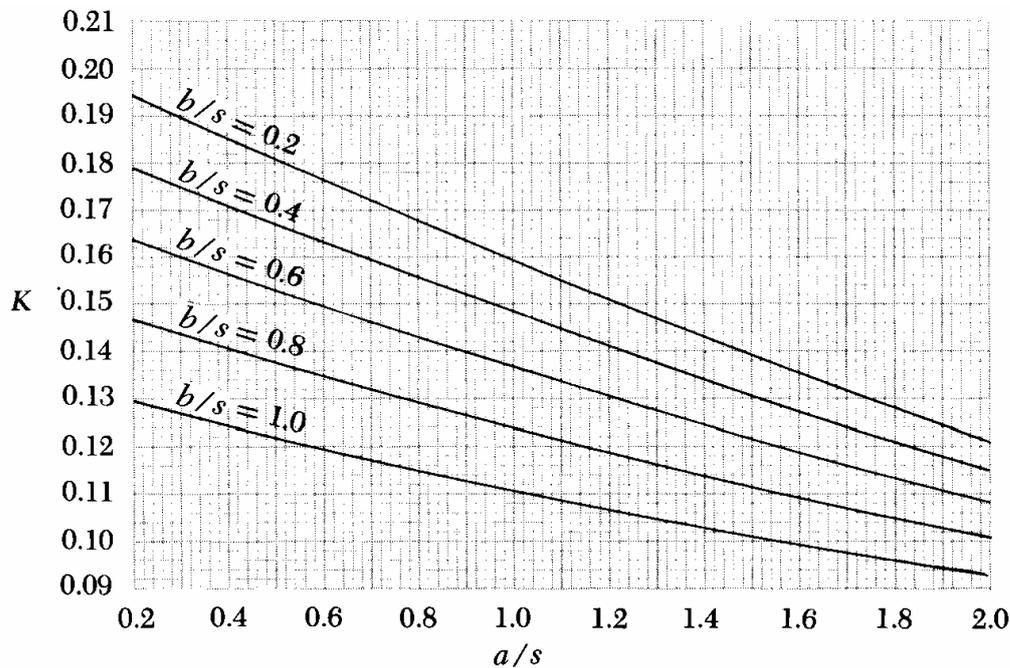
where

- k = 8.05 (25.2, 1)
 K = as given in 3-2-3/Figure 1
 n = 1.0 where $\ell/s > 2.0$ and 0.85 where $\ell/s = 1.0$. For intermediate values of ℓ/s , n is to be obtained by interpolation
 C = 1.5 for wheel loads of vehicles stowed at sea and 1.1 for vehicles operating in port
 W = static wheel load, in kN (tf, Ltf)
 a = wheel imprint dimension, in mm (in.), parallel to the longer edge, ℓ , of the plate panel
 b = wheel imprint dimension, in mm (in.), perpendicular to the longer edge, ℓ , of the plate panel
 s = spacing of deck beams or deck longitudinals, in mm (in.)
 ℓ = length of the plate panel, in mm (in.)

For wheel loading, the strength deck plating thickness is to be not less than 110% of that required by the above equation, and platform deck plating thickness is to be not less than 90% of that required by the above equation.

Where the wheels are close together, special consideration will be given to the use of the combined imprint and load. Where the intended operation is such that only the larger dimension of the wheel imprint is perpendicular to the longer edge of the plate panel, b above may be taken as the larger wheel imprint dimension, in which case, a is to be the lesser one.

FIGURE 1
Wheel Loading Curves of K



9 Higher-strength Material

9.1 Thickness

In general, applications of higher strength materials are to take into consideration the suitable extension of the higher strength material below the deck, forward and aft. Care is to be taken to avoid the adoption of reduced thickness of material such as might be subject to damage during normal operation. The thickness of deck plating for longitudinally framed decks, where constructed of higher-strength material, is to be not less than required for longitudinal strength, nor is it to be less than that obtained from the following equation:

$$t_{hts} = (t_{ms} - C) \sqrt{Q} + C$$

where

- t_{hts} = thickness of higher-strength material, in mm (in.)
- t_{ms} = thickness of ordinary-strength steel, in mm (in.), as required 3-2-3/3.1 and 3-2-3/3.3
- C = 4.3 mm (0.17 in.)
- Q = is as defined in 3-2-1/7.5

Where the deck plating is transversely framed, or where the Rules do not provide a specific thickness for the deck plating, the thickness of the higher-strength material will be specially considered, taking into consideration the size of the vessel, intended service and the foregoing Rule requirements.

9.3 Wheel Loading

Where deck or flats are constructed of higher strength material and provision is made for the operation or stowage of vehicles having rubber tires, the thickness of plating is to be not less than that obtained from the following equation:

$$t_{hts} = t_{ms} \sqrt{(M/Y)} \text{ mm (in.)}$$

where

- t_{hts} = thickness of higher-strength material, in mm (in.)
- t_{ms} = thickness of ordinary-strength steel, as obtained from 3-2-3/7
- Y = as defined in 3-2-7/5.1
- M = 235 (24, 34000)

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PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **4 Bottom Structure**

1 Double Bottoms

1.1 General

Inner bottoms are to be fitted fore and aft between the peaks or as near thereto as practicable in vessels of ordinary design of 500 GT or over. Where, for special reasons, it may be desired to omit the inner bottom, the arrangements are to be clearly indicated on the plans when first submitted for approval. A double bottom need not be fitted in way of deep tanks, provided the safety of the vessel in the event of bottom damage is not thereby impaired. It is recommended that the inner bottom be arranged to protect the bilges as much as possible and that it be extended to the sides of the vessel.

Shell longitudinals and frames in way of deep tanks are to have not less strength than is required for stiffeners on deep tank bulkheads.

1.3 Center Girder

A center girder is to be fitted extending as far forward and aft as practicable. The plates are to be continuous within the midship three-quarters length; elsewhere, they may be intercostal between floors. Where double bottoms are to be used for fuel oil or fresh water, the girders are to be intact, but need not be tested under pressure. This requirement may be modified in narrow tanks at the ends of the vessel or where other intact longitudinal divisions are provided at about $0.25B$ from centerline. Where the girders are not required to be intact, manholes may be cut in every frame space outside the midships three-quarters length; they may be cut in alternate frames spaces within the midships three-quarters length. For vessels which have a length more than 61 m (200 ft) and the length of the cargo hold is greater than $1.2B$, the thickness and depth of center girder plates are to be specially considered based on the results of a direct structural calculation.

1.3.1 Thickness Amidships

The thickness of the center girder within the midship one-half length is not to be less than that obtained from the following equation.

$$t = 0.056L + 5.5 \text{ mm}$$

$$t = 0.00067L + 0.22 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined 3-1-1/3}$$

1.3.2 Thickness at Ends

The thickness of the center girder forward and aft of the midship one-half length may be reduced to 85% of the girder thickness amidships.

1.3.3 Depth

The depth of the center girder is not to be less than that obtained from the following equation:

$$h_g = 32B + 190\sqrt{d} \text{ mm}$$

$$h_g = 0.384B + 4.13\sqrt{d} \text{ in.}$$

where

$$h_g = \text{depth, in mm (in.)}$$

$$B = \text{breadth of vessel, in m (ft), as defined in 3-1-1/5}$$

$$d = \text{draft for scantlings, in m (ft), as defined in 3-1-1/9}$$

1.5 Side Girders

Where the distance between the center girder and the side shell exceeds 4.57 m (15 ft), intercostal side girders are to be fitted approximately midway between the center girder and the side shell. The minimum thickness of the intercostal side girders is not to be less than obtained from the following equation.

$$t = 0.036L + c \text{ mm}$$

$$t = 0.00043L + c \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in 3-1-1/3}$$

$$c = 4.7 \text{ mm (0.18 in.)}$$

1.7 Floors

Solid floors are to be fitted at every frame under the engine room, at every frame in the peaks and under transverse bulkheads. Elsewhere, the solid floors are to have a maximum spacing of 3.66 m (12 ft) in association with intermediate open floors or longitudinal framing. The thickness of solid floors is to be equal to the thickness of side girders obtained in 3-2-4/1.5, except that for widely spaced floors in association with longitudinal framing, c is to be taken as 6.2 mm (0.24 in.).

1.9 Frames

In transversely framed vessels, open floors consisting of frames and reverse frames are to be fitted at all frames where solid floors are not fitted. Center and side brackets are to overlap the frames and reverse frames for a distance equal to $0.05B$. They are to be of the thickness required for side girders in the same location and are to be flanged on their outer edges. Alternatively, longitudinal framing is to be fitted in association with widely spaced floors. The section modulus, SM , of each frame, reverse frame or bottom, or inner bottom longitudinal in association with the plating to which it is attached is not to be less than that obtained from the following equation.

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- s = frame spacing, in m (ft)
- ℓ = unsupported span between supporting members, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by bulkheads, inner bottom, or side shell, the length, ℓ , may be measured as permitted therein.
- h = vertical distance, in m (ft), from the middle of ℓ to the deck at side. In way of a deep tank, h is the greatest distance from the middle of ℓ to a point located at two-thirds of the distance from the top of the tank to the top of the overflow; a point located above the top of the tank not less than $0.01L + 0.15$ m or 0.46 m ($0.01L + 0.5$ ft or 1.5 ft), whichever is greatest.

c for transverse frames and reverse frames:

- = 0.8 clear of tanks
- = 1.0 in way of tanks
- = 0.5 with struts

c for longitudinal frames:

- = 1.0 without struts
- = 0.55 with struts

c for inner bottom longitudinals:

- = 0.85 without struts
- = 0.45 with struts

Frames and reverse frames in way of tanks are not to be less than that required clear of tanks if that be greater.

1.11 Struts

Struts are to be angle bar sections fitted midway between floors. In general, they are not to be used where cargo is discharged by grabs, where heavy cargoes are carried or in the bottom forward slamming area. The permissible load, W_a , for struts is to be determined in accordance with 3-2-6/3.5.3. The calculated load, W , is to be determined by:

$$W = nphs \quad \text{kN (tf, Ltf)}$$

where

- n = 10.5 (1.07, 0.03)
- p = the sum of the half lengths, in m (ft), on each side of the strut, of the frames supported
- h = as defined in 3-2-4/1.9
- s = frame spacing, in m (ft)

Struts are to be positioned so as to divide the span into approximately equal intervals.

1.13 Inner-bottom Plating

The thickness of the inner-bottom plating throughout the length of the vessel is to be not less than that obtained from the following equation. Where applicable, the plating is to meet deep tank requirements.

$$t = 0.037L + 0.009s + c \quad \text{mm}$$

$$t = 0.000445L + 0.009s + c \quad \text{in.}$$

where

t	=	thickness, in mm (in.)
L	=	length of vessel, in m (ft), as defined in 3-1-1/3
s	=	frame spacing, in mm (in.)
c	=	1.5 mm (0.06 in.) in engine space
	=	-0.5 mm (-0.02 in.) elsewhere

Where no ceiling is fitted under cargo hatchways, except for vessels intended for the exclusive carriage of containers on the inner bottom, the thickness of the inner-bottom plating is to be increased 2.0 mm (0.08 in.). For vessels with longitudinally-framed inner bottoms, the minimum thickness of inner-bottom plating may be reduced by 1 mm (0.04 in.).

For vessels regularly engaged in trades where the cargo is handled by grabs or similar mechanical appliances, it is recommended that flush inner-bottom plating be adopted throughout the cargo space, and that the plating be suitably increased, but the increase need not exceed 5 mm (0.20 in.). It is also recommended that the minimum thickness be not less than 12.5 mm with 610 mm (0.50 in. with 24 in.) frame spacing and 19 mm with 915 mm (0.74 in. with 36 in.) frame spacing. Intermediate thicknesses may be obtained by interpolation.

Where provision is to be made for the operation or stowage of vehicles having rubber tires, and after all other requirements are met, the thickness of the inner bottom plating is to be not less than that obtained from 3-2-3/7.

Margin plates which are approximately horizontal are to have thicknesses not less than the adjacent inner bottom plating. Where they are nearly vertical, they are to be not less than the required inner bottom plating in the engine space and are to extend the full depth of the inner bottom.

1.15 Sea Chests

Where the double bottom structure forms part of a sea chest, the thickness of the plating is to be not less than the required thickness of the shell plating, using the approximate value of stiffener spacing, s .

1.17 Access, Lightening, Air, and Drainage Holes

Access holes in double bottom tank tops and lightening holes in nontight members are to be sufficient in size and number to assure accessibility to all parts of the double bottom. The proposed locations and sizes of the holes are to be indicated on the drawings submitted for approval. Tank top access hole covers are to be of steel or equivalent material, and where no ceiling is fitted in a cargo hold, the covers are to be protected against damage by the cargo. Air and drainage holes are to be cut in all nontight parts of the double bottom structure to assure the free escape of gases to the vents and the free drainage of liquids to the suction.

3 Single Bottoms with Floors and Keelsons

3.1 General

Where double bottom construction is not required by 3-2-4/1.1 or is not applied, single bottom construction is to be in accordance with 3-2-4/3 or 3-2-4/5, as may be applicable.

3.3 Center Keelsons

Single-bottom vessels are to have center keelsons formed of continuous or intercostal center girder plates with horizontal top plates. The thickness of the keelson and the area of the horizontal top plate are to be not less than that obtained from the following equations. Vessels less than 30.5 m (100 ft) in length will be subject to special consideration. Tapering of the horizontal top plate area at the ends is not normally considered for vessels less than 30.5 m (100 ft) in length. The keelsons are to extend as far forward and aft as practicable.

3.3.1 Center-girder Plate Thickness Amidships

$$t = 0.063L + 5 \text{ mm}$$

$$t = 0.00075L + 0.2 \text{ in.}$$

3.3.2 Center-girder Plates Thickness at Ends

$$t = 85\% \text{ of center keelson thickness amidships}$$

3.3.3 Horizontal Top-plate Area Amidships

$$A = 0.168L^{3/2} - 8 \text{ cm}^2$$

$$A = 0.0044L^{3/2} - 1.25 \text{ in}^2$$

3.3.4 Horizontal Top-Plate Area at Ends [$L \geq 30.5 \text{ m (100 ft)}$]

$$A = 0.127L^{3/2} - 1 \text{ cm}^2$$

$$A = 0.0033L^{3/2} - 0.15 \text{ in}^2$$

where

$$t = \text{thickness of center-girder plate, in mm (in.)}$$

$$L = \text{length of vessel, as defined in 3-1-1/3, in m (ft)}$$

$$A = \text{area of horizontal top plate, in cm}^2 \text{ (in}^2\text{)}$$

3.5 Side Keelsons

Side keelsons are to be arranged so that there are not more than 2.13 m (7 ft) from the center keelson to the inner side keelson, from keelson to keelson and from the outer keelson to the lower turn of bilge. Forward of the midship one-half length, the spacing of keelsons on the flat of floor is not to exceed 915 mm (36 in.). Side keelsons are to be formed of continuous rider plates on top of the floors. They are to be connected to the shell plating by intercostal plates. The intercostal plates are to be attached to the floor plates. In the engine space, the intercostal plates are to be of not less thickness than the center girder plates. The scantlings of the side keelsons are to be obtained from the following equations but need not exceed 3-2-4/3.3, if that be less.

3.5.1 Side Keelson and Intercostal Thickness Amidships

$$t = 0.063L + 4 \text{ mm}$$

$$t = 0.00075L + 0.16 \text{ in.}$$

3.5.2 Side Keelson and Intercostal Thickness at Ends

$$t = 85\% \text{ of center thickness amidships}$$

3.5.3 Side Keelson and Intercostal, Horizontal Top Plate Area Amidships

$$A = 0.038L^{3/2} + 17 \text{ cm}^2$$

$$A = 0.001L^{3/2} + 2.6 \text{ in}^2$$

3.5.4 Side Keelson and Intercostal, Horizontal Top Plate Area at Ends

$$A = 0.025L^{3/2} + 20 \text{ cm}^2$$

$$A = 0.00065L^{3/2} + 3.1 \text{ in}^2$$

t , L and A are as defined in 3-2-4/3.3.

3.7 Floors

3.7.1 Section Modulus

With transverse framing, a floor as shown in 3-2-4/Figure 1 is to be fitted on every frame and is to be of the scantlings necessary to obtain a section modulus, SM , not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.55$$

$$h = \text{draft, } d, \text{ in m (ft), as defined in section 3-1-1/9, but not to be less than } 0.66D \text{ or } 0.066L, \text{ whichever is greater.}$$

$$s = \text{floor spacing, in m (ft)}$$

$$\ell = \text{span, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by bulkheads, inner bottom or side shell, the length, } \ell, \text{ may be measured as permitted therein.}$$

The section modulus may be calculated at the centerline of the vessel, provided the rise of floor is such that the depth at the toe of brackets is not less than one-half of the depth at the centerline. The above requirements are limited to cargo holds where cargoes of specific gravity 0.715 or less are uniformly loaded. In way of engine room and in the forward 0.2L, the floor face bar area is to be doubled.

3.7.2 Depth

The minimum depth of floors at centerline is not be less than that obtained from the following equation:

$$h_f = 62.5\ell \text{ mm}$$

$$h_f = 0.75\ell \text{ in.}$$

where

$$h_f = \text{floor depth, in mm (in.)}$$

$$\ell = \text{unsupported span of floors, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5, the length, } \ell, \text{ may be measured as permitted therein.}$$

3.7.3 Thickness

The minimum thickness of floors is not to be less than that obtained from the following equation:

$$t = 0.01h_f + 3 \text{ mm}$$

$$t = 0.01h_f + 0.12 \text{ in.}$$

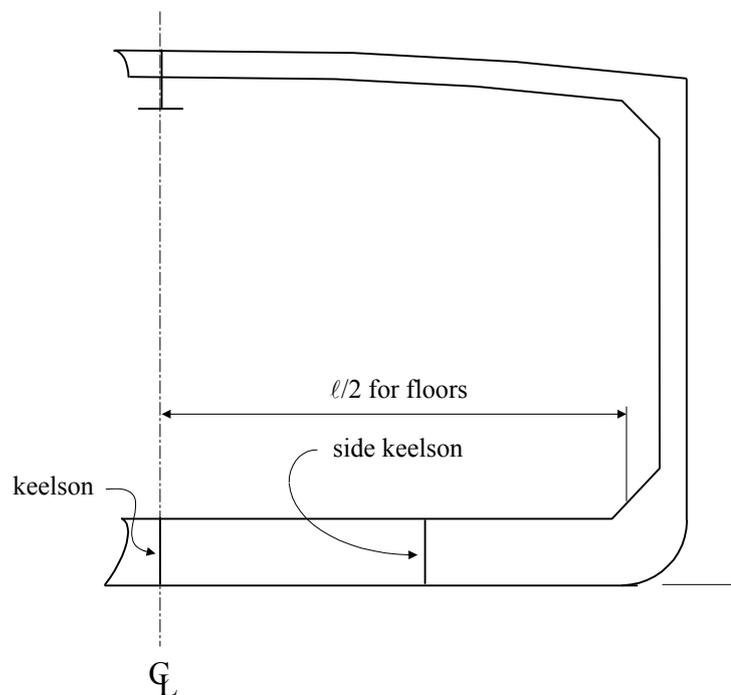
where

t = floor thickness, in mm (in.)

h_f = floor depth, in mm (in.)

Floors under engine girders are to be not less in thickness than the thickness required for keelsons.

**FIGURE 1
Plate Floors**



5 Single Bottoms with Longitudinal or Transverse Frames

5.1 General

Where longitudinal frames supported by bottom transverses or transverse frames supported by longitudinal girders and bottom transverses are proposed in lieu of keelsons referred to in 3-2-4/3, the construction is to be in accordance with this subsection. Frames are not to have less strength than is required for watertight bulkhead stiffeners or girders in the same location in association with head to the bulkhead deck. In way of deep tanks, frames are not to have less strength than is required for stiffeners or girders on deep tank bulkheads. See 3-2-4/Figure 2, 3-2-4/Figure 3 and 3-2-4/Figure 4.

FIGURE 2
Round Bottom Floors with Deadrise

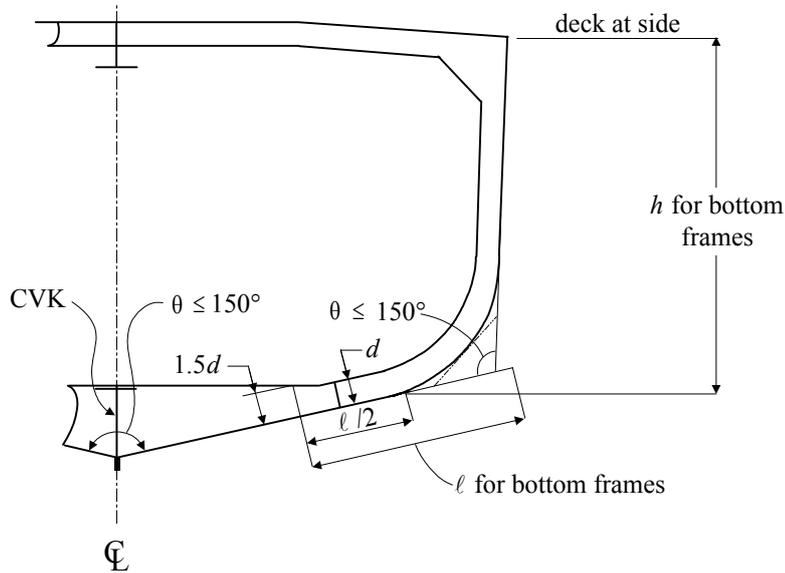


FIGURE 3
Transverse Bottom Frames with Longitudinal Side Girders

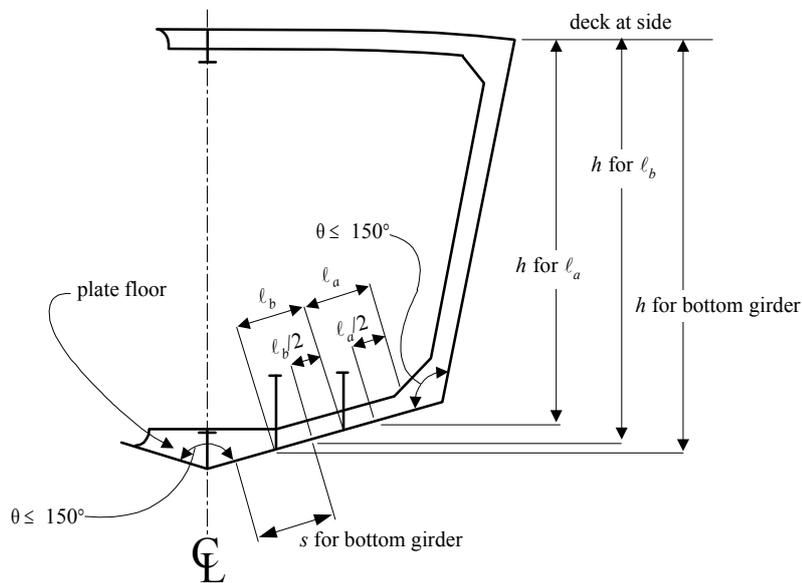
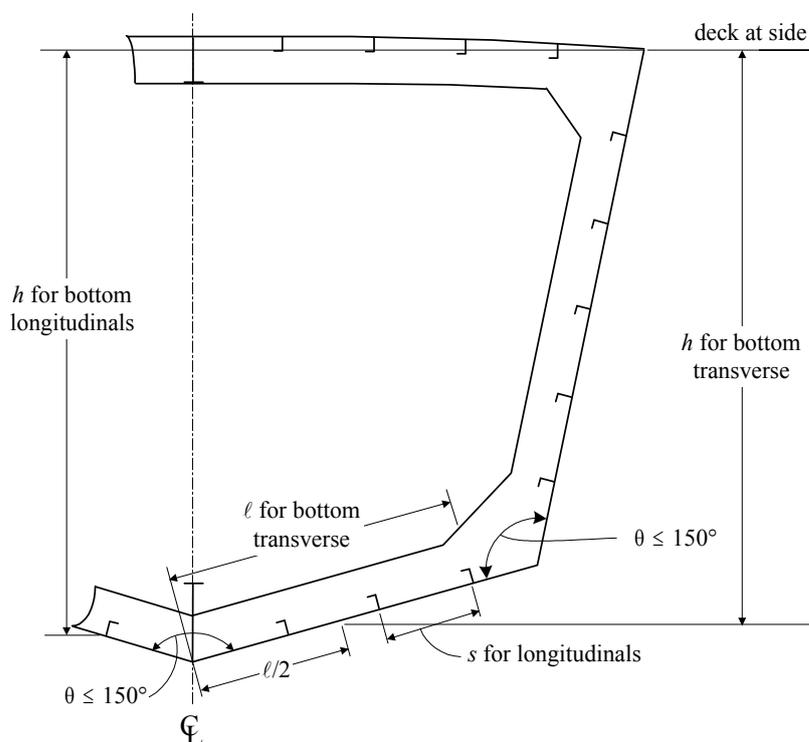


FIGURE 4
Longitudinal Frames with Transverse Webs



5.3 Bottom Girders and Transverses

5.3.1 Section Modulus

The section modulus, SM , of each bottom girder and transverse, where intended as a primary supporting member, in association with the plating to which it is attached, is not to be less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- c = 0.915
- h = vertical distance, in m (ft), from the center of area supported to the deck at side
- s = spacing, in m (ft)
- ℓ = unsupported span, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by bulkheads, inner bottom or side shell, the length, ℓ , may be measured as permitted therein.

Tripping brackets are to be fitted at intervals of about 3 m (10 ft) and stiffeners are to be fitted as may be required.

5.3.2 Depth

The minimum depth of the girder or transverse is to be not less than 2.5 times the depth of the cutouts for bottom frames, unless effective compensation for cutouts is provided, nor less than that obtained from the following equation:

$$h_w = 145\ell \text{ mm}$$

$$h_w = 1.75\ell \text{ in.}$$

where

$$h_w = \text{girder or transverse depth, in mm (in.)}$$

ℓ is defined in 3-2-4/5.3.1.

5.3.3 Thickness

The minimum thickness of the web is to be not less than that obtained from the following equation:

$$t = 0.01h_w + 3 \text{ mm}$$

$$t = 0.01h_w + 0.12 \text{ in.}$$

where

$$t = \text{floor thickness, in mm (in.)}$$

h_w is as given in 3-2-4/5.3.2.

5.3.4 Non-prismatic Members

Where the cross sectional properties of the member is not constant throughout the length of the girders or transverses, the above requirements will be specially considered with particular attention being paid to the shearing forces at the ends.

5.5 Center Girder

In general, a center girder is to be fitted, complying with 3-2-4/5.3; however, alternative arrangements that provide suitable support for docking will be considered.

5.7 Frames

The section modulus, SM , of each bottom frame to the chine or upper turn of bilge, in association with the plating to which it is attached, is not to be less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.80 \text{ for transverse frames clear of tanks}$$

$$= 1.00 \text{ for longitudinal frames clear of tanks, and in way of tanks}$$

$$= 1.00 \text{ for transverse frames in way of tanks}$$

$$s = \text{frame spacing in, m (ft)}$$

$$\ell = \text{unsupported span, in m (ft), Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by bulkheads, inner bottom or side shell, the length, } \ell, \text{ may be measured as permitted therein.}$$

h = vertical distance, in m (ft), from the middle of ℓ to the deck at side. In way of a deep tank, h is the greatest of the distances, in m (ft), from the middle of ℓ to a point located at two-thirds of the distance from the top of the tank to the top of the overflow, a point located above the top of the tank not less than $0.01L + 0.15$ m ($0.01L + 0.5$ ft) or 0.46 m (1.5 ft), whichever is greatest.

L is as defined in 3-1-1/3.

7 Fore-end Strengthening

7.1 General

For vessels 61 m (200 ft) in length and over, where the heavy weather ballast draft forward is less than $0.04L$, strengthening of the flat of bottom forward is to be in accordance with 3-2-4/7.3, 3-2-4/7.5, 3-2-4/7.7 and 3-2-2/3.5. Information on the heavy weather ballast draft forward used for the required fore-end strengthening is to be furnished to the master for guidance. The heavy weather ballast draft is also to be indicated on the shell expansion plan.

7.3 Extent of Strengthening

The flat of bottom forward is defined as being forward of the locations indicated in 3-2-4/Table 1. For intermediate values of C_b , the locations are to be obtained by interpolation. Aft of these locations, a suitable transition is to be provided between the increased scantlings and structural arrangements of the flat of bottom forward and the structure aft of the locations given in 3-2-4/Table 1.

TABLE 1
Location of Flat of Bottom Forward

C_b is the block coefficient at the summer load waterline, based on L , as defined in 3-1-1/3.

C_b	Location Forward of Amidships
0.6 or less	0.25L
0.8 or more	0.30L

7.5 Longitudinal Framing

When longitudinal framing is used for the bottom and inner bottom, longitudinals and side girders are to be continued as far forward as practicable at not more than their amidship spacing. The section modulus of flat of bottom longitudinals forward of the location indicated in 3-2-4/Table 1 is to be not less than required by the following equation, nor less than required by 3-2-4/5.7.

$$SM = 8.47(0.005L^2 - 1.3d_f^2)s\ell^2/d_f \text{ cm}^3$$

$$SM = 0.0044(0.005L^2 - 1.3d_f^2)s\ell^2/d_f \text{ in}^3$$

where

d_f = heavy weather ballast draft at the forward perpendicular, in m (ft)

L = length of vessel, as defined in 3-1-1/3

s = spacing of longitudinals, in m (ft)

ℓ = distance between floors, in m (ft)

The spacing of floors forward of 0.25L forward of amidships is to be not greater than that given in 3-2-4/Table 2, nor greater than the spacing amidships.

TABLE 2
Spacing of Floors

d_f	C_b	From 0.25L to 0.3L from amidships	Forward of 0.3L from amidships
0.02L or less	0.60 or less	3s	2s
	> 0.60	3s	3s
0.035L	all values	3s	3s
0.04L and more	all values	As required elsewhere in the Rules	

Notes:

d_f is the heavy weather ballast draft at the forward perpendicular and C_b is the block coefficient at the summer load waterline, based on L as defined in 3-1-1/3.

s is the spacing of the transverse side frames, or $s = 2.08L + 438$ (mm) [0.025L + 17.25 (in.)], where the side shell is longitudinally framed.

For values of d_f between 0.02L, 0.035L and 0.04L m (ft), the required floor spacing may be obtained by interpolation.

7.7 Transverse Framing

Where the heavy weather ballast draft forward is less than 0.04L, solid floors are to be fitted on every frame and additional full-depth and half-depth side girders are to be introduced so that the spacing of full-depth girders forward of the locations in 3-2-4/Table 1 does not exceed 2.13 m (7 ft) and so that the spacing of alternating half- and full-depth girders forward of the location in 3-2-4/Table 1 does not exceed 1.07 m (3.5 ft). Where the heavy weather ballast draft forward is 0.04L or more, the arrangement of solid floors and side girders may be in accordance with 3-2-4/1.7 and 3-2-4/1.5.

9 Higher-strength Materials

9.1 General

In general, applications of higher-strength materials for bottom structures are to meet the requirements of this section, but may be modified as permitted by the following paragraphs. Care is to be exercised to avoid the adoption of reduced thickness of material such as might be subject to damage during normal operation, and calculations are to be submitted to show adequate provision against buckling. Longitudinal framing members are to be of essentially the same material as the plating they support.

9.3 Inner-bottom Plating

Inner-bottom plating, where constructed of higher-strength material and where longitudinally framed, is to be not less in thickness than required by 3-2-4/1.13 or for tank top plating as modified by the following equation:

$$t_{hts} = [t_{ms} - C][(Q + 2\sqrt{Q})/3] + C$$

where

- t_{hts} = thickness of higher-strength material, in mm (in.)
- t_{ms} = thickness of mild steel, as required by 3-2-4/1.13, in mm (in.), increased where required for no ceiling
- C = 3 mm (0.12 in.) or 5 mm (0.20 in.) where the plating is required by 3-2-4/1.13 to be increased for no ceiling
- Q = as defined in 3-2-1/7.5

The thickness of inner-bottom plating, where transversely framed, will be specially considered.

Where cargo is handled by grabs, or similar mechanical appliances, the recommendations of 3-2-4/1.13 are applicable to t_{hts} .

9.5 Bottom and Inner-bottom Longitudinals

The section modulus of bottom and inner-bottom longitudinals, where constructed of higher-strength material and in association with the higher-strength plating to which they are attached, are to be determined as indicated in 3-2-4/1.9, except that the value may be reduced by the factor Q , as defined in 3-2-1/7.5.

9.7 Center Girders, Side Girders and Floors

Center girders, side girders and floors, where constructed of higher-strength materials, are generally to comply with the requirements of 3-2-4/1.3, 3-2-4/1.5 or 3-2-4/1.7, but may be modified as permitted by the following equation:

$$t_{hts} = [t_{ms} - C][(Q + 2\sqrt{Q})/3] + C$$

where

t_{hts} , t_{ms} and C are defined in 3-2-4/9.3.

Q is as defined in 3-2-1/7.5.

11 Machinery Space

11.1 General

Special attention is directed to arranging for the provision of plated through beams and such casing and pillar supports as are required to secure structural efficiency. All parts of the machinery, shafting, etc., are to be efficiently supported and the adjacent structure is to be adequately stiffened.

Consideration is to be given to the submittal of plans of the foundations for main propulsion units, reduction gears and thrust bearings and of the structure supporting those foundations to the machinery manufacturer for review. (See also 4-3-1/21.)

11.3 Engine Foundations

11.3.1 Single Bottom Vessels

In vessels with single bottoms, the engines are to be seated on thick plates laid across the top of deep floors or upon heavy foundation girders efficiently bracketed and stiffened. Intercostal plates are to be fitted between the floors beneath the lines of bolting to distribute the weight effectively through the bottom structure to the shell. Seat plates are to be of thickness and width appropriate to the holding-down bolts and are to be effectively attached to girders and intercostals.

11.3.2 Double Bottom Vessels

On vessels with double bottoms, the engines are to be seated directly upon thick inner-bottom plating or upon thick seat plates on top of heavy foundations arranged to distribute the weight effectively. Additional intercostal girders are to be fitted within the double bottom to ensure the satisfactory distribution of the weight and the rigidity of the structure.

11.5 Thrust Foundations

Thrust blocks are to be bolted to efficient foundations extending well beyond the thrust blocks and arranged to distribute the loads effectively into the adjacent structure. Extra intercostal girders, effectively attached, are to be fitted in way of the foundations, as may be required.

11.7 Shaft Stools and Auxiliary Foundations

Shaft stools and auxiliary foundations are to be of ample strength and stiffness in proportion to the weight supported.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **5 Side Frames, Webs, and Stringers**

1 General

1.1 Basic Considerations

Frames or webs and stringers are not to have less strength than is required for watertight bulkhead stiffeners, or girders, in the same location in association with heads to the bulkhead deck. In way of deep tanks, frames or webs and stringers are not to have less strength than is required for stiffeners or girders on deep-tank bulkheads. The calculated section modulus is based upon the intact sections being used. Where a hole is cut in the flange of any member or a large opening is made in the web of the member, the net section is to be used in determining the section modulus of the member in association with the plating to which it is attached.

1.3 End Connections

At the ends of unbracketed frames, both the web and the flange are to be welded to the supporting member. At bracketed end connections, continuity of strength is to be maintained at the connection to the bracket and at the connection of the bracket to the supporting member. Welding is to be in accordance with 3-2-16/Table 1. Where longitudinal frames are not continuous at bulkheads, end connections are to effectively develop their sectional area and resistance to bending. Where a structural member is terminated, structural continuity is to be maintained by suitable back-up structure, fitted in way of the end connection of frames, or the end connection is to be effectively extended by a bracket or flat bar to an adjacent beam, stiffener, etc.

3 Longitudinal Side Frames

3.1 Section Modulus

The section modulus, SM , of each longitudinal side frame above the chine or upper turn of bilge is to be not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.915$$

$$h = \text{vertical distance, in m (ft), from the frame to the freeboard deck at side, but not less than } 0.02L + 0.46 \text{ m (} 0.02L + 1.5 \text{ ft)}$$

- s = frame spacing, in m (ft)
 ℓ = straight-line unsupported span, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by bulkheads, the length, ℓ , may be measured as permitted therein.

5 Transverse Side Frames

5.1 Section Modulus

The section modulus, SM , of each transverse side frame other than tween deck frames above the chine or upper turn of bilge, in association with the plating to which the frame is attached, is not to be less than that obtained from the following equation. See 3-2-5/Figure 1, 3-2-5/Figure 2 and 3-2-5/Figure 3.

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- c = 0.915 for frames having no tween decks above
 = $0.90 + 5.8/\ell^3 (0.90 + 205/\ell^3)$ for frames having tween decks above
 s = frame spacing, in m (ft)
 ℓ = straight-line unsupported span, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by decks or inner bottoms, the length ℓ may be measured as permitted therein. Where tween decks are located above the frame, ℓ is to be taken as the length between the toes of the brackets, except where beam knees are fitted on alternate frames, ℓ is to be increased by one half the depth of the beam knees. ℓ is not to be taken less than 2.1 m (7.0 ft).
 h = on frames having no tween decks above, the vertical distance, in m (ft), from the mid length of the frame to the freeboard deck at side, but not less than $0.02L + 0.46 \text{ m} (0.02L + 1.5 \text{ ft})$.
 = on frames having tween decks above, the vertical distance, in m (ft), from the middle of ℓ to the load line or 0.4ℓ , whichever is greater, plus $bh_1/33 (bh_1/100)$.
 b = horizontal distance, in m (ft), from the outside of the frames to the first row of deck beam supports.
 h_1 = vertical distance, in m (ft), from the deck at the top of the frame to the bulkhead or freeboard deck plus the height of all cargo tween deck spaces above the bulkhead or freeboard deck plus one-half the height of all passenger spaces above the bulkhead or freeboard deck, or plus 2.44 m (8 ft), if that is greater. Where the cargo load differs from $715 \text{ kgf/m}^3 (45 \text{ lbf/ft}^3)$, h_1 is to be adjusted accordingly.

FIGURE 1
Transverse Side Frame

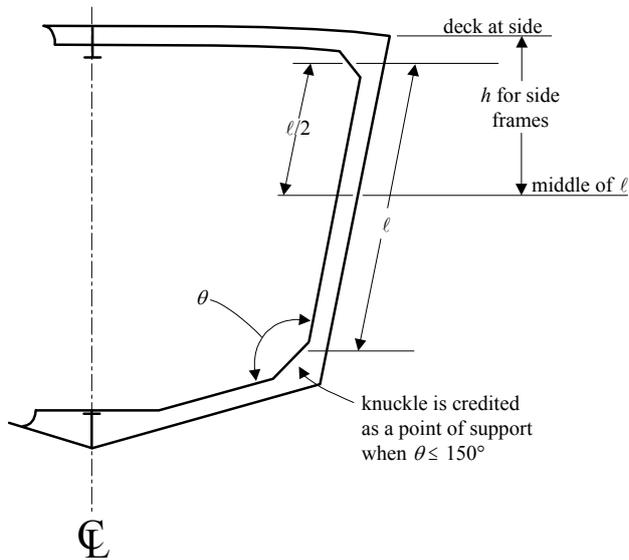


FIGURE 2
Transverse Side Frame

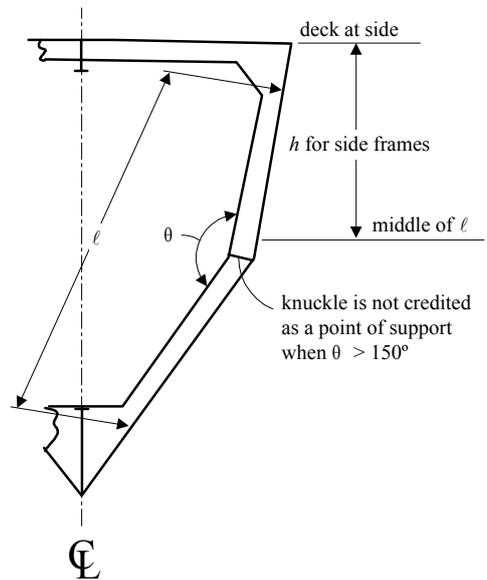
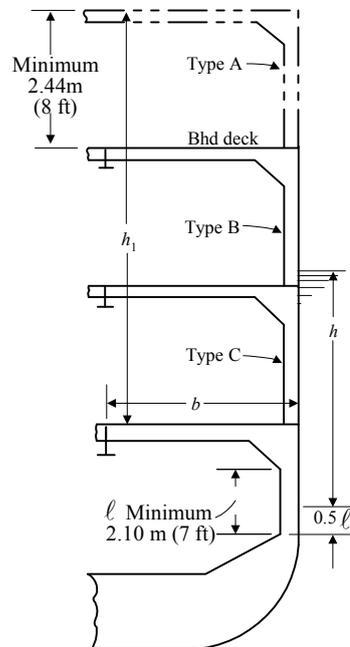


FIGURE 3
Hold and Tween Deck Frames



5.3 Tween-deck Frames

The section modulus, SM , of each transverse side frame above the chine or upper turn of bilge, in association with the plating to which the frame is attached, is not to be less than obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.90$$

$$h = 0.032L - 0.68 \text{ m (0.032L - 2.23 ft) type A frame}$$

$$= 0.049L - 0.81 \text{ m (0.049L - 2.66 ft) type B frame}$$

$$= 0.052L - 0.13 \text{ m (0.052L - 0.43 ft) type C frame}$$

$$= \text{in no case less than the vertical distance, in m (ft), from the mid-length of the frame to the freeboard deck, but not less than } 0.02L + 0.46 \text{ m (0.02L + 1.5 ft)}$$

$$s = \text{frame spacing, in m (ft)}$$

$$\ell = \text{tween deck height or unsupported length along frame, whichever is greater, in m (ft), not to be taken less than 2.13 m (7.0 ft). For frame types, see 3-2-5/Figure 3. Forward of } 0.125L, \text{ frames above the bulkhead or freeboard deck are to be type B frames.}$$

Longitudinal tween deck frames are to meet the requirements of 3-2-5/3. The section modulus of each longitudinal tween deck frame forward of $0.125L$ from the stem is to be not less than required by 3-2-5/5.1 for transverse frames in the same location, taking ℓ as the unsupported span along the frame length.

5.5 Peak Frames

5.5.1 General

For vessels 61 m (200 ft) or greater in length, peak frames are to be efficiently connected to deep floors of not less thickness than that obtained from 3-2-4/1.7 for floors in engine spaces. The floors are to extend as high as necessary to give lateral stiffness to the structure and are to be properly stiffened on their upper edges. Care is to be taken in arranging the framing and floors to assure no wide areas of unsupported plating adjacent to the stem. Breast hooks are to be arranged at rectangular intervals at and between the stringers above and below the waterline. In general, the frames above the lowest deck are to be as required by 3-2-5/5, but in vessels having large flare with unusually long frames, stringers and webs above the lowest deck or suitably increased frames may be required.

5.5.2 Section Modulus

For vessels 61 m (200 ft) or greater in length, the section modulus of each peak frame is to be not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 1.13 \text{ for forepeak frames}$$

$$= 0.90 \text{ for aftpeak frames}$$

- h = 0.110L – 1.990 m (0.110L – 6.53 ft) for forepeak frames
 = 0.062L – 1.122 m (0.062L – 3.68 ft) for aftpeak frames
 s = frame spacing, in m (ft)
 ℓ = straight line unsupported span, in m (ft), not to be taken less than 2.1 m (7.0 ft).
 L = length as defined in 3-1-1/3, but is not to be taken less than 30 m (98.5 ft)

7 Side Web Frames

7.1 Section Modulus

The section modulus, SM , of each side web frame supporting longitudinal framing or shell stringers above the chine or upper turn of bilge, in association with the plating to which the web frame is attached, is not to be less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- c = 0.915 aft of the forepeak
 = 1.13 in the forepeak of vessel 61 m (200 ft) or greater in length.
 s = frame spacing, in m (ft)
 ℓ = straight-line unsupported span, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by decks or inner bottoms, the length, ℓ , may be measured as permitted therein
 h = on frames having no tween decks above, the vertical distance, in m (ft), from the mid length of the frame to the freeboard deck at side, but not less than 0.02L + 0.46 m (0.02L + 1.5 ft).
 = on frames having tween decks above, the vertical distance, in m (ft), from the middle of ℓ to the load line or 0.5 ℓ , whichever is greater, plus $bh_1/45K$ ($bh_1/150K$).
 b = horizontal distance, in m (ft), from the outside of the frames to the first row of deck beams supports.
 h_1 = vertical distance, in m (ft), from the deck at the top of the web frame to the bulkhead or freeboard deck plus the height of all cargo tween deck spaces above the bulkhead or freeboard deck plus one-half the height of all passenger spaces above the bulkhead or freeboard deck, or plus 2.44 m (8 ft), if that is greater. Where the cargo load differs from 715 kgf/m³ (45 lbf/ft³), h_1 is to be adjusted accordingly.
 K = 1.0 where the deck is longitudinally framed and a deck transverse is fitted in way of each web frame.
 = the number of transverse frame spaces between web frames where the deck is transversely framed.

7.3 Tween-deck Web Frames

Tween-deck web frames are to be fitted below the bulkhead deck over the hold web frames, as may be required to provide continuity of transverse strength above the main web frames in holds and machinery space.

7.5 Proportions

The depth of each web frame is to be not less than 125ℓ mm (1.5 ℓ in.) or, unless effective compensation is provided for cutouts, 2.5 times the cutout for frame or longitudinal if greater. The thickness of the web of web frame or stringer is to be not less than 0.01 times the depth plus 3 mm (0.12 in.), ℓ is as defined in 3-2-5/7.1.

7.7 Tripping Brackets and Stiffeners

7.7.1 Stiffeners

Where the shell is longitudinally framed, stiffeners attached to the longitudinal frames and extending to the full depth of the web frame are to be fitted at least at alternate longitudinal frames. Other stiffening arrangements may be considered based on the structural stability of the web plates.

7.7.2 Tripping Brackets

Tripping brackets are to be fitted at intervals of about 3 m (10 ft) and near the change of section. Where the breadth of the flanges on either side of the web exceeds 200 mm (8 in.), tripping brackets are to be arranged to support the flange.

9 Recommendation for Vessels Subject to Impact

For vessels subject to impact loadings during routines, it is recommended that side frames with a section modulus 25% greater than that obtained above be provided.

11 Side Stringers

11.1 Section Modulus

The section modulus, SM , of each side stringer in association with the plating to which the side stringer is attached is not to be less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.915$$

= 1.13 in the forepeak of vessel 61 m (200 ft) or greater in length.

h = vertical distance, in m (ft), from the middle of s to the freeboard deck at side, but not less than $0.02L + 0.46$ m ($0.02L + 1.5$ ft).

= for stringers above the lowest deck or at a similar height in relation to the design draft, not less than given in 3-2-5/5.3 appropriate to the tween deck location.

= for stringers in the peaks of vessels 61 m (200 ft) or greater in length, not less than given in 3-2-5/5.5.

- s = sum of the half lengths, in m (ft), (on each side of the stringer) of the frames supported.
- ℓ = span, in m (ft), between web frames, or between web frame and bulkhead. Where brackets are fitted in accordance with 3-1-2/5.5 and are supported by transverse bulkheads, the length, ℓ , may be measured as permitted therein.

11.3 Proportions

Side stringers are to have a depth of not less than 0.125ℓ (1.5 in per ft of span ℓ) plus one-quarter of the depth of the slot for the frames, but need not exceed the depth of the web frames to which they are attached. In general, the depth is not to be less than 2.5 times the depth of the slots, or the slots are to be fitted with filler plates. The thickness of each stringer is to be not less than $0.014L + 7.2$ mm ($0.00017L + 0.28$ in.) where L is as defined in 3-1-1/3.

11.5 Tripping Brackets and Stiffeners

11.5.1 Stiffeners

Stiffeners attached to the frame and extending to the full depth of the stringer are to be fitted at least at alternate transverse frames. Other stiffening arrangements may be considered based on the structural stability of the web plates.

11.5.2 Tripping Brackets

The arrangements of tripping brackets are to be in accordance with 3-2-5/7.7.2.

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PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **6 Beams, Deck Girders, Deck Transverses, and Pillars**

1 Beams

1.1 Spacing

Beams may be fitted either transversely or longitudinally. Transverse beams, where provided, are to be fitted at each transverse side frame at the tops of tanks, tunnel tops, and bulkhead recesses. Elsewhere, these beams are not to be more than two frame spaces apart and those in different tiers are to be fitted on the same frames.

1.3 Section Modulus

The section modulus, SM , of each transverse or longitudinal beam, in association with the plating to which it is attached, is not to be less than that obtained from the following equations:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$\begin{aligned} c &= 1.00 \text{ for transverse or longitudinal beams at the tops of tank, with deep tank } h \\ &= 1/(1.709 - 0.651k) \text{ for longitudinal beams of strength decks and effective lower decks} \\ &= 0.60 \text{ for all other transverse beams} \\ &= 0.70 \text{ for all other longitudinal beams} \end{aligned}$$

$$k = SM_R Y / I_A$$

$$SM_R = \text{required hull-girder section modulus amidships from 3-2-1/3, in cm}^2\text{-m (in}^2\text{-ft)}$$

$$Y = \text{distance, in m (ft), from the neutral axis to the deck being considered, always taken as positive}$$

$$I_A = \text{hull girder moment of inertia of the vessel amidships, in cm}^2\text{-m}^2 \text{ (in}^2\text{-ft}^2)$$

The values of I_A and Y are to be those obtained using the area of the longitudinal beams given by the above equation.

- s = beam spacing, in m (ft)
- ℓ = unsupported span, in m (ft). At the tops of tanks and bulkhead recesses, the maximum span permissible between supports is 4.57 m (15 ft). Where brackets are fitted in accordance with 3-1-2/5.5, the length, ℓ , may be measured as permitted therein.
- h = height, in m (ft), as follows:
- = *for a deep tank top*, is the greatest of the following: two-thirds of the distance from the top of the tank to the top of the overflow, or
 - two-thirds of the distance from the top of the tank to the bulkhead deck or freeboard deck, or
 - the height to the load line, or
 - $0.01L + 0.15$ m ($0.01L + 0.5$ ft)
 - = *for a lower deck on which cargo or stores are carried*, the tween-deck height at side. Where the cargo weights differ from 7010 N/m^3 (715 kgf/m^3 , 45 lbf/ft^3), h is to be proportionately adjusted.
 - = *for an exposed deck on which cargo is carried*, 3.66 m (12 ft). Where it is intended to carry deck cargoes in excess of 25850 N/m^2 (2636 kgf/m^2 , 540 lbf/ft^2), this head is to be increased in proportion to the added loads which will be imposed on the structure.

Elsewhere, the value of h is obtained from the appropriate equation below, where L = length of the vessel, in m (ft), as defined in 3-1-1/3.

1.3.1 Exposed Freeboard Deck Having no Deck Below

$$h = 0.02L + 0.76 \text{ m}$$

$$h = 0.02L + 2.5 \text{ ft}$$

1.3.2 Exposed Freeboard Deck Having a Deck Below, Forecastle Deck, Superstructure Deck Forward of Amidships 0.5L

$$h = 0.02L + 0.46 \text{ m}$$

$$h = 0.02L + 1.5 \text{ ft}$$

1.3.3 Freeboard Deck within Superstructure, any Deck Below Freeboard Deck, Superstructure Deck Between 0.25L Forward of and 0.30L Aft of Amidships

$$h = 0.01L + 0.61 \text{ m}$$

$$h = 0.01L + 2.0 \text{ ft}$$

1.3.4 All Other First Tier Above Freeboard Deck Locations

$$h = 0.01L + 0.30 \text{ m}$$

$$h = 0.01L + 1.0 \text{ ft}$$

1.3.5 Second Tier Above Freeboard Deck; Deckhouse Top or Short Superstructure*

$$h = 0.01L + 0.15 \text{ m}$$

$$h = 0.01L + 0.5 \text{ ft}$$

* Where used only as weather covering, may be used as 3-2-6/1.3.6, but L need not be taken greater than 45.70 m (150 ft).

1.3.6 Third Tier Above Freeboard Deck Deckhouse Top or Short Superstructure*

$$h = 0.01L \text{ m}$$

$$h = 0.01L \text{ ft}$$

* Where used only as weather covering, may be used as 3-2-6/1.3.6, but L need not be taken greater than 45.70 m (150 ft).

1.5 Special Heavy Beams

Special reinforced beams are to be fitted under concentrated loads such as the ends of deckhouses, masts, winches, auxiliary machinery, etc. Beams at the heads of web frames are to be suitably increased in strength and stiffness.

1.6 Deck Fittings (2007)

1.6.1 General

The strength of supporting hull structures used for mooring operations and/or normal towing operations at bow, sides and stern are to comply with the requirements of this section.

Deck fittings for mooring and/or towing are to be located on longitudinals, beams and/or girders, which are part of the deck construction so as to facilitate efficient distribution of the mooring and/or towing load. The same attention is to be paid to recessed bits, if fitted, of their structural arrangements and strength of supporting structures.

1.6.2 Design Loads

Unless greater safe working load (SWL) of deck fittings is specified by the applicant, the minimum design load to be used is the greater values obtained from 3-2-6/1.6.2(a) or 3-2-6/1.6.2(b), whichever is applicable:

1.6.2(a) Mooring Operations. The minimum design load for deck fittings for mooring operations is the applicable value obtained from 3-2-6/1.6.2(a)i) or 3-2-6/1.6.2(a)ii):

i) *Mooring Line Force.* 1.25 times the breaking strength of the mooring line according to 3-5-1/Table 2 for each equipment number (EN). EN is the corresponding value used for determination of the vessel's equipment. (See Note)

Notes:

- 1 Side projected area including maximum stacks of deck cargoes is to be taken into account for assessment of lateral wind forces, arrangements of tug boats and selection of mooring lines.
- 2 Where the tabular breaking strength exceeds 490 kN (50,000 kgf, 110,200 lbf), the breaking strength of individual mooring line may be reduced with corresponding increase of number of the mooring lines, provided that the total breaking load of all lines aboard the vessel is not less than the total loads as specified in 3-5-1/Table 2. The number of mooring lines is not less than 6 and no one line is to have a strength less than 490 kN. (See also 3-5-1/17 and 3-5-1/19.7)

ii) *Mooring Winch Force.* The design load applied to supporting hull structures for winches, etc. is to be 1.25 times the intended maximum brake holding load and, for capstans, 1.25 times the maximum hauling-in force.

1.6.2(b) Towing Operations. The minimum design load for deck fittings for towing operations is the applicable value obtained from 3-2-6/1.6.2(b)i) or 3-2-6/1.6.2(b)ii):

i) *Normal towing operations (e.g., harbor/maneuvering).* 1.25 times the intended maximum towing load (e.g., static bollard pull) as indicated on the towing and mooring arrangements plan.

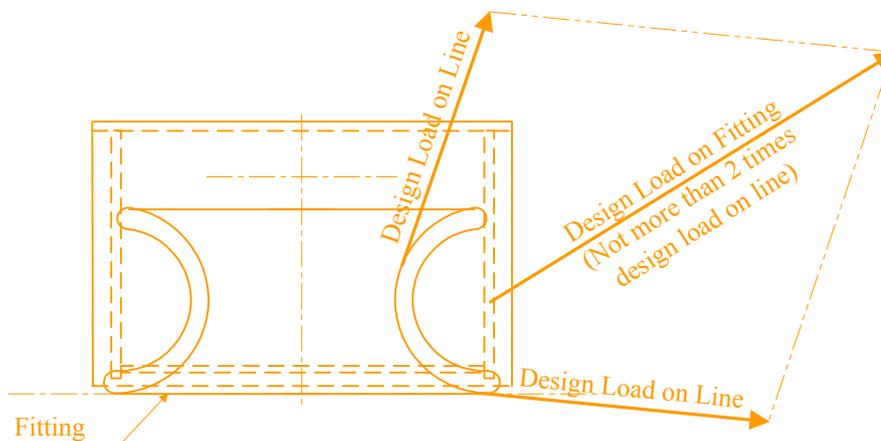
- ii) *Other towing service (e.g., escort).* The nominal breaking strength of the tow line according to the 3-5-1/Table 2 for each equipment number (EN). EN is the corresponding value used for determination of the vessel's equipment. (See Note)

Note: Side projected area including maximum stacks of deck cargoes is to be taken into account for assessment of lateral wind forces, arrangements of tug boats and selection of mooring lines.

1.6.2(c) *Application of Design Loads.* The design load is to be applied through the mooring line or tow line, whichever is applicable, according to the arrangement shown on the mooring and towing arrangements plan.

The method of application of the design load to the supporting hull structures is to be taken into account such that the total load need not be more than twice the design load specified in 3-2-6/1.6.2(b) above, i.e., no more than one turn of one line (see 3-2-6/Figure 1 below).

FIGURE 1
Application of Design Loads (2007)



When a specific SWL is applied for a deck fitting at the request of the applicant, by which the design load will be greater than the above minimum values, the strength of the supporting hull structures is to be designed using this specific design load.

1.6.3 Supporting Structures

1.6.3(a) *Arrangement.* The reinforced structural members (e.g., carling) are to be arranged beneath the deck where deck fittings are located and effectively distribute the loads from deck fittings for any variation of direction (horizontally and vertically).

1.6.3(b) *Line Forces.* The acting point of the mooring and/or towing force on deck fittings is to be taken at the attachment point of a mooring line or a towing line, as applicable.

1.6.3(c) *Allowable Stresses.* Allowable stresses under the design load conditions as specified in 3-2-6/1.6.2 are as follows:

- Normal stress: 100% of the specified minimum yield point of the material;
- Shearing stress: 60% of the specified minimum yield point of the material;

No stress concentration factors being taken into account. Normal stress is the sum of bending stress and axial stress with the corresponding shearing stress acting perpendicular to the normal stress.

1.6.4 Scantlings

1.6.4(a) Net Scantlings. The net minimum scantlings of the supporting hull structure are to comply with the requirements given in 3-2-6/1.6.3. The net thicknesses, t_{net} , are the member thicknesses necessary to obtain the above required minimum net scantlings. The required gross thicknesses are obtained by adding the total corrosion additions, t_c , given in 3-2-6/1.6.4(b), to t_{net} .

1.6.4(b) Corrosion Addition. The total corrosion addition, t_c , in mm (in.), for both sides of the hull supporting structure is not to be less than the following values:

- Ships covered by Common Structural Rules (CSR) for bulk carriers and CSR for double hull oil tankers: Total corrosion additions defined in these Rules
- Other ships: 2.0 (0.08)

1.7 Container Loading

Where it is intended to carry containers, the exact locations of the container pads and the maximum total static load on the pads are to be indicated on the plans. Where the pads are not in line with the supporting structures, headers are to be provided to transmit the loads to these members.

Each member intended to support containers is to have a section modulus, SM , in cm^3 (in^3), not less than that obtained from the following equation:

$$SM = M/f$$

where

M = maximum bending moment due to maximum static container loading in kN-cm (kgf-cm, Ltf-in)

f = permissible maximum bending stress, as given in 3-2-6/Table 1.

In determining the maximum bending moment, members may be considered fixed-ended, provided the member is continuous over the adjacent spans or is effectively attached to a bulkhead stiffener or frame or has suitable end connections. Where this is not the case, the member is to be considered simply-supported. Where weather deck containers are supported by pedestals, the section modulus required by 3-2-6/1.3, with h equal to the distance between the deck and the underside of the container, but not greater than 50% of the value given in 3-2-1/1.3.1 through 3-2-1/1.3.6, is to be added to the above required section modulus.

TABLE 1
Values of f

	kN/cm^2	kgf/cm^2	Ltf/in^2
Effective longitudinal members	12.36	1262	8
Transverse members and longitudinal members inside the line of openings	13.90	1420	9
<p>The net sectional area of the web of the member in cm^2 (in^2), including effective brackets, where applicable, is to be not less than that obtained from the following equation:</p> $A = F/q$ <p>where</p> <p>F = shearing force at the point under consideration, kN (kgf, Ltf)</p> <p>q = allowable average shear stress in the web, not to exceed 10.35 kN/cm^2 (1057 kgf/cm^2, 6.7 Ltf/in^2)</p>			

3 Deck Girders and Deck Transverses

3.1 General

Girders and transverses are to be fitted as required to support beams and longitudinals. Additional girders are to be fitted as required under masts, king posts, deck machinery or other heavy concentrated loads.

3.3 Deck Girders and Transverses Clear of Tanks

Section modulus, SM , of each longitudinal deck girder and deck transverse clear of tanks is not to be less than that obtained from the following equation:

$$SM = 7.8cbh\ell^2 \text{ cm}^3$$

$$SM = 0.0041cbh\ell^2 \text{ in}^3$$

where

$$c = 0.60$$

$$b = \text{mean breadth of area of deck supported (for girders), or spacing of deck transverses (for transverses), in m (ft)}$$

$$h = \text{height, in m (ft), as required by 3-2-6/1.3 for the beams supported}$$

$$\ell = \text{unsupported span, in m (ft). Where brackets are fitted at bulkhead supports, in accordance with 3-1-2/5.5, the length, } \ell, \text{ may be measured as permitted therein.}$$

3.5 Proportions

The minimum depth of a deck girder or transverse supporting member is to be 58.3ℓ mm (0.7ℓ in.), where ℓ is as defined in 3-2-6/3.3; the depth is also not to be less than 2.5 times the cutout for the beam or longitudinal unless effective compensation is provided for the cutouts. The minimum thickness is to be 1 mm per 100 millimeters (0.01 in. per inch) of depth plus 4 mm (0.16 in.).

3.7 Tripping Brackets and Stiffeners

Tripping brackets are to be fitted on girders and transverses at a spacing of about 3 m (10 ft). Stiffeners are to be fitted as may be required.

3.9 Deck Girders and Transverses in Tanks

The requirements for deck girders or transverse supporting members in tanks may be obtained in the same manner as given in 3-2-6/3.3, 3-2-6/3.5 and 3-2-6/3.7, except that c is equal to 0.915. The minimum depth of a girder or transverse supporting member is to be 83.3ℓ mm (1.0ℓ in.), where ℓ is as defined in 3-2-6/3.3.

3.11 Hatch Side Girders

Scantlings for hatch side girders supporting athwartship shifting beams or supporting hatch covers are to be obtained in the same manner as deck girders (3-2-6/3.3 and 3-2-6/3.9). Such girders along lower deck hatches under trunks in which covers are omitted are to be increased in proportion to the extra load which may be required to be carried due to the loading up into the trunks. The structure on which the hatch covers are seated is to be effectively supported.

Where deep coamings are fitted above decks, such as at weather decks, the girder below deck may be modified so as to obtain a section modulus in cm^3 (in^3), when taken in conjunction with the coaming up to and including the horizontal coaming stiffener, of not less than 35% more than the required girder value, as derived from 3-2-6/3.3. Where hatch side girders are not continuous under deck beyond the hatchways to the bulkheads, brackets extending for at least two frame spaces beyond the ends of the hatchways are to be fitted.

Where hatch side girders are continuous beyond the hatchways, care is to be taken in proportioning their scantlings beyond the hatchway. Gusset plates are to be fitted at hatchway corners, arranged so as to tie effectively the flanges of the side coamings and extension pieces or continuous girders and the hatch-end beam flanges both beyond and in the hatchway.

3.13 Container Loading

Where it is intended to carry containers, the structure is to comply with 3-2-6/1.7.

3.15 End Attachments

The ends of deck girders and transverses are to be effectively attached by welding.

3.17 Hatch-end Beams

Each hatch-end beam, similar to that shown in 3-2-6/Figure 2, which is supported by a centerline pillar without a pillar at the corner of the hatchway, is to have a section modulus, SM , not less than that obtained from the following equations:

3.17.1 Where Deck Hatch-side Girders are Fitted Fore and Aft Beyond the Hatchways

$$SM = K(AB + CD)h\ell \text{ cm}^3$$

$$SM = 0.000527K(AB + CD)h\ell \text{ in}^3$$

3.17.2 Where Girders are not Fitted on the Line of the Hatch Side Beyond the Hatchway

$$SM = KABh\ell \text{ cm}^3$$

$$SM = 0.000527KABh\ell \text{ in}^3$$

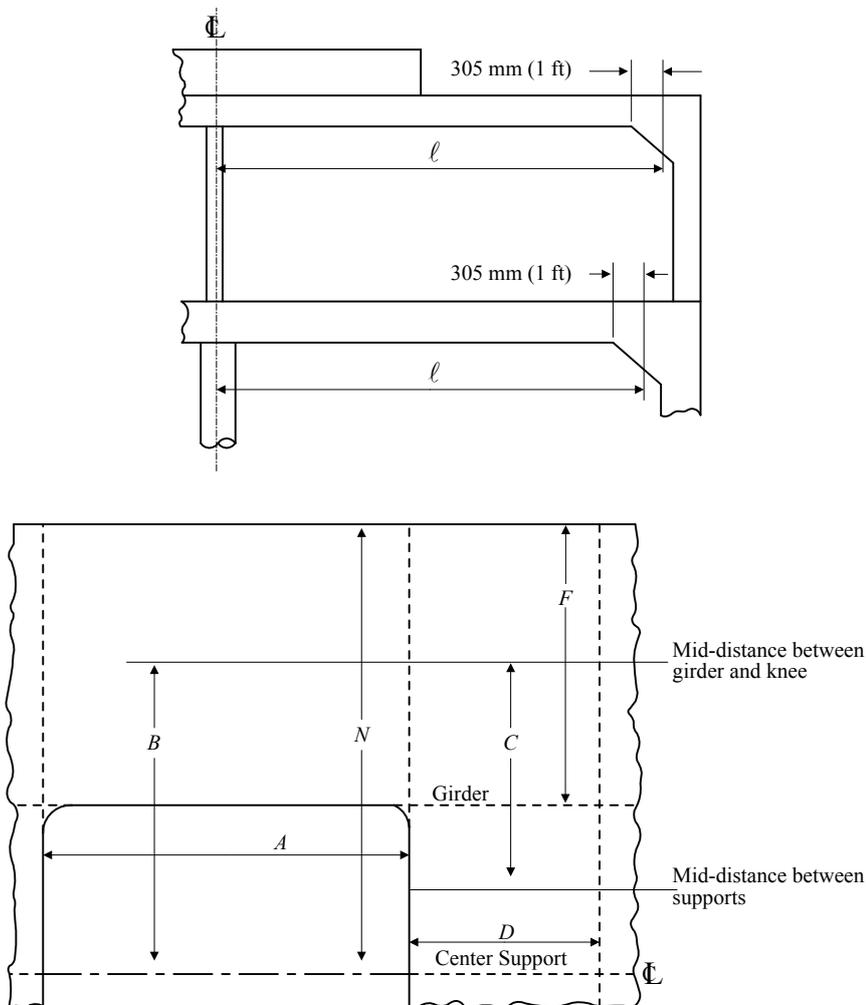
where

- A = length of the hatchway, in m (ft)
- B = distance from the centerline to the midpoint between the hatch side and the line of the toes of the beam knees, in m (ft)
- C = distance from a point midway between the centerline and the line of the hatch side to the midpoint between the hatch side and the line of the toes of the beam knees, in m (ft). Where no girder is fitted on the centerline beyond the hatchway, C is equal to B
- D = distance from the hatch-end beam to the adjacent hold bulkhead, in m (ft)
- h = height for the beams of the deck under consideration, as given in 3-2-6/1.3, in m (ft)
- ℓ = distance from the toe of the beam knee to the centerline plus 0.305 m (1 ft), in m (ft)
- K = $2.20 + 1.29(F/N)$ when $F/N \leq 0.6$
- K = $4.28 - 2.17(F/N)$ when $F/N > 0.6$
- N = one-half the breadth of the vessel in way of the hatch-end beam
- F = distance from the side of the vessel to the hatch-side girder

Weather deck hatch-end beams which have deep coamings above deck for the width of the hatch may have the flange area reduced from a point well within the line of the hatch side girder to approximately 50% of the required area at centerline; in such cases, it is recommended that athwartships brackets be fitted above deck at the ends of the hatch-end coaming.

Brackets at the end of hatch-end beams are to be generally as described in 3-1-2/5.5. Where brackets are not fitted, the length, ℓ , is to be measured to the side of the vessel and the face plates or flanges on the beams are to be attached to the shell by heavy horizontal brackets extending to the adjacent frame.

FIGURE 2
Hatch-end Beams



5 Stanchions and Pillars

5.1 General

Supports under pillars are to be sufficient strength to distribute the loads effectively. Tween-deck pillars are to be arranged directly above those below, or effective means are to be provided for transmitting their loads to supports below. Tripping brackets are to be fitted on members in way of pillars, both when the pillar is over and under the member.

5.3 Permissible Load

The permissible load a pillar can carry is to be equal to or greater than the pillar load, W , as determined in 3-2-6/5.5. The permissible load may be obtained from the following equation:

$$W_a = (k - n\ell/r)A$$

where

- W_a = load in kN (tf, Ltf)
- k = 12.09 (1.232, 7.83)
- n = 0.0444 (0.00452, 0.345)
- ℓ = the unsupported length of the pillar, in cm (ft)
- r = least radius of gyration of pillar, in cm (in.)
- A = area of pillar, in cm² (in²)

5.5 Calculated Load

The load on a pillar is to be obtained from the following equation:

$$W = nbhs$$

where

- W = load, in kN (tf, Ltf)
- n = 7.04 (0.715, 0.02)
- b = mean breadth, in m (ft), of area supported
- h = height, in m (ft), above the deck supported, as defined below
- s = mean length, in m (ft), of area supported

For a pillar below an exposed deck on which cargo is carried, h is the distance from the deck supported to a point 3.66 m (12 ft) above the exposed deck. Where it is intended to carry deck cargoes in excess of 2636 kilograms per square meter (540 pounds per square foot), this head is to be increased in proportion to the added loads which will be imposed on the structure.

For a pillar below the freeboard deck, h is to be measured to a point not less than $0.02L + 0.76$ m ($0.02L + 2.5$ ft) above the freeboard deck.

For a pillar below the superstructure deck, h is to be measured to a point not less than $0.02L + 0.46$ m ($0.02L + 1.5$ ft) above the superstructure deck.

The height, h , for any pillar is not to be less than the given height in 3-2-6/1.3 for the beams at the top of the pillar plus the sum of the heights given in the same paragraphs for the beams of all complete cargo decks and one-half the heights given for all partial superstructure decks above.

L is the length of vessel, in m (ft), as defined in 3-1-1/3.

5.7 Stanchions in Double Bottoms and Under Tank Tops

Stanchions in double bottoms and under the tops of deep tanks are to be solid in cross section. Stanchions under the tops of deep tanks are not to be less than required by 3-2-6/5.3 and 3-2-6/5.5, nor are they to have less section area than $cW \text{ cm}^2$ (in^2) where W is to be obtained from the following equation:

$$W = nbhs$$

where

W = load, in kN (tf, Ltf)

n = 10.5 (1.07, 0.03)

b = breadth, in m (ft), of the area of the top of the tank supported by the stanchion

h = height, in m (ft), as required by 3-2-6/1.3, for the tank-top beams

s = length, in m (ft), of the area of the top of the tank supported by the stanchion

c = 0.1035 (1.015, 0.16)

5.9 Bulkheads

Bulkheads supporting girders or bulkheads fitted in lieu of girders are to be stiffened to provide supports not less effective than required for pillars.

5.11 Attachments

Wide-spaced tubular or solid pillars are to bear solidly at head and heel and are to be attached by welding, properly proportioned to the size of the pillar. The attachments of stanchions or pillars under bulkhead recesses, tunnel tops or deep-tank tops which may be subjected to tension loads are to be specially developed to provide sufficient welding to withstand the tension load.

7 Higher-strength Materials

7.1 General

In general, applications of higher-strength materials for deck beams, girders and transverses are to meet the requirements of this section, but may be modified as permitted by the following paragraph.

Calculations are to be submitted to show adequate provision against buckling. Longitudinal members are to be of essentially the same material as the plating they support.

7.3 Beams, Girders and Transverses of Higher-strength Materials

Each beam, girder and transverse of higher-strength material, in association with the higher-strength plating to which it is attached, is generally to comply with the requirements of the appropriate preceding paragraphs of this section and is to have a section modulus, SM_{hts} , not less than obtained from the following equation:

$$SM_{hts} = SM(Q)$$

where

SM = required section modulus in ordinary-strength material, as determined elsewhere in this section.

Q = as defined in 3-2-1/7.5

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **7 Watertight Bulkheads and Doors**

1 General

All vessels having lengths, L , equal to or exceeding 15 m (50 ft) are to be provided with watertight bulkheads in accordance with this section. The plans submitted are to clearly show the location and extent of each watertight bulkhead. Watertight bulkheads constructed in accordance with the Rules will be recorded in the *Record* as **WT** (watertight), the symbols being prefixed in each case by the number of such bulkheads.

1.1 Openings and Penetrations (2006)

The number of openings in watertight subdivisions is to be kept to a minimum, compatible with the design and proper working of the vessel. Where penetrations of watertight bulkheads and internal deck are necessary for access, piping, ventilation, electrical cables, etc., arrangements are to be made to maintain the watertight integrity. Relaxation in the watertightness of openings above the freeboard deck may be considered, provided it is demonstrated that any progressive flooding can be easily controlled and that the safety of the vessel is not impaired.

Ventilation penetrations through watertight subdivision bulkheads are to be avoided. Where penetrations are unavoidable, the ventilation ducting is to satisfy watertight bulkhead requirements or watertight closing appliances are to be installed at the bulkhead penetrations. For ventilation penetrations below the bulkhead deck or below damage equilibrium waterlines, the closing appliances are to be operable from the bridge. Otherwise, local, manual controls may be provided.

3 Arrangement of Watertight Bulkheads

3.1 Collision Bulkheads

3.1.1 General

A collision bulkhead is to be fitted on all vessels. It is to be intact, that is, without openings, except as permitted in 4-4-1/9.11. It is to extend, preferably in one plane, to the freeboard deck. In the case of vessels having long superstructures at the fore end, it is to be extended weathertight to the superstructure deck. The extension need not be fitted directly over the bulkheads below, provided the location of the extension meets the following requirements and the part of the deck which forms the step is made effectively weathertight.

On vessels with bow-doors, that part of their sloping loading ramps that form part of the extension of a collision bulkhead and are more than 2.3 m (7.5 ft) above the freeboard deck may extend forward of the limit below.

Collision bulkhead requirements for passenger vessels are as indicated in the *ABS Guide for Building and Classing Passenger Vessels (Passenger Vessel Guide)*.

3.1.2 Location

The collision bulkhead is to be located at any point not less than $0.05L_f$ abaft the reference point. At no point on vessels having 500 or more gross tonnage, except as specially permitted, is it to be further than $0.08L_f$ from the reference point.

3.1.3 Reference Point

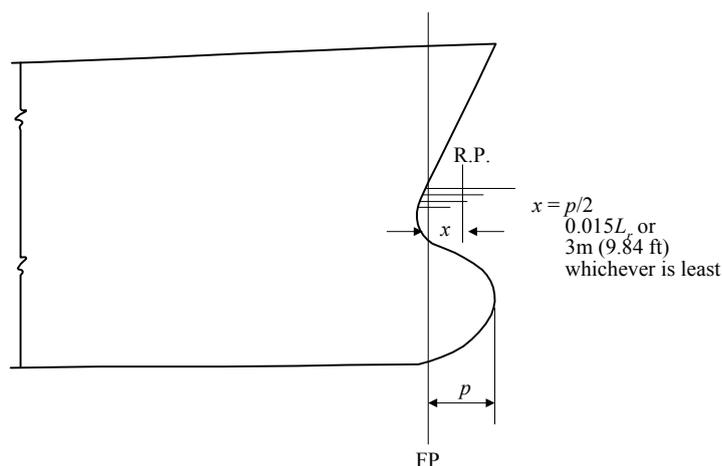
The reference point in determining the location of the collision bulkhead is the forward end of L_f , except that in the case of vessels having any part of the underwater body, such as the bulbous bow, extending forward of the forward end of L_f , the required distances are to be measured from a reference point located a distance forward of the forward end of L_f . This distance, x is the lesser of the following (see 3-2-7/Figure 1):

- i) Half the distance between the forward end of L_f and the extreme forward end of the extension, $p/2$ or
- ii) $0.015L_f$
- iii) 3 m (9.84 ft)

where L_f is as defined in 3-1-1/3.3.

The forward end of L_f is to coincide with the fore side of the stem on the waterline at which L_f is measured.

FIGURE 1
Reference Point of Vessels with Bulbous Bow



3.3 Engine Room

The engine room is to be enclosed by watertight bulkheads extending to the freeboard deck.

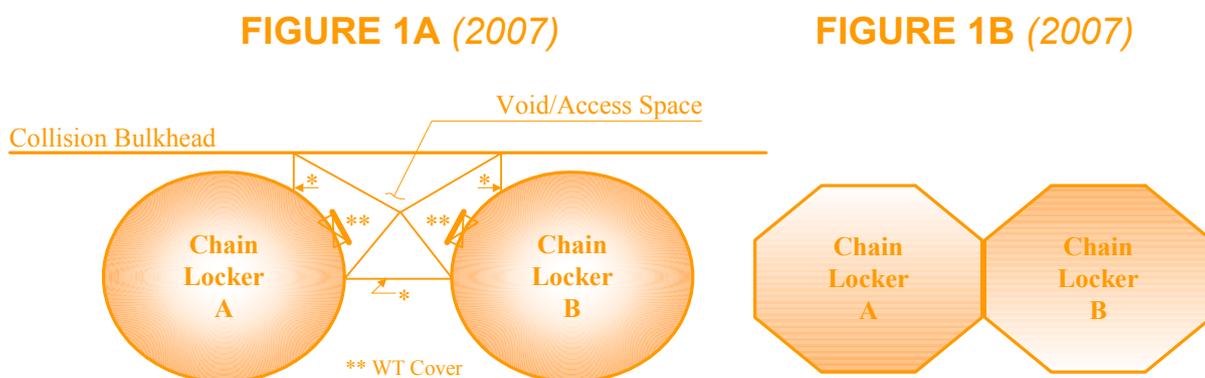
3.5 Chain Lockers (2007)

For vessels with freeboard length, L_f (as defined in 3-1-1/3.3) greater than 24 meters (79 feet), chain lockers and chain pipes are to be made watertight up to the weather deck. The arrangements are to be such that accidental flooding of the chain locker cannot result in damage to auxiliaries or equipment necessary for the proper operation of the vessel nor in successive flooding into other spaces. Bulkheads between separate chain lockers not forming a part of subdivision bulkhead (* see 3-2-7/Figure 1A below), or bulkheads which form a common boundary of chain lockers (see 3-2-7/Figure 1B below), need not be watertight.

Where means of access into chain lockers are provided, they are to be closed by a substantial cover secured by closely spaced bolts. Doors are not permitted.

For closure of chain pipes, see 3-2-12/23.7.

The arrangements on vessels that are not subject to the International Convention on Load Lines or its Protocol may be specially considered.



3.7 Hold Bulkheads (1 July 1998)

In addition to the above required bulkheads, for vessels of applicable type and size, the number and arrangement of hold bulkheads are to satisfy the subdivision and damage requirements in 3-3-1/3.3. Review procedures for this requirement are indicated in 3-3-1/5.

5 Construction of Watertight Bulkheads

5.1 Plating (1998)

Watertight bulkhead plating thickness is to be obtained from the following equation:

$$t = sk\sqrt{qh}/c + 1.5 \text{ mm} \quad \text{but not less than 6 mm or } s/200 + 2.5 \text{ mm, whichever is greater}$$

$$t = sk\sqrt{qh}/c + 0.06 \text{ in.} \quad \text{but not less than 0.24 in. or } s/200 + 0.10 \text{ in., whichever is greater}$$

where

$$\begin{aligned} t &= \text{thickness, in mm (in.)} \\ s &= \text{spacing of stiffeners, in mm (in.)} \\ k &= (3.075\sqrt{\alpha} - 2.077)/(\alpha + 0.272) \quad (1 \leq \alpha \leq 2) \\ &= 1.0 \quad (\alpha > 2) \end{aligned}$$

- α = aspect ratio of the panel (longer edge/shorter edge)
- q = $235/Y$ N/mm² ($24/Y$ kgf/mm², $34,000/Y$ psi)
- Y = specified minimum yield point or yield strength, in N/mm² (kgf/mm², psi), as defined in 2-1-1/13, for the higher strength material or 72% of the specified minimum tensile strength, whichever is less
- h = distance from the lower edge of the plate to the deepest equilibrium waterline in the one compartment damaged condition, in m (ft)
- For passenger vessels, h is to be taken as not less than the distance to the margin line.
 - For cargo vessels, h is to be not less than the distance to the bulkhead deck at center unless a deck lower than the uppermost continuous deck is designated as the freeboard deck, as allowed in 3-1-1/13.1, in which case, h is to be not less than the distance to the designated freeboard deck at center.
- c = 254 (460) for collision bulkhead
 = 290 (525) for other watertight bulkheads

For vessels under 30.5 m (100 ft) in length, the following deductions may be made to the thicknesses obtained from the above equation for mild steel only.

L meters	Deduction mm	L feet	Deduction in.
24.40 to 30.50	0.25	80 to 100	0.01
21.35 to 24.40	0.50	70 to 80	0.02
18.30 to 21.35	0.75	60 to 70	0.03
Under 18.30	1.00	Under 60	0.04

In general, main non-tight transverse strength bulkhead plating is to be similar to that required for watertight bulkheads. Other non-tight strength bulkheads plating is to be not less than $s/150$, or 4 mm (0.16 in.), whichever is greater. The section modulus of non-watertight bulkhead stiffeners is to be not less than one-half of that required by 3-2-7/5.3.

5.3 Stiffeners (1998)

The section modulus, SM of each bulkhead stiffener, in association with the plating to which it is attached, is to be not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- c = 0.30 for a stiffener with effective brackets at both ends of its span. An effective bracket for the application of this value of c is to have scantlings not less than shown in 3-1-2/Table 3 and is to extend onto the stiffener for a distance at least one-eighth of the length, ℓ , of the stiffener.
- = 0.43 for a stiffener with an effective bracket at one end and a clip connection or horizontal girder at the other end. An effective bracket for the application of this value of c is to have scantlings not less than shown in 3-1-2/Table 3 and is to extend onto the stiffener for a distance at least one-eighth of the length, ℓ , of the stiffener.

- c = 0.56 for a stiffener with clip connections at both ends or a clip connection at one end and a horizontal girder at the other end.
- = 0.60 for a stiffener between horizontal girders or for a stiffener with no end attachments.
- h = distance from the middle of ℓ to the deepest equilibrium waterline in the one compartment damaged condition, in m (ft)
- For passenger vessels, h is to be taken as not less than the distance to the margin line.
 - For cargo vessels, h is to be taken as not less than the distance to the bulkhead deck at center unless a deck lower than the uppermost continuous deck is designated as the freeboard deck, as allowed in 3-1-1/13.1, in which case, h is to be not less than the distance to the designated freeboard deck at center.
 - For all vessels, where this distance is less than 6.10 m (20 ft), h is to be taken as 0.8 times the distance plus 1.22 m (4 ft).
- s = spacing of stiffeners, in m (ft)
- ℓ = distance, in m (ft), between the heels of the end attachments. Where horizontal girders are fitted, ℓ is the distance from the heel of the end attachment to the first girder, or the distance between the horizontal girders.

In vessels under 46 meters (150 ft) in length, the above values for c may be 0.29, 0.38, 0.46 and 0.58, respectively, and h may be taken as the distance in meters or in feet from the middle of ℓ to the bulkhead deck at center in every case. For vessels between 46 and 65.5 meters (150 and 215 feet), intermediate values for c may be obtained by interpolation.

The section modulus of stiffeners on collision bulkheads is to be increased by 25% over the section modulus of stiffeners on ordinary watertight bulkheads.

5.5 Girders and Webs (1998)

Each horizontal girder or vertical web supporting bulkhead stiffeners is to have a section modulus, SM , not less than that obtained from the following equation:

$$SM = 4.74chs\ell^2 \text{ cm}^3$$

$$SM = 0.0025chs\ell^2 \text{ in}^3$$

where

- c = 1.0
- h = vertical distance, in m (ft), to the deepest equilibrium waterline in the one compartment damaged condition from the middle of s in the case of a horizontal girder or from the middle of ℓ in the case of a vertical web.
- For passenger vessels, h is to be taken as not less than the distance to the margin line.
 - For cargo vessels, h is to be not less than the distance to the bulkhead deck at center unless a deck lower than the uppermost continuous deck is designated as the freeboard deck, as allowed in 3-1-1/13.1, in which case, h is to be not less than the distance to the designated freeboard deck at center.
 - For all vessels, where this distance is less than 6.10 m (20 ft), h is to be taken as 0.8 times the distance plus 1.22 m (4 ft).

- s = sum of half lengths, in m (ft), (on each side of the girder or web) of the stiffeners supported by the girder or web
- ℓ = unsupported span of girder or web, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5, the length, ℓ , may be measured as permitted therein.

The required section modulus of girders or webs on collision bulkheads are to be increased by 25% over the required section modulus of girders or webs on ordinary bulkheads. The depth of a girder or web is not to be less than twice the depth of the cutout unless effective compensation is provided for stiffener cutouts. Tripping brackets are to be fitted at intervals of about 3 m (10 ft), and stiffeners are to be fitted as may be required.

Lower brackets to inner bottoms are to extend over the floor adjacent to the bulkhead. Where stiffeners cross horizontal girders, they are to be effectively attached.

5.7 Corrugated Bulkheads

5.7.1 Plating

The plating of corrugated bulkheads is to be of the thickness required by 3-2-7/5.1 with the following modification. The spacing to be used is the greater of dimensions a or c , as indicated in 3-2-7/Figure 2. The angle ϕ is to be 45 degrees or more.

5.7.2 Stiffeners

The section modulus, SM , for a corrugated bulkhead is to be not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

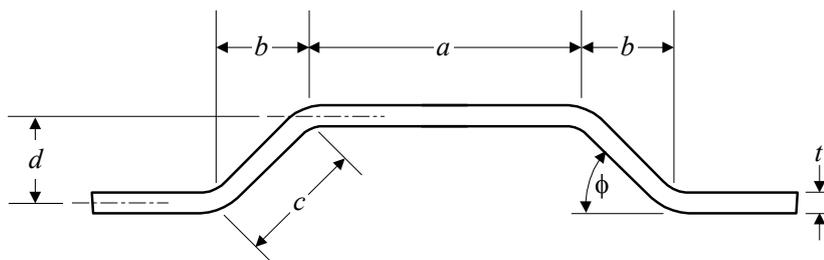
where

- ℓ = distance between supporting members, in m (ft)
- s = $a + b$, where a and b are as defined in 3-2-7/Figure 2, in m (ft)
- c = 0.56
- h = as defined in 3-2-7/5.3

The developed section modulus, SM , may be obtained from the following equation, where a , t , and d are as indicated in 3-2-7/Figure 2.

$$SM = td^2/6 + (adt/2)$$

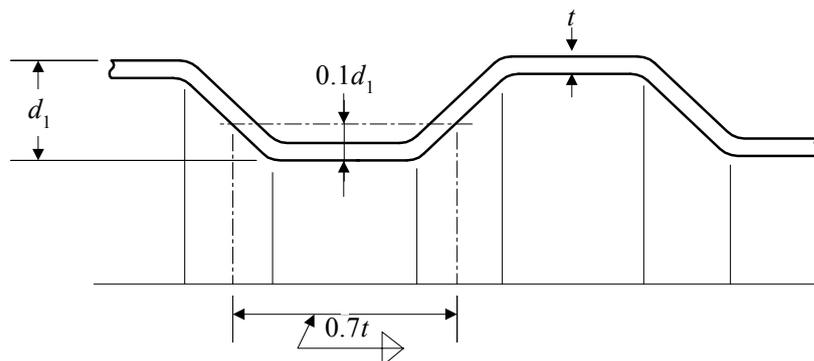
FIGURE 2
Corrugated Bulkhead



5.7.3 End Connections

The structural arrangements and size of welding at the ends of corrugations are to be designed to develop the required strength of corrugation stiffeners. Joints within 10% of the depth of corrugation from the outer surface of corrugation, d_1 , are to have double continuous welds with fillet size, w , not less than 0.7 times the thickness of the bulkhead plating or penetration welds of equal strength (3-2-7/Figure 3). See also 3-2-16/3.

FIGURE 3
Corrugated Bulkhead End Connections



7 Watertight Doors

7.1 Vessels Requiring Subdivision and Damage Stability

7.1.1 Doors Used While at Sea (2002)

Doors which are used while at sea are to be sliding watertight doors capable of being remotely closed from the bridge and are also to be operable locally from each side of the bulkhead. Indicators are to be provided at the control position showing whether the doors are open or closed, and an audible alarm is to be provided which is to sound whenever the door is closed remotely by power. The power-operated doors, control systems and indicators are to be functional in the event of main power failure. Particular attention is to be paid to minimize the effect of control system failure. Each power-operated sliding watertight door is to be provided with an individual hand-operated mechanism. It is to be possible to open and close the door by hand at the door itself from each side. For vessels less than 80 m (262.4 ft) in length (L_f as defined in 3-1-1/3.3), hinged quick-acting doors, operable from both sides of the door, are permitted above a deck, the molded line of which at its lowest point at side, is at least 2.14 m (7 ft) above the deepest load line. Hinged quick-acting doors, operable from both sides, are also permitted for vessels less than 80 m (262.4 ft) in length, L_f , in way of inboard compartments that are not within the assumed extent of damage that may be applicable to the vessel.

7.1.2 Access Doors Normally Closed at Sea

Access doors and access hatch covers normally closed at sea may be substantially constructed hinged type, fitted with gaskets and dogs, spaced and designed to ensure that the opening may be closed thoroughly watertight. These closing appliances are to be provided with means of indicating locally and on the bridge whether they are open or closed. A notice is to be affixed to each closing appliance to the effect that it is not to be left open.

7.1.3 Doors or Ramps Dividing Large Cargo Spaces

Watertight doors or ramps of satisfactory construction may be fitted to internally subdivide large cargo spaces, provided it is demonstrated to the Bureau that such doors or ramps are essential.

These doors or ramps may be hinged, rolling, or sliding doors or ramps, but are not to be remotely controlled. Such doors or ramps may be approved on condition that they be closed by the shipboard personnel before the voyage commences and kept closed during navigation and that the time of opening such doors or ramps in port and of closing them before the vessel leaves port is to be entered in the log book.

Doors or ramps accessible during the voyage are to be fitted with a device which prevents unauthorized opening.

7.1.4 Other Openings Closed at Sea

Closing appliances which are kept permanently closed at sea to ensure the watertight integrity of internal openings in watertight bulkheads and decks and are not fitted with a device which prevents unauthorized opening are to be provided with a notice affixed to each such closing appliance to the effect that it is to be kept closed. Manholes fitted with closely bolted covers need not be so marked.

7.3 Other Vessels

Watertight doors may be installed in all watertight bulkheads, except collision bulkheads.

7.5 Construction

Watertight doors are to be of ample strength for the water pressure to which they may be subjected. Door frames are to be carefully fitted to the bulkheads; where liners are required, the material is to be not readily injured by heat or by deterioration. Sliding doors are to be carefully fitted to the frames and are to be tested at the maker's works.

Where stiffeners are cut in way of watertight doors, the openings are to be framed and bracketed to maintain the full strength of the bulkheads without taking the strength of the door frames into consideration.

9 Testing

Requirements for testing are contained in Section 3-7-1.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **8 Deep Tanks**

1 General Arrangement

The arrangement of all deep tanks, their intended service, and the heights of the overflow pipes are to be clearly indicated on the drawings submitted for approval. Tanks forward of the collision bulkhead are not to be arranged for the carriage of oil or other flammable or combustible substances.

3 Construction

Boundary bulkheads and tight divisions of all deep tanks are to be constructed in accordance with the requirements of this section where they exceed those of Section 3-2-7. Where the specific gravity of the liquid exceeds 1.05, the design head, h , in this section is to be increased by the ratio of the specific gravity of 1.05.

5 Construction of Deep-tank Bulkheads

5.1 Plating

The minimum thickness of deep-tank boundary bulkheads and tight divisions is to be obtained from the following equation:

$$t = (sk\sqrt{qh}/254) + 2.5 \text{ mm} \quad \text{but not less than 6.5 mm or } s/150 + 2.5 \text{ mm, whichever is greater}$$

$$t = (sk\sqrt{qh}/460) + 0.10 \text{ in.} \quad \text{but not less than 0.25 in. or } s/150 + 0.10 \text{ in., whichever is greater}$$

where

$$\begin{aligned} t &= \text{thickness, in mm (in.)} \\ s &= \text{stiffener spacing, in mm (in.)} \\ k &= (3.075\sqrt{\alpha} - 2.077)/(\alpha + 0.272) \quad (1 \leq \alpha \leq 2) \\ &= 1.0 \quad (\alpha > 2) \\ \alpha &= \text{aspect ratio of the panel (longer edge/shorter edge)} \\ q &= 235/Y \text{ N/mm}^2 \text{ (24/Y kgf/mm}^2\text{, 34,000/Y psi)} \end{aligned}$$

- Y = as defined in 3-2-7/5.1
- h = the greatest of the following distances, in m (ft), from the lower edge of the plate to:
- A point located at two-thirds of the distance to the bulkhead or freeboard deck, or
 - A point located at two-thirds the distance from the top of the tank to the top of the overflow, or
 - The load line, or
 - A point located above the top of the tank, not less than the greater of the following:
 - $0.01L + 0.15$ m (0.5 ft), where L is as defined in 3-1-1/3, or 0.46 m (1.5 ft)

5.3 Stiffeners

The section modulus, SM , of each deep-tank stiffener, in association with the plating to which it is attached, is not to be less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

- c = 0.594 for stiffeners having effective bracket attachments at both ends. An effective bracket for the application of this value of c is to have scantlings not less than shown in 3-1-2/Table 2 and is to extend onto the stiffener for a distance at least one-eighth of the length, ℓ , of the stiffener.
- = 0.747 for stiffeners having an effective bracket attachment at one end and a clip connection or horizontal girder at the other end. An effective bracket for the application of this value of c is to have scantlings not less than shown in 3-1-2/Table 2 and is to extend onto the stiffener for a distance at least one-eighth of the length, ℓ , of the stiffener.
- = 0.90 for stiffeners having clip connections at both ends or having such attachments at one end and horizontal girders at the other end.
- = 1.00 for stiffeners having horizontal girders at both ends.
- ℓ = the distance, in m (ft), between the heels of the end attachments. Where horizontal girders are fitted, ℓ is the distance from the heel of the end attachment to the first girder or the distance between the horizontal girders.
- s = stiffener spacing, in m (ft)
- h = the greatest of the following distances, in m (ft), from the middle of ℓ to:
- A point located at two-thirds of the distance from the middle of ℓ to the bulkhead or freeboard deck, or
 - A point located at two-thirds of the distance from the top of the tank to the top of the overflow, or
 - The load line, or
 - A point located above the top of the tank, not less than the greater of the following:
 - $0.01L + 0.15$ m (0.5 ft), where L is the length of a vessel as defined in 3-1-1/3, or 0.46 m (1.5 ft)

5.5 Corrugated Bulkheads

Where corrugated bulkheads are used as deep-tank boundaries, the scantlings may be developed from 3-2-7/5.7. The plating thickness, t , and value of SM are to be as required by 3-2-8/5.1 and 3-2-8/5.3, respectively, with $c = 0.90$.

5.7 Girders and Webs

Horizontal girders or vertical webs supporting bulkhead stiffeners in deep tanks are to have a section modulus as required by this paragraph. Girders or webs supporting frames or beams in deep tanks are to have section modulus as required by Sections 3-2-5 and 3-2-6, respectively, or as required by this paragraph, whichever is the greater. The section modulus, SM , of each girder or web is not to be less than that obtained from the following equation:

$$SM = 4.74chs\ell^2 \text{ cm}^3$$

$$SM = 0.0025chs\ell^2 \text{ in}^3$$

where

$$c = 1.5$$

$$h = \text{vertical distance, in m (ft), from the middle of } s \text{ in the case of a girder or from the middle of } \ell \text{ in the case of a web to the same heights to which } h \text{ for the stiffeners is measured (see 3-2-8/5.3)}$$

$$s = \text{sum of half lengths, in m (ft) (on each side of the girder or web), of the frames or stiffeners supported by the girder or web.}$$

$$\ell = \text{unsupported length of girder or web, in m (ft). Where brackets are fitted in accordance with 3-1-2/5.5, the length, } \ell, \text{ may be measured as permitted therein.}$$

The depth of a girder or web is not to be less than 2.5 times the depth of the cutout unless effective compensation is provided for stiffener cutouts. The thickness is to be not less than 1 mm per 100 millimeters (0.01 inch per inch) of depth plus 3 mm (0.12 in.). Tripping brackets are to be fitted at intervals of about 3 m (10 ft) and stiffeners are to be fitted as may be required.

7 Tank Top Plating

Tops of tanks are to have plating 1 mm (0.04 in.) thicker than would be required for vertical plating at the same level. The thickness is not to be less than required for deck plating. Beams, girders and pillars are to be as required by Section 3-2-6.

9 Higher-strength Materials

9.1 General

In general, applications of higher-strength materials for deep-tank plating are to meet the requirements of this section, but may be modified as permitted by the following paragraphs. Calculations are to be submitted to show adequate provision to resist buckling.

9.3 Plating

Deep-tank plating of higher-strength material is to be of not less thickness than that obtained by 3-2-8/5.1.

9.5 Stiffeners

Each stiffener of higher-strength material, in association with the higher-strength plating to which it is attached, is to have section modulus, SM_{hts} , not less than that obtained from the following equation:

$$SM_{hts} = 7.8chs\ell^2Q \text{ cm}^3$$

$$SM_{hts} = 0.0041chs\ell^2Q \text{ in}^3$$

c , h , s and ℓ are as defined in 3-2-8/5.3 and Q is as defined in 3-2-1/7.5.

11 Drainage and Air Escape

Limber and air holes are to be cut, as required, in non-tight parts of the tanks to ensure the free flow of liquids to the suction pipes and the escape of air to the vents. Arrangements are to be made for draining the tops of the tanks.

13 Testing

Requirements for testing are contained in Section 3-7-1.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **9 Superstructures and Deckhouses**

1 Superstructure Scantlings

1.1 Side and Top Plating

The thickness of superstructure side plating is to be not less than that obtained from the requirements of 3-2-2/5. The thickness is also not to be less than that required by 3-2-9/3.5 for exposed aft-end bulkheads. Superstructure top plating is to be in accordance with Section 3-2-3.

1.3 Framing and Internal Bulkheads

Superstructure side frames are to be in accordance with 3-2-5/5.3. Bulkheads, partial bulkheads or web frames are to be fitted in the superstructure over the main hull bulkheads and elsewhere, as may be required to give effective transverse rigidity.

1.5 Breaks in Continuity

Breaks in the continuity of superstructures are to be specially strengthened (See 3-2-2/13). The arrangements in this area are to be clearly shown on the plans submitted for approval. Openings and changes in the scantlings of the decks and shell are to be kept well clear of the breaks.

1.7 Structural Support

Main bulkheads in the hull are to be arranged to provide support under the ends of the superstructures.

3 Exposed Bulkheads of Superstructures and Deckhouses

3.1 General

The scantlings of the exposed bulkheads of superstructures and deckhouses are to be in accordance with the following paragraphs, except that the requirements for house side stiffeners need not exceed the requirements of Section 3-2-5 for the side frames directly below the deck on which the house is located.

Special consideration may be given to the bulkhead scantlings of deckhouses which do not protect openings in the freeboard deck, superstructure deck or in the top of a lowest tier deckhouse or which do not protect machinery casings, provided they do not contain accommodation or do not protect equipment essential to the operation or safety of the vessel.

Superstructures or deckhouses located within the midship $0.4L$ that have lengths greater than $0.1L$ are to have effective longitudinal scantlings to give a hull-girder section modulus through the superstructure or deckhouse meeting the requirements for the main hull-girder. The superstructure scantlings are to be in accordance with 3-2-9/1 and the house top and side plating of long deckhouses are to be not less than $0.009s + 0.8$ mm ($0.009s + 0.032$ in.) where s is the spacing of the deck beams in mm (in.).

Partial bulkheads, deep webs, etc. are to be fitted at the ends and sides of large superstructures or deckhouses to provide resistance to racking.

In general, the first or lowest tier is that located on the freeboard deck. Where the depth to the uppermost continuous weather deck is such that the freeboard to this deck exceeds tabular freeboard by at least one standard superstructure height, deckhouses and superstructures on this weather deck may be considered second tier. Watertight bulkheads are to extend to this weather deck. This consideration of excess freeboard may be followed in a similar manner to determine third tier deckhouses or superstructures.

3.3 Stiffeners

Each stiffener, in association with the plating to which it is attached, is to have section modulus, SM , not less than that obtained from the following equation:

$$SM = 3.5s\ell^2h \text{ cm}^3$$

$$SM = 0.00185s\ell^2h \text{ in}^3$$

where

s = stiffener spacing, in m (ft)

ℓ = tween deck height or unsupported length, in m (ft)

h = $a[(bf) - y]c$, design head in m (ft). For unprotected front bulkheads on the lowest tier, h is to be taken as not less than 9.9 m (32.5 ft), and for sides and ends of first tier, h is to be taken as not less than 3.3 m (10.8 ft). For all other bulkheads the minimum value of h is to be not less than $1.25 + L/200$ m ($4.1 + L/200$ ft).

a = coefficient given in 3-2-9/Table 1.

$$b = 1.0 + \left[\frac{(x/L) - 0.45}{C_b + 0.2} \right]^2 \quad \text{where } (x/L) \leq 0.45$$

$$b = 1.0 + 1.5 \left[\frac{(x/L) - 0.45}{C_b + 0.2} \right]^2 \quad \text{where } (x/L) > 0.45$$

C_b = block coefficient at summer load waterline, based on the vessel's length, L , as defined in 3-1-1/3, not to be taken less than 0.60 nor greater than 0.80. For aft end bulkheads forward of amidships, C_b need not be taken as less than 0.80.

x = distance, in m (ft), between the after perpendicular and the bulkhead being considered. Deckhouse side bulkheads are to be divided into equal parts not exceeding $0.15L$ in length, and x is to be measured from the after perpendicular to the center of each part considered.

L = length of vessel, as defined in 3-1-1/3, in m (ft)

$$f = (L/10)(e^{-L/300}) - [1 - (L/150)^2] \quad \text{for } L, \text{ in m, see also 3-2-9/Table 2}$$

$$(L/10)(e^{-L/984}) - [3.28 - L/272]^2 \quad \text{for } L, \text{ in ft, see also 3-2-9/Table 2}$$

- y = vertical distance, in m (ft), from the summer load waterline to the midpoint of the stiffener span.
- c = $(0.3 + 0.7b_1/B_1)$, but is not to be taken as less than 1.0 for exposed machinery casing bulkheads. In no case is b_1/B_1 to be taken as less than 0.25.
- b_1 = breadth of deckhouse at position being considered, in m (ft)
- B_1 = actual breadth of vessel at the freeboard deck at the position being considered, in m (ft)

Where windows are fitted in bulkheads, the spacing, s , is to be the spacing of the mullion stiffeners. The mullion stiffeners are to extend continuously from deck to deck.

TABLE 1
Values of a

Bulkhead Location	Metric Units	US Units
Unprotected front Lowest tier	$2.0 + L/120$	$2.0 + L/393.6$
Unprotected front Second tier	$1.0 + L/120$	$1.0 + L/393.6$
Unprotected front Third tiers	$0.5 + L/150$	$0.5 + L/492$
Protected front All tiers	$0.5 + L/150$	$0.5 + L/492$
Sides, All tiers	$0.5 + L/150$	$0.5 + L/492$
Aft ends, aft of amidships, All tiers	$0.7 + (L/1000) - 0.8x/L$	$0.7 + (L/3280) - 0.8x/L$
Aft ends, forward of amidships, All tiers	$0.5 + (L/1000) - 0.4x/L$	$0.5 + (L/3280) - 0.4x/L$

TABLE 2
Values of f

Intermediate values of f may be obtained by interpolation

SI and MKS Units		US Units	
L, m	f	L, ft	f
24	1.24	79	4.09
40	2.57	130	8.34
60	4.07	200	13.6
80	5.41	250	17.0
90	6.00	295	19.8

3.5 Plating

The plating is to be not less in thickness than that obtained from the following equation:

$$t = 3s\sqrt{h} \text{ mm}$$

$$t = s\sqrt{h}/50 \text{ in.}$$

where

s and h are as defined in 3-2-9/3.3 above. When determining h , y is to be measured to the middle of the panel.

In no case is the thickness for bulkheads, other than the lowest tier, to be less than 5.0 mm (0.20 in.).

In addition, the thicknesses are to be not less than the following:

For the lowest tier and for deckhouses on the forecastle deck:

For front bulkheads:

$$t = (s/0.60)(6 + 0.02L) \text{ mm}$$

$$t = (s/1.97)(0.24 + 0.00024L) \text{ in.}$$

For side and end bulkheads:

$$t = (s/0.60)(5 + 0.02L) \text{ mm}$$

$$t = (s/1.97)(0.20 + 0.00024L) \text{ in.}$$

Where L is as defined in 3-2-9/3.3 and s is as defined in 3-2-9/3.3, but is not to be taken less than 0.60 m (1.97 ft).

3.7 End Attachments

The upper and lower ends of all lowest tier bulkheads stiffeners, of all bulkhead stiffeners of the tier on the forecastle deck and of all unprotected front bulkhead stiffeners are to be effectively attached. The end attachments are to be formed by end brackets, by welding the webs all around to the deck plating or by lapping the stiffener onto the deck beam.

The ends of all transverse webs in way of girders or transverses are to be welded.

Where only the webs are welded, the flange or face bar is to be sniped not more than 30°.

The webs of all the bulkhead stiffeners may be sniped, having due regard to their scantlings and the depth of the web.

3.9 Raised-quarter-deck Bulkheads

Raised-quarter-deck bulkheads are to have plating of not less thickness than required for unprotected front bulkheads.

The sizes of stiffeners are to be specially considered on the basis of the length of the vessel, the actual height of the raised quarter deck and the arrangement of the structure.

5 Enclosed Superstructures

5.1 Closing Appliances

All openings in the bulkheads of enclosed superstructures are to be provided with efficient means of closing, so that in any sea conditions, water will not penetrate the vessel. Opening and closing appliances are to be framed and stiffened so that the whole structure, when closed, is equivalent to the unpierced bulkhead.

Doors for access openings into enclosed superstructures are to be of steel or other equivalent material, permanently and strongly attached to the bulkhead. The doors are to be provided with gaskets and clamping devices, or other equivalent arrangements, permanently attached to the bulkhead or to the doors themselves, and the doors are to be so arranged that they can be operated from both sides of the bulkhead.

Portlights in the end bulkheads of enclosed superstructures are to be of substantial construction and provided with efficient inside deadlights. Also see 3-2-14/7 and 3-2-14/9.

The location and means of the closing appliances for windows are to be in accordance with 3-2-14/9.

5.3 Sills of Access Openings

Except as otherwise provided in these Rules, the height of the sills of access openings in bulkheads at the ends of enclosed superstructures is to be at least 380 mm (15 in.) above the deck. See 3-2-12/Table 1 for required sill heights.

5.5 Means of Access

Superstructures are not to be regarded as enclosed unless access is provided for the crew to reach machinery and other working spaces inside these superstructures by alternate means which are available at all times when bulkhead openings are closed.

7 Open Superstructures

Superstructures with openings which do not fully comply with 3-2-9/5 are to be considered as open superstructures. See also 3-2-14/5.7.

9 Deckhouses

Deckhouses are to comply with 3-2-9/3. Bulkheads are to be arranged as necessary in the main hull to support deckhouses.

The closing appliances for the openings in deckhouse bulkheads are to comply with 3-2-9/5.1.

Doors for access openings into deckhouses are to be of steel or other equivalent material, permanently and strongly attached to the bulkhead. The doors are to be provided with gaskets and clamping devices, or other equivalent arrangements, permanently attached to the bulkhead or to the doors themselves, and the doors are to be arranged so that they can be operated from both sides of the bulkhead.

10 Aluminum Deckhouses (2002)

10.1 Scantling Correction

Where deckhouses are constructed of aluminum alloys, the required plate thickness and stiffener section modulus, SM , are first to be determined as required for steel deckhouses, and are then to be increased by the material factor, Q_0 , as indicated below.

For all deck and bulkhead plating and stiffeners, the required thickness and section modulus for aluminum alloy plate and shapes are obtained from the following equations:

Deck plating:

$$t_{al} = \frac{0.9(Q + \sqrt{Q})}{2} t_s$$

Bulkhead plating:

$$t_{al} = 0.9Q_0 t_s$$

Deck and bulkhead stiffeners:

$$SM_{al} = 0.9Q_0 SM_s$$

where

- t_{al} = minimum thickness of aluminum plate
- t_s = required plate thickness for steel obtained from 3-2-3/3 for decks and 3-2-9/3.5 for side and end bulkheads

- SM_{al} = minimum section modulus of aluminum stiffeners
 SM_s = minimum section modulus of steel stiffeners, as determined from 3-2-6/1 and 3-2-6/3 for deck stiffeners and 3.2.9/3.3 for bulkhead stiffeners
 Q = material factor, as determined from 3-2-9/10.3 below
 Q_0 = material factor, as determined from 3-2-9/10.3 below

10.3 Material Factors

The material factor, Q , is obtained from the following equation:

$$Q = 0.9 + (120/Y_{al}) \quad \text{SI Units}$$

$$Q = 0.9 + (12/Y_{al}) \quad \text{MKS Units}$$

$$Q = 0.9 + (17000/Y_{al}) \quad \text{U.S. Units}$$

but is not to be taken as less than Q_0 below.

The material factor, Q_0 , is obtained from the following equation:

$$Q_0 = 635/(\sigma_y + \sigma_u) \quad \text{SI Units}$$

$$Q_0 = 65/(\sigma_y + \sigma_u) \quad \text{MKS Units}$$

$$Q_0 = 92000/(\sigma_y + \sigma_u) \quad \text{U.S. Units}$$

- Y_{al} = minimum yield strength of the welded aluminum alloy under consideration at 2% offset in a 254 mm (10 in.) gauge length, in N/mm² (kgf/mm², psi), in accordance with the requirements of the table below
 σ_u = minimum ultimate strength of the welded aluminum alloy under consideration, in N/mm² (kgf/mm², psi), in accordance with the table below
 σ_y = minimum yield strength of the welded aluminum alloy under consideration, in N/mm² (kgf/mm², psi), in accordance with the table below

Minimum Mechanical Properties for Butt-Welded Aluminum Alloys

Alloy	Ultimate Tensile Strength (σ_u) N/mm ² (kgf/mm ² , psi)	Yield Strength (σ_y) ⁽³⁾ N/mm ² (kgf/mm ² , psi)
5083 ⁽¹⁾	275 (28.1, 40000)	125 (12.7, 18000)
5086 ⁽¹⁾	240 (24.6, 35000)	95 (9.85, 14000)
5454 ⁽¹⁾	215 (21.8, 31000)	85 (8.45, 12000)
5456 ⁽¹⁾	290 (29.5, 42000)	130 (13.4, 19000)
6061-T6 ⁽²⁾	165 (16.9, 24000)	105 (10.6, 15000)

Notes:

- All tempers
- Values when welded with 4043, 5183, 5356 or 5556 filler wire.
- Yield strength is not required for weld procedure qualification. Values shown apply to the yield strength values of 3-2-9/10.3.

For other alloys, refer to Table 4 of Section 3 of the Aluminum Association's Aluminum Construction Manual.

10.5 Attachments

Stiffeners on bulkheads are to be attached to the deck plating at their upper and lower ends by welding all around, for which cladding metal is to be inserted between stiffeners and steel deck plate. Suitable means are to be taken to avoid direct contact of faying surfaces of aluminum to steel.

11 Helicopter Decks

11.1 General

Helicopter landing facilities, where provided, are to meet the following structural and safety requirements. The attention of owners, builders and designers is directed to various international and governmental regulations and guides regarding the operational and other design requirements for helicopters landing on ships. See also Section 1-1-5 of the *ABS Rules for Conditions of Classification (Part 1)* and 4-5-1/7 of these Rules. Plans showing the arrangement, scantlings and details of the helicopter deck are to be submitted. The arrangement plan is to show the overall size of the helicopter deck and the designated landing area. If the arrangement provides for the securing of a helicopter or helicopters to the deck, the predetermined position(s) selected to accommodate the secured helicopter, in addition to the locations of deck fittings for securing the helicopter, are to be shown. The type of helicopter to be considered is to be specified and calculations for appropriate loading conditions are to be submitted.

11.3 Structure

Scantlings of helicopter decks and supporting structure are to be determined on the basis of the following loading conditions, whichever is greater, in association with the allowable factors of safety shown in 3-2-9/Table 3. Plastic design considerations may be applied for deck plating and stiffeners.

11.3.1 Overall Distributed Loading

A minimum distributed loading of 2010 N/m² (205 kgf/m², 42 lbf/ft²) is to be taken over the entire helicopter deck.

11.3.2 Helicopter Landing Impact Loading

A load of not less than 75% of the helicopter maximum take-off weight is to be taken on each of two square areas, 0.3 m × 0.3 m (1 ft × 1 ft). Alternatively, the manufacturer's recommended wheel impact loading will be considered. The deck is to be considered for helicopter landings at any location within the designated landing area. The structural weight of the helicopter deck is to be added to the helicopter impact loading when considering girders, stanchions, truss supports, etc. Where the upper deck of the superstructure or deckhouse is used as a helicopter deck and the spaces below are normally manned (quarters, bridge, control room, etc.), the impact loading is to be multiplied by a factor of 1.15.

11.3.3 Stowed Helicopter Loading

If provisions are made to accommodate helicopters secured to the deck in a predetermined position, the structure is to be considered for a local loading equal to the manufacturer's recommended wheel loadings at maximum take-off weight, multiplied by a dynamic amplification factor based on the predicted motions of the vessel for this condition, as may be applicable for the vessel under consideration.

In addition to the helicopter load, a uniformly distributed loading of 490 N/m² (50 kgf/m², 10.5 lbf/ft²) representing wet snow or ice is to be considered, if applicable. For the girders, stanchions, truss supports, etc., the structural weight of the helicopter deck is also to be considered.

11.3.4 Loading due to Motions of Vessel

The structure supporting helicopter decks is to withstand the loads resulting from the motions of the vessel.

11.3.5 Special Landing Gear

Helicopters fitted with landing gear other than wheels will be specially considered.

11.3.6 Environmental Loading

Calculations are to consider anticipated wind and wave impact loadings on helicopter decks and their supporting structures.

11.5 Safety Net

The unprotected perimeter of the helicopter landing deck is to be provided with safety netting or equivalent.

11.7 Aluminum Decks

Aluminum may be used for helicopter decks above deckhouses provided the following conditions are complied with:

- i) There are to be no openings in the exterior bulkheads directly below the helicopter decks.
- ii) All windows in the lower exterior bulkheads are to be fitted with steel shutters.

TABLE 3
Allowable Factors of Safety Based on Y for Helicopter Decks

Y = specified minimum yield point or yield strength of the material

	<i>Plating</i>	<i>Beams</i>	<i>Girders, Stanchions Truss Supports, etc. (See Note 3)</i>
Overall Distributed Loading	1.67 (See Note 1)	1.67	1.67
Helicopter Landing Impact Loading	(See Note 2)	1.00	1.10
Stowed Helicopter Loading	1.00	1.10	1.25

Notes:

- 1 The minimum plate thickness, t , is generally not to be less than that obtained from the following:

<i>Beam Spacing</i>	<i>t</i>
460 mm	4.0 mm
610 mm	5.0 mm
760 mm	6.0 mm

<i>Beam Spacing</i>	<i>t</i>
18 in.	0.16 in.
24 in.	0.20 in.
30 in.	0.24 in.

- 2 An allowable stress that exceeds Y may be considered, provided the rationale of the submitted analysis is sufficiently conservative.
- 3 For members subjected to axial compression, the factor of safety is to be based on the yield stress or critical buckling stress, whichever is less.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **10 Keels, Stems, Stern Frames, Shaft Struts, and Propeller Nozzles**

1 Keels

1.1 Bar Keels

Bar keels are to have thicknesses and depths not less than that obtained from the following equations:

$$t = 0.625L + 12.5 \text{ mm}$$

$$t = 0.0075L + 0.50 \text{ in.}$$

$$h = 1.46L + 100 \text{ mm}$$

$$h = 0.0175L + 4 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$h = \text{depth, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in Section 3-1-1}$$

Thicknesses and widths other than given above are acceptable, provided the section moduli and moments of inertia about the transverse horizontal axis are not less than given above, nor is h/t more than 4.5.

1.3 Plate Keels

The thickness of the plate keel throughout the length of the vessel is to be not less than the bottom shell required in 3-2-2/3.3.

3 Stems

3.1 Bar Stems

Bar stems are to have thicknesses and widths not less than that obtained from the following equations:

$$t = 0.625L + 6.35 \text{ mm}$$

$$t = 0.0075L + 0.25 \text{ in.}$$

$$w = 1.25L + 90 \text{ mm}$$

$$w = 0.015L + 3.5 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$w = \text{width, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in Section 3-1-1}$$

This thickness and width is to be maintained between the keel and design load waterline. Above the design load waterline, they may be gradually reduced until the area at the head is 70% of that obtained from the equations.

Thicknesses and widths other than given above are acceptable, provided the section moduli and moments of inertia about the longitudinal axis are not less than above, nor w/t more than 5.5. The thickness of the bar stem, in general, should also not be less than twice the shell thickness.

3.3 Cast or Forged Stems

Cast or forged stems of special shape are to be proportioned to provide strengths at least equivalent to those of bar stems, as obtained in 3-2-10/3.1, and all joints and connections are to be at least as effective as would be required on equivalent bar stems.

3.5 Plate Stems

Where plate stems are used, they are not to be less in thickness than the bottom shell plating required in 3-2-2/1 and 3-2-2/3, where s is the frame spacing, or 610 mm (24 in.), if greater.

5 Sternposts

5.1 Bar Sternposts

Bar sternposts without propeller bosses are to have thicknesses and widths not less than that obtained from the following equations:

$$t = 0.73L + 10 \text{ mm} \qquad t = 0.0088L + 0.39 \text{ in.}$$

$$b = 1.283L + 87.4 \text{ mm} \qquad b = 0.0154L + 3.44 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$b = \text{width, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in Section 3-1-1}$$

Above the bottom shell plating, sternposts may be gradually reduced until the areas at their heads are half the areas obtained from the above equations.

Thickness or width less than given above are acceptable, provided the section modulus and moment of inertia about the longitudinal axis are not less than those of a plate having the minimum thickness and width given above, and with b/t not more than 4.0.

5.3 Cast, Forged, or Fabricated Sternposts

Cast, forged or fabricated sternposts of special shape are to be so proportioned as to provide strengths at least equivalent to those of bar posts, as obtained from 3-2-10/5.1, and all joints and connections are to be at least as effective as would be required on equivalent bar posts.

7 Stern Frames

Except as modified in 3-2-10/9, the scantlings of stern frames of single screw vessels are to be in accordance with the following, as applicable.

7.1 Below the Boss

7.1.1 Fabricated Stern Frame

The thickness, t , width, w , and length, ℓ , are not to be less than given by the following equations:

$$t = 0.225\sqrt{L} \text{ cm} \qquad t = 0.049\sqrt{L} \text{ in.}$$

$$w = 5\sqrt{L} \text{ cm} \qquad w = 1.09\sqrt{L} \text{ in.}$$

$$\ell = 4\sqrt{L} \text{ cm} \qquad \ell = 0.87\sqrt{L} \text{ in.}$$

Widths and lengths other than given above are acceptable, provided the section modulus, SM , about the longitudinal axis is not less than:

$$SM = 1.60L^{1.5} \text{ cm}^3 \qquad SM = 0.0164L^{1.5} \text{ in}^3$$

where

t = thickness of side plating, in cm (in.) (See 3-2-10/Figure 1)

w = width of stern frame, in cm (in.) (See 3-2-10/Figure 1)

ℓ = length of stern frame, in cm (in.) (See 3-2-10/Figure 1)

L = length of vessel, in m (ft), as defined in Section 3-1-1

7.1.2 Cast Stern Frame

The thicknesses, t_1 , t_2 , width, w , and length, ℓ , are not to be less than given by the following equations:

$$t_1 = 0.3\sqrt{L} \text{ cm} \qquad t_1 = 0.065\sqrt{L} \text{ in.}$$

but not less than 2.5 cm (1.0 in.)

$$t_2 = 1.25t_1$$

$$w = 5\sqrt{L} \text{ cm} \qquad w = 1.09\sqrt{L} \text{ in.}$$

$$\ell = 4\sqrt{L} \text{ cm} \qquad \ell = 0.87\sqrt{L} \text{ in.}$$

Widths and lengths other than given above are acceptable, provided the section modulus, SM , about the longitudinal axis is not less than:

$$SM = 1.60L^{1.5}K_g \text{ cm}^3 \qquad SM = 0.0164L^{1.5}K_g \text{ in}^3$$

where

t_1 = thickness of casting at end. (See 3-2-10/Figure 1)

t_2 = thickness of casting at mid-length. (See 3-2-10/Figure 1)

K_g = material factor defined in 3-2-11/1.3

w , ℓ , L are as defined in 3-2-10/7.1.1.

The thickness in way of butt welding to shell plating may be tapered below t_1 . The length of taper is to be at least three times the offset.

The castings are to be cored out to avoid large masses of thick material likely to contain defects and to maintain a relatively uniform section throughout. Suitable radii are to be provided in way of changes in section.

7.3 Above the Boss

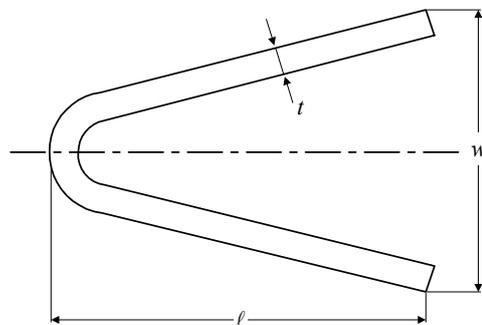
Above the propeller boss, the scantlings are to be in accordance with 3-2-10/7.1, except that in the upper part of the propeller aperture where the hull form is full and centerline supports are provided, the thickness may be reduced to 80% of the requirements in 3-2-10/7.1, subject to the same minimum for cast steel stern frames.

7.5 Secondary Members

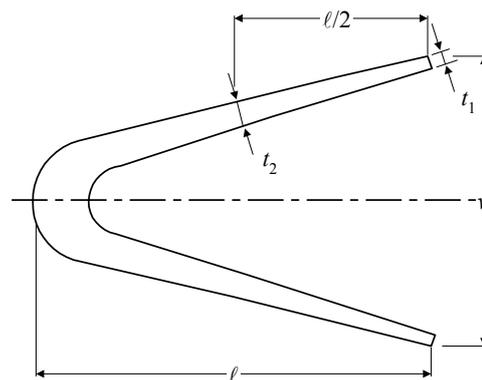
Where round bars are used at the after edge of stern frames, their scantlings and connection details are to be such as to facilitate welding.

Ribs or horizontal brackets of thickness not less than $0.8t$ or $0.8t_1$ are to be provided at suitable intervals, extended forward and attached to the adjacent floor. Where t or t_1 is reduced in accordance with 3-2-10/7.3, a proportionate reduction in the thickness of ribs or horizontal brackets may be made.

FIGURE 1
Stern Frame



a. Fabricated



b. Cast

9 Stern Frames with Shoepieces

The scantlings below the boss of stern frames with shoepieces are to be gradually increased to provide strength and stiffness in proportion to those of the shoepieces.

11 Shoepieces

11.1 General

The shoepiece is to be sloped to avoid pressure from the keel blocks when docking and is to extend at least two frame spaces forward of the forward edge of the propeller boss.

11.3 Design Stress

The equivalent stress, σ_e , in the shoepiece at any section is not to exceed $115/K_g$ N/mm² ($11.7/K_g$ kgf/mm², $16700/K_g$ psi) and is to be obtained from the following equation:

$$\sigma_e = n\sqrt{\sigma_b^2 + 3\tau^2}$$

where

$$n = 1000 \text{ (1000, 2240)}$$

$$K_g = K, \text{ as defined in 3-2-11/1.3 for castings and forgings}$$

$$= 1.0 \text{ for ordinary strength hull steel plate}$$

$$\sigma_b = \text{bending stress} = 0.5C_R\ell/Z_v$$

$$C_R = \text{rudder force, as defined in 3-2-11/3.}$$

$$\ell = \text{horizontal distance between centerline of rudder stock and the particular section of the stern frame shoe, in m (in.) (see 3-2-10/Figure 2)}$$

$$Z_v = \text{section modulus of shoepiece about the vertical axis at the particular section under consideration, in cm}^3 \text{ (in}^3\text{)}$$

$$\tau = \text{shear stress} = 0.5C_R/A_s$$

$$A_s = \text{sectional area at the section of the shoepiece under consideration, in mm}^2 \text{ (in}^2\text{)}$$

11.5 Minimum Scantlings

In addition, shoepiece width is to be approximately twice the depth, and vertical and horizontal section modulus and sectional area are in no case less than required by the following equations.

$$Z_z = k_z C_R \ell K_g \quad \text{cm}^3 \text{ (in}^3\text{)}$$

$$Z_y = 0.5Z_z \quad \text{cm}^3 \text{ (in}^3\text{)}$$

$$A_s' = k_a C_R K_g \quad \text{mm}^2 \text{ (in}^2\text{)}$$

where

$$Z_z = \text{minimum required section modulus of shoepiece about the vertical axis at the particular section under consideration}$$

$$Z_y = \text{minimum required section modulus of shoepiece about the transverse horizontal axis at the particular section under consideration}$$

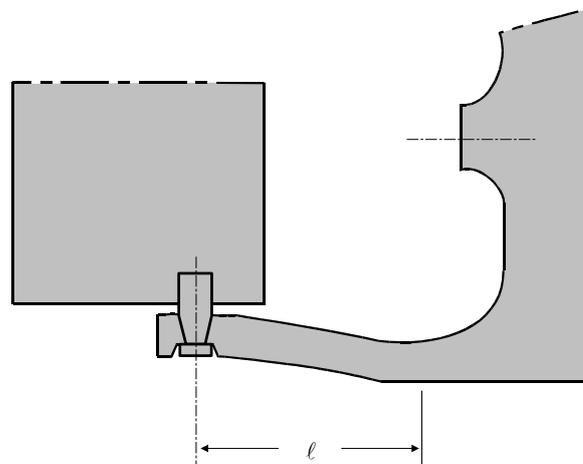
$$A_s' = \text{minimum required sectional area of shoepiece at the section under consideration}$$

$$k_z = 6.25 \text{ (61.3, 0.0967)}$$

$$k_a = 10.4 \text{ (102, 0.161)}$$

C_R , ℓ and K_g are as defined in 3-2-10/11.3.

FIGURE 2
Shoepiece



13 Rudder Horns

Vessels that have rudder horns are to meet the requirements in 3-2-13/5 of the *Steel Vessel Rules*.

15 Rudder Gudgeons

Rudder gudgeons are to be an integral part of the stern frame. The bearing length of the pintle is to be between 1.0 and 1.2 times the pintle diameter, and the thickness of the pintle housing is not to be less than 25% of the pintle diameter.

17 Shaft Struts

17.1 General

Tail-shaft (propeller-shaft) struts, where provided, may be of the V or I type. The thickness of the strut barrel or boss is to be at least one-fourth the diameter of the tail shaft. The length of the strut barrel or boss is to be adequate to accommodate the required length of propeller-end bearings. The following equations are for struts having streamlined cross-sectional shapes.

17.3 V Strut

17.3.1 Inertia

The moment of inertia, I_{x-x} , of each strut arm is not to be less than that obtained from the following equation:

$$I_{x-x} = 0.0044D^4 \quad \text{mm}^4 \text{ (in}^4\text{)}$$

where

$$D = \text{required diameter of ABS Grade 2 tail shaft, in mm (in.) (see Section 4-3-1)}$$

17.3.2 Section Modulus

The section modulus, SM_{x-x} , of each strut arm is not to be less than that obtained from the following equation:

$$SM_{x-x} = 0.024D^3 \quad \text{mm}^3 \text{ (in}^3\text{)}$$

where

$$D = \text{required diameter of ABS Grade 2 tail shaft, in mm (in.)}$$

Where the included angle is less than 45 degrees, the foregoing scantlings are to be specially considered.

17.5 I Strut

17.5.1 Inertia

The moment of inertia, I_{x-x} , of the strut arm is not to be less than that obtained from the following equation:

$$I_{x-x} = 0.018D^4 \quad \text{mm}^4 \text{ (in}^4\text{)}$$

where

$$D = \text{required diameter of ABS Grade 2 tail shaft, in mm (in.)}$$

17.5.2 Section Modulus

The section modulus, SM_{x-x} , of the strut is not to be less than that obtained from the following equation:

$$SM_{x-x} = 0.068D^3 \quad \text{mm}^3 \text{ (in}^3\text{)}$$

where

$$D = \text{required diameter of ABS Grade 2 tail shaft, in mm (in.)}$$

17.7 Strut Length

The length of the longer leg of a V strut or the leg of an I strut, measured from the outside perimeter of the strut barrel or boss to the outside of the shell plating, is not to exceed 10.6 times the diameter of the tail shaft. Where this length is exceeded, the width and thickness of the strut are to be increased, and the strut design will be given special consideration.

19 Propeller Nozzles (2009)

19.1 Application

The requirements in this section are applicable for fixed propeller nozzles with inner diameter d of 5 meters (16.4 feet) or less. Nozzles of larger inner diameter are subject to special consideration with all supporting documents and calculations submitted for review.

19.3 Design Pressure

The design pressure of the nozzle is to be obtained from the following:

$$p_d = 10^{-6} \cdot c \cdot \varepsilon \cdot \left(\frac{N}{A_p} \right) \quad \text{N/mm}^2 \text{ (kgf/mm}^2\text{, psi)}$$

where

- c = coefficient as indicated in 3-2-10/Table 1
- ε = coefficient as indicated in 3-2-10/Table 2, but not to be taken less than 10
- N = maximum shaft power, in kW (hp)
- A_p = propeller disc area
 $= D^2 \frac{\pi}{4}$, in m² (ft²)
- D = propeller diameter, in m (ft)

TABLE 1
Coefficient c (2009)

Propeller Zone (see 3-2-10/Figure 3)	c		
	p_d in N/mm ²	p_d in kgf/mm ²	p_d in psi
2	10.0	1.02	11.62×10^3
1 & 3	5.0	0.51	5.81×10^3
4	3.5	0.36	4.067×10^3

TABLE 2
Coefficient ε (2009)

	p_d in N/mm ²	p_d in kgf/mm ²	p_d in psi
ε	$21 - 2 \times 10^{-2} \left(\frac{N}{A_p} \right)$	$21 - 2 \times 10^{-2} \left(\frac{N}{A_p} \right)$	$21 - 16 \times 10^{-2} \left(\frac{N}{A_p} \right)$

19.5 Nozzle Cylinder

19.5.1 Shell Plate Thickness

The thickness of the nozzle shell plating, in mm (in.), is not to be less than:

$$t = t_o + t_c, \text{ but not to be taken less than } 7.5 \text{ (0.3) mm (in.)}$$

where

- t_o = thickness obtained from the following formula:
 $= c_n \cdot S_p \cdot \sqrt{p_d}$ mm (in.)
- c_n = coefficient as indicated in 3-2-10/Table 3
- S_p = spacing of ring webs in mm (in.)
- p_d = nozzle design pressure in N/mm² (kgf/mm², psi), as defined in 3-2-10/19.3
- t_c = corrosion allowance determined by 3-2-10/Table 4
- K_n = nozzle material factor as defined in 3-2-11/1.3

TABLE 3
Coefficient c_n (2009)

	p_d in N/mm^2	p_d in kgf/mm^2	p_d in psi
c_n	1.58×10^{-1}	4.95×10^{-1}	1.32×10^{-2}

TABLE 4
Corrosion Allowance t_c (2009)

Value of t_o	t_c mm (in.)
If $t_o \leq 10.0$ (0.4)	1.5 (0.06)
If $t_o > 10.0$ (0.4)	the lesser of b_1, b_2
where	
$b_1 = 3.0$ (0.12) mm (in.)	
$b_2 = \left(\frac{t_o}{\sqrt{K_n}} + 5 \right) \times 10^{-1}$ mm or $b_2 = \left(\frac{t_o}{\sqrt{K_n}} + 0.2 \right) \times 10^{-1}$ in.	

19.5.2 Internal Diaphragm Thickness

Thickness of nozzle internal ring web is not to be less than the required nozzle shell plating for Zone 3.

19.7 Nozzle Section Modulus

The minimum requirement for nozzle section modulus is obtained from the following formula:

$$SM = d^2 b V_d^2 Q n \text{ cm}^3 \text{ (in}^3\text{)}$$

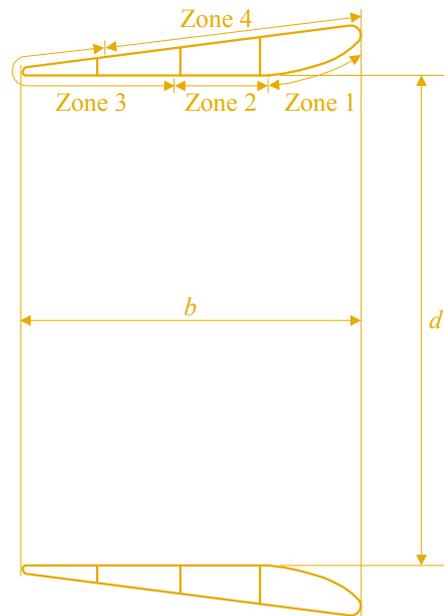
where

- d = nozzle inner diameter, in m (ft)
- b = nozzle length, in m (ft)
- V_d = design speed in ahead condition, in knots, as defined in 3-2-11/3.1
- Q = reduction factor conditional on material type
 - = 1.0 for ordinary strength steel
 - = 0.78 for H32 strength steel
 - = 0.72 for H36 strength steel
 - = 0.68 for H40 strength steel

Q factor for steel having yield strength other than above is to be specially considered.

- n = nozzle type coefficient taken equal to 0.7 (0.0012) for fixed nozzles

FIGURE 3
Nozzle Ring Section View (2009)



19.9 Welding Requirement

The inner and outer nozzle shell plating is to be welded to the internal stiffening ring webs with double continuous welds as far as practicable. Plug/slot welding is prohibited for the inner shell, but may be accepted for the outer shell plating, provided that the nozzle ring web spacing is not greater than 350 mm (13.8 in.).

21 Inspection of Castings

The location of radiographic or other subsurface inspections of large stern-frame and rudder-horn castings is to be indicated on the approved plans. See applicable parts of Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)*.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **11 Rudders (2009)**

1 General

1.1 Application

Requirements specified in this section are applicable to:

- i) Ordinary profile rudders described in 3-2-11/Table 1A;
- ii) High-lift rudders described in 3-2-11/Table 1B;

Rudders not covered in 3-2-11/Table 1A or Table 1B are subject to special consideration, provided that all the required calculations are prepared and submitted for review in full compliance with the requirements in this section.

Rudders provided on Ice Classed vessels, are subject to additional requirements specified in 6-1-1/43 of the *Steel Vessel Rules*.

1.3 Rudder and Rudder Stock Materials

Rudder stocks, pintles, coupling bolts and keys are to be made from material in accordance with the requirements of Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)*. **The Surveyor need not witness** material tests for coupling bolts and keys. The surface of rudder stocks in way of exposed bearings are to be of non-corrosive material.

Material factors of castings and forgings used for the shoe (piece) (K_g), horn (K_h), stock (K_s), bolts (K_b), coupling flange (K_f) and pintles (K_p), and nozzles (K_n) are to be obtained for their respective material from the following equation.

$$K = (n_y/Y)^e$$

where

- n_y = 235 N/mm² (24 kgf/mm², 34000 psi)
- Y = Specified minimum yield strength of the material, in N/mm² (kgf/mm², psi), but is not to be taken as greater than 0.7U or 450 N/mm² (46 kgf/mm², 65000 psi), whichever is lesser
- U = minimum tensile strength of material used, in N/mm² (kgf/mm², psi)
- e = 1.0 for $Y \leq 235$ N/mm² (24 kgf/mm², 34000 psi)
= 0.75 for $Y > 235$ N/mm² (24 kgf/mm², 34000 psi)

1.5 Expected Torque

The torque considered necessary to operate the rudder in accordance with 4-3-3/1.9 is to be indicated on the submitted rudder or steering gear plan. See 4-3-3/1.5 and 3-2-11/5.7.

Note that this expected torque is not the design torque for rudder scantlings.

1.7 Rudder Stops

Strong and effective structural rudder stops are to be fitted. Where adequate positive mechanical stops are provided within the steering gear in accordance with 4-3-3/5.1, structural stops will not be required.

3 Rudder Design Force

Rudder force, C_R , upon which rudder scantlings are to be based, is to be obtained from equation described either in Section 3-2-11/3.1 or Section 3-2-11/3.3 as applicable. Where for the ordinary rudders the rudder angle, ϕ , exceeds 35° , the rudder force, C_R , is to be increased by a factor of $1.74 \sin(\phi)$.

3.1 Rudder Blades without Cutouts

Where the rudder profile can be defined by a single quadrilateral, the rudder force is to be obtained from the following equation:

$$C_R = n k_R k_c k_\ell A V_R^2 \quad \text{kN (tf, Ltf)}$$

where

$$n = 0.132 \text{ (0.0135, 0.00123)}$$

$$k_R = (b^2/A_t + 2)/3 \text{ but not taken more than 1.33}$$

$$b = \text{mean height of rudder area, in m (ft), as determined from 3-2-11/Figure 1}$$

$$A_t = \text{sum of rudder blade area, } A, \text{ and the area of rudder post or rudder horn within the extension of rudder profile, in m}^2 \text{ (ft}^2\text{)}$$

$$A = \text{total projected area of rudder as illustrated in 3-2-11/Figure 1, in m}^2 \text{ (ft}^2\text{)}$$

For steering nozzles, A is not to be taken less than 1.35 times the projected area of the nozzle.

$$k_c = \text{coefficient depending on rudder cross section as indicated in 3-2-11/Table 1A and 1B. For cross section differing from those in 3-2-11/Table 1A and 1B, } k_c \text{ is subject to special consideration.}$$

$$k_\ell = \text{coefficient as specified in 3-2-11/Table 2}$$

$$V_R = \text{vessel speed, in knots}$$

$$= \text{for ahead condition } V_R \text{ equals } V_d \text{ or } V_{\min}, \text{ whichever is greater}$$

$$= \text{for astern condition } V_R \text{ equals } V_a \text{ or } 0.5V_d, \text{ or } 0.5V_{\min}, \text{ whichever is greater}$$

$$V_d = \text{design speed in knots with the vessel running ahead at the maximum continuous rated shaft rpm and at the summer load waterline}$$

$$V_a = \text{maximum astern speed in knots}$$

$$V_{\min} = (V_d + 20)/3$$

3.3 Rudder Blades with Cutouts

This paragraph applies to rudders with cutouts (semi-spade rudders), such that the whole blade area cannot be adequately defined by a single quadrilateral. See 3-2-11/Figure 2. Equations derived in this paragraph are based on a cutout blade with two quadrilaterals. Where more quadrilaterals are needed to define the rudder shape, similar rules apply.

The total rudder force described in 3-2-11/3.1 is applicable for rudders with cutout(s), with A being the summation of sub-quadrilaterals that make up the whole area of the rudder blade. Rudder force distribution over each quadrilateral is to be obtained from the following equations:

$$C_{R1} = C_R A_1 / A \quad \text{kN (tf, Ltf)}$$

$$C_{R2} = C_R A_2 / A \quad \text{kN (tf, Ltf)}$$

where

C_R and A are as defined in 3-2-11/3.1

A_1 and A_2 are as described in 3-2-11/Figure 2.

TABLE 1A
Coefficient k_c for Ordinary Rudders (2009)

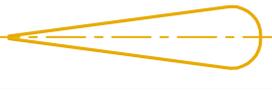
	Profile Type	k_c	
		Ahead Condition	Astern Condition
1	Single plate 	1.0	1.0
2	NACA-OO Göttingen 	1.1	0.80
3	Flat side 	1.1	0.90
4	Mixed (e.g., HSVA) 	1.21	0.90
5	Hollow 	1.35	0.90

TABLE 1B
Coefficient k_c for High-Lift/Performance Rudders (2009)

	Profile Type	k_c	
		Ahead Condition	Astern Condition
1	Fish tail (e.g., Schilling high-lift rudder) 	1.4	0.8
2	Flap rudder 	1.7	1.3
3	Rudder with steering nozzle 	1.9	1.5

FIGURE 1
Rudder Blade without Cutouts (2009)

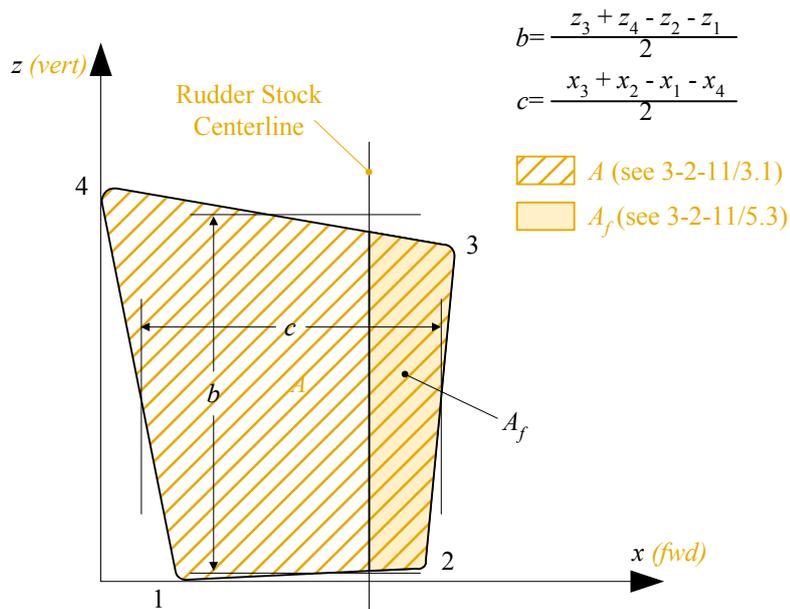
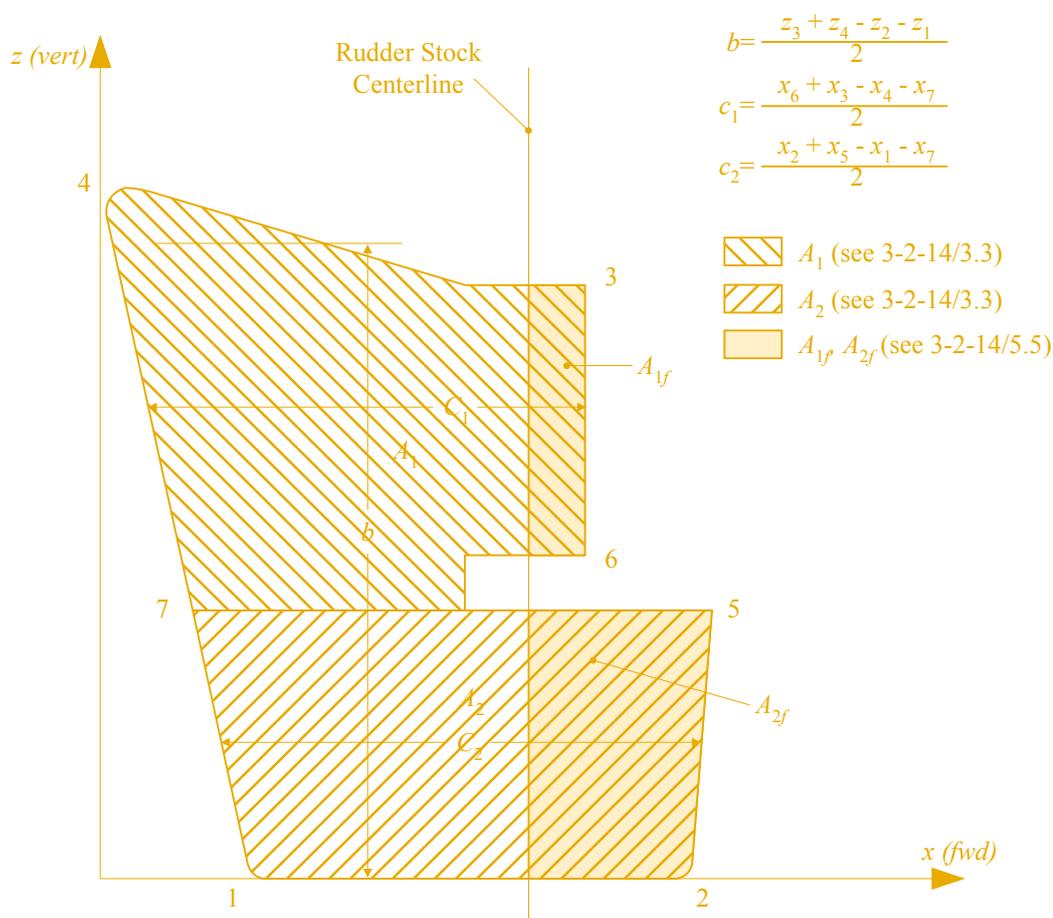


FIGURE 2
Rudder Blade with Cutouts (2009)



5 Rudder Design Torque

5.1 General

The rudder design torque, Q_R , for rudder scantling calculations is to be in accordance with 3-2-11/5.3 or 3-2-11/5.5 as applicable.

5.3 Rudder Blades without Cutouts

Rudder torque, Q_R , is to be determined from the following equation for both ahead and astern conditions.

$$Q_R = C_R r \quad \text{kN-m (tf-m, Ltf-ft)}$$

where

- C_R = rudder force, as calculated in 3-2-11/3
- r = $c(\alpha - k)$ but not less than $0.1c$ for ahead condition
- c = mean breadth of rudder area, as shown in 3-2-11/Figure 1, in m (ft)
- α = coefficient as indicated in 3-2-11/Table 3

- $k = A_f/A$
- $A_f =$ area of rudder blade situated forward of the centerline of the rudder stock, in m² (ft²), as shown in 3-2-11/Figure 1
- $A =$ whole rudder area as described in 3-2-11/3.1

TABLE 2
Coefficient k_ℓ (2009)

<i>Rudder/Propeller Layout</i>	k_ℓ
Rudders outside propeller jet	0.8
Rudders behind a fixed propeller nozzle	1.15
All others	1.0

TABLE 3
Coefficient α (2009)

<i>Rudder Position or High-lift</i>	α	
	<i>Ahead Condition</i>	<i>Astern Condition</i>
Located behind a fixed structure, such as a rudder horn	0.25	0.55
Located where no fixed structure forward of it	0.33	0.66
High-Lift Rudders (see 3-2-11/Table 1B)	Special consideration (0.40 if unknown)	Special consideration

5.5 Rudders Blades with Cutouts

This paragraph refers to rudder blades with cutouts (semi-spade rudders) as defined in 3-2-11/3.3. Equations derived in this paragraph are based on a cutout blade with two quadrilaterals. Where more quadrilaterals are needed to define the rudder shape, similar rules apply.

Total rudder torque, Q_R , in ahead and astern conditions is to be obtained from the following equation:

$$Q_R = C_{R1} r_1 + C_{R2} r_2 \quad \text{kN-m (tf-m, Ltf-ft)}$$

but not to be taken less than $Q_{R\min}$ in the ahead condition

where

$$Q_{R\min} = 0.1 C_R (A_1 c_1 + A_2 c_2) / A$$

$$r_1 = c_1 (\alpha - k_1) \quad \text{m (ft)}$$

$$r_2 = c_2 (\alpha - k_2) \quad \text{m (ft)}$$

$$c_1, c_2 = \text{mean breadth of partial area } A_1, A_2, \text{ from 3-2-11/Figure 2}$$

α = coefficient as indicated in 3-2-11/Table 3

k_1, k_2 = $A_{1f}/A_1, A_{2f}/A_2$ where A_{1f}, A_{2f} = area of rudder blade situated forward of the centerline of the rudder stock for each part of the rudder, as shown in 3-2-11/Figure 2

$C_R, C_{R1}, C_{R2}, A_1, A_2$ are as defined in 3-2-11/3.3.

5.7 Trial Conditions

The above equations for Q_R are intended for the design of rudders and should not be directly compared with the torque expected during the trial (see 3-2-11/1.5) or the rated torque of steering gear (see 4-3-3/1.5).

7 Rudder Stocks

7.1 Upper Rudder Stocks

The upper rudder stock is that part of the rudder stock above the neck bearing or above the top pintle, as the case may be.

At the upper bearing or tiller, the upper stock diameter is not to be less than that obtained from the following equation:

$$S = N_u \sqrt[3]{Q_R K_s} \quad \text{mm (in.)}$$

where

$$N_u = 42.0 \text{ (89.9, 2.39)}$$

$$Q_R = \text{total rudder torque, as defined in 3-2-11/5, in kN-m (tf-m, Ltf-ft)}$$

$$K_s = \text{material factor for upper rudder stock, as defined in 3-2-11/1.3}$$

7.3 Lower Rudder Stocks

In determining lower rudder stock scantlings, values of rudder force and torque calculated in 3-2-11/3 and 3-2-11/5 are to be used. Bending moments, shear forces, as well as the reaction forces are to be determined from 3-2-11/7.5 and 3-2-11/13.5, and are to be submitted for review. For rudders supported by shoe pieces or rudder horns, these structures are to be included in the calculation model to account for support of the rudder body. Guidance for calculation of these values is given in Appendix 3-2-A2.

The lower rudder stock diameter is not to be less than that obtained from the following equation:

$$S_\ell = S \sqrt[6]{1 + (4/3)(M/Q_R)^2} \quad \text{mm (in)}$$

where

$$S = \text{upper stock required diameter from 3-2-11/7.1, in mm (in.)}$$

$$M = \text{bending moment at the station of the rudder stock considered in kN-m (tf-m, Ltf-ft)}$$

$$Q_R = \text{rudder torque from 3-2-11/5, in kN-m (tf-m, Ltf-ft)}$$

Above the neck bearing, a gradual transition is to be provided where there is a change in the diameter of the rudder stock.

7.5 Bending Moments

The bending moment on the rudder and rudder stock may be determined in accordance with Appendix 3-2-A2 or in accordance with the following equations.

7.5.1 Spade Rudders

$$M_n = C_R \ell_n \quad \text{kN-m (Ltf-ft)}$$

$$M_s = C_R \frac{A_1}{A} \ell_c \quad \text{kN-m (Ltf-ft)}$$

where

M_n = bending moment at neck bearing

M_s = bending moment at section under consideration

ℓ_n = distance from center of neck bearing to the centroid of rudder area, m (ft)

ℓ_c = distance from section under consideration to the centroid of rudder area, A_1 , m² (ft²)

A_1 = area below section under consideration, m² (ft²)

C_R and A are as defined in 3-2-11/3.

7.5.2 Balanced Rudders with Shoepiece Support

The bending moment at the neck bearing may be taken as indicated below. Bending moments at other locations are to be determined by direct calculation and are to be submitted. See Appendix 3-2-A2 for guidance in calculating bending moments.

$$M_n = NC_R \ell_b \quad \text{kN-m (Ltf-ft)}$$

where

M_n = bending moment at neck bearing

ℓ_b = distance between center of neck bearing and center of shoepiece pintle bearing, m (ft)

$$N = \left[\frac{0.5 + \frac{\alpha_1}{8}}{1 + \alpha_1 \left(1 + \frac{\ell_u I_b}{\ell_b I_u} \right)} \right]$$

$$\alpha_1 = \frac{\ell_b^3 I_d}{\ell_s^3 I_b}$$

I_d = mean moment of inertia of shoepiece about the vertical axis, cm⁴ (in⁴)

ℓ_s = distance between center of shoepiece pintle bearing and the effective support point of the shoepiece in the hull, m (ft)

I_b = mean moment of inertia of the rudder, cm⁴ (in⁴), considering a width of rudder plating twice the athwartship dimension of the rudder and excluding welded or bolted cover plates for access to pintles, inc.

ℓ_u = distance between center of the neck bearing and the center of the rudder carrier bearing, m (ft)

I_u = mean moment of inertia of rudder stock, between neck bearing and rudder carrier bearing, cm⁴, (in⁴)

C_R is as defined in 3-2-11/3.

9 Flange Couplings

9.1 General

Rudder flange couplings are to comply with the following requirements:

- i) Couplings are to be supported by an ample body of metal worked out from the rudder stock.
- ii) The smallest distance from the edge of the bolt holes to the edge of the flange is not to be less than two-thirds of the bolt diameter.
- iii) Coupling bolts are to be fitted bolts.
- iv) Suitable means are to be provided for locking the nuts in place.

In addition to the above, rudder flange couplings are to meet the type-specific requirements in 3-2-11/9.3 (horizontal couplings) or 3-2-11/9.5 (vertical couplings) as applicable.

9.3 Horizontal Couplings

9.3.1 Coupling Bolts

There are to be at least six coupling bolts in horizontal couplings, and the diameter, d_b , of each bolt is not to be less than that obtained by the following equation:

$$d_b = 0.62 \sqrt{d_s^3 K_b / (nr K_s)} \quad \text{mm (in.)}$$

where

- d_s = required rudder stock diameter, S (3-2-11/7.1) or S_ℓ (3-2-11/7.3) as applicable, in way of the coupling
- n = total number of bolts in the horizontal coupling
- r = mean distance, in mm (in.), of the bolt axes from the center of the bolt system
- K_b = material factor for bolts, as defined in 3-2-11/1.3
- K_s = material factor for stock, as defined in 3-2-11/1.3

9.3.2 Coupling Flange

Coupling flange thickness is not to be less than the greater of the following equations:

$$t_f = d_{bt} \sqrt{K_f / (K_b)} \quad \text{mm (in.)}$$

$$t_f = 0.9 d_{bt} \quad \text{mm (in.)}$$

where

- d_{bt} = calculated bolt diameter as per 3-2-11/9.3.1 based on a number of bolts not exceeding 8
- K_f = material factor for flange, as defined in 3-2-11/1.3
- K_b = material factor of bolts, as defined in 3-2-11/1.3

9.5 Vertical Couplings

9.5.1 Coupling Bolts

There are to be at least eight coupling bolts in vertical couplings and the diameter, d_b , of each bolt is not to be less than that obtained from the following equation:

$$d_b = 0.81d_s \sqrt{K_b / (nK_s)} \quad \text{mm (in.)}$$

where

n = total number of bolts in the vertical coupling

d_s , K_b , K_s are as defined in 3-2-11/9.3.

In addition, the first moment of area, m , of the bolts about the center of the coupling is not to be less than that given by the following equation:

$$m = 0.00043d_s^3 \quad \text{mm}^3 \text{ (in}^3\text{)}$$

where

d_s = diameter, in mm (in.), as defined in 3-2-11/9.3

9.5.2 Coupling Flange

Coupling flange thickness is not to be less than d_b , as defined in 3-2-11/9.5.1.

11 Tapered Stock Couplings

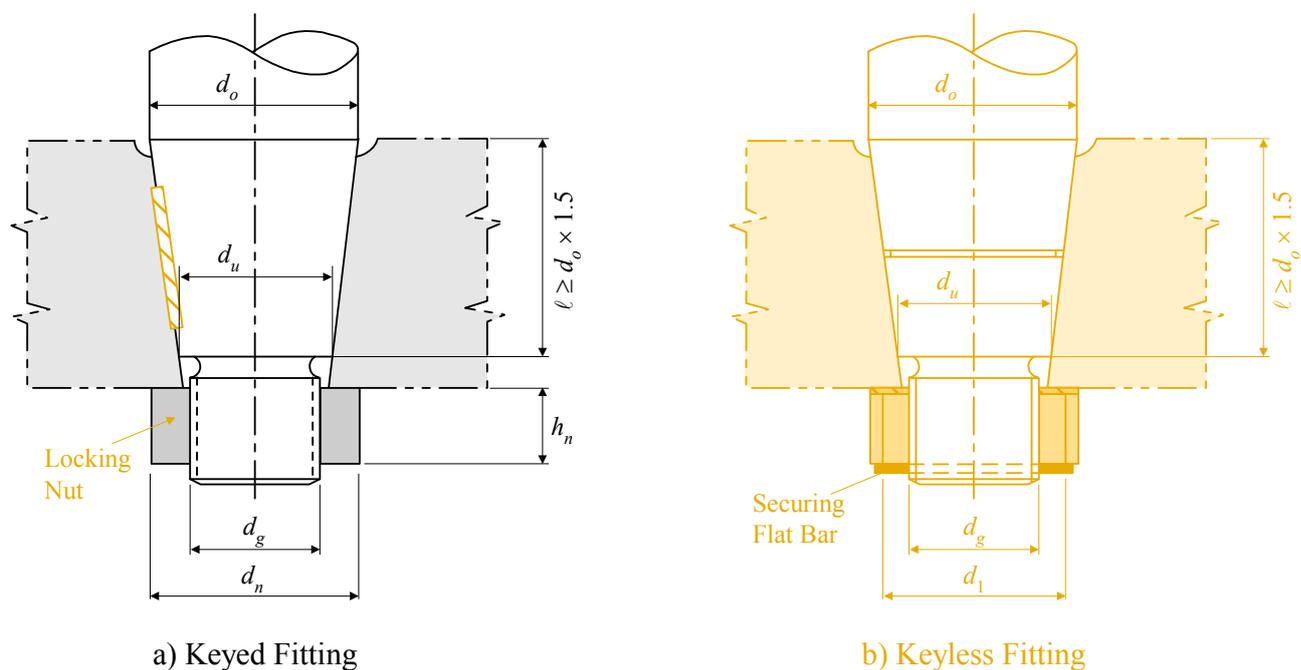
11.1 Coupling Taper

Tapered stock couplings are to comply with the following general requirements in addition to type-specific requirements given in 3-2-11/11.3 or 3-2-11/11.5 as applicable:

- i) Tapered stocks, as shown in 3-2-11/Figure 3, are to be effectively secured to the rudder casting by a nut on the end.
- ii) Taper length (ℓ) in the casting is generally not to be less than 1.5 times the stock diameter (d_o) at the top of the rudder.
- iii) The taper on diameter (c) is to be 1/12 to 1/8 for keyed taper couplings and 1/20 to 1/12 for couplings with hydraulic mounting/dismounting arrangements, as shown in the following table.
- iv) Where mounting with an oil injection and hydraulic nut, the push-up oil pressure and the push-up length are to be specially considered upon submission of calculations.

Type of Coupling Assembly	$c = \frac{d_o - d_u}{\ell}$
Without hydraulic mounting/dismounting	$1/12 \leq c \leq 1/8$
With hydraulic mounting/dismounting	$1/20 \leq c \leq 1/12$

FIGURE 3
Tapered Couplings (2009)



11.3 Keyed Fitting

Where the stock is keyed to the rudder casting, it is to be fitted in accordance with the following:

- i) The top of the keyway is to be located well below the top of the rudder.
- ii) Torsional strength of the key equivalent to that of the required upper stock is to be provided.
- iii) For higher strength materials, shear and bearing areas of keys and keyways are to be based on the respective material properties of the key and the keyways.

11.5 Keyless Fitting

Hydraulic and shrink fit keyless couplings are to be fitted in accordance with the following:

- i) Hydraulic pressure is to be specially considered upon submittal of detailed preloading stress calculations and fitting instructions;
- ii) The calculated torsional holding capacity is to be at least 2.0 times the transmitted torque based on the steering gear relief valve setting;
- iii) Preload stress is not to exceed 70% of the minimum yield strength of either the stock or the bore;
- iv) Prior to applying hydraulic pressure, at least 75% of theoretical contact area of rudder stock and rudder bore is to be achieved in an evenly distributed manner;
- v) The upper edge of the upper main piece bore is to have a slight radius;
- vi) The locking nut is to be fitted in accordance with 3-2-11/11.7.

11.7 Locking Nut

Dimensions of the securing nut, as shown in 3-2-11/Figure 3, are to be proportioned in accordance with the following and the nut is to be fitted with an effective locking device.

Height	$h_n \geq 0.6d_g$
Outer diameter of nut	$d_n \geq 1.2d_u$ or $1.5d_g$ whichever is greater
External thread diameter	$d_g \geq 0.65d_o$

In the case of a hydraulic pressure secured nut, a securing device such as a securing flat bar is to be provided. Calculations proving the effectiveness of the securing device are to be submitted.

13 Pintles

13.1 General

- i) Pintles are to be fitted in the gudgeons by conical attachment to the full extent of the gudgeon depth
- ii) The depth of the pintle boss is not to be less than the required pintle diameter d_p , and bearing length is to be between 1.0 and 1.2 times d_p .
- iii) The taper on the diameter is to be:
 1/12 to 1/8 for keyed and other manually assembled pintles with locking nut.
 1/20 to 1/12 for pintle mounted with oil injection and hydraulic nut.
- iv) Threads and nuts are to be in accordance with 3-2-11/11.7.
- v) For rudders on horns with two pintles, as shown in 3-2-13/Figure 3b of the *Steel Vessel Rules*, calculations are to include pintle bearing forces with the vessel running ahead at the maximum continuous rated shaft rpm and at the lightest operating draft.
- vi) The bearing allowable pressure is to be in accordance with 3-2-11/Table 6.

13.3 Diameter

The diameter of the pintles is not to be less than that obtained from the following equation.

$$d_p = k_1 \sqrt{BK_p} \quad \text{mm (in.)}$$

where

k_1	=	11.1 (34.7, 1.38)
B	=	bearing force, in kN (tf, Ltf), from 3-2-11/13.5 but not to be taken less than B_{\min} as specified in 3-2-11/Table 4
K_p	=	material factor for the pintle, as defined in 3-2-11/1.3

TABLE 4
Minimum Bearing Force B_{\min} (2009)

<i>Pintle Type</i>		B_{\min}
Conventional two pintle rudder		$0.5 C_R$
3-2-A2/Figure 2	lower pintle	$0.5 C_R$
3-2-A2/Figure 3	main pintle	$C_R \ell_a / \ell_p^*$
3-2-13/Figure 3 of the <i>Steel Vessel Rules</i>	main pintle	$C_R \ell_a / \ell_p^*$
	upper pintle	$0.25 C_R$

* $B_{\min} = C_R$ where $\ell_a / \ell_p \geq 1$

ℓ_a, ℓ_p as described in 3-2-13/Figure 3 of the *Steel Vessel Rules*

13.5 Shear and Bearing Forces

The shear and bearing forces may be determined in accordance with Appendix 3-2-A2 or by the equations given below.

13.5.1 Spade Rudder

$$\text{Bearing force at rudder carrier: } P_u = \frac{M_n}{\ell_u} \quad \text{kN (tf Ltf)}$$

$$\text{Bearing force at neck bearing: } P_n = C_R + P_u \quad \text{kN (tf Ltf)}$$

$$\text{Shear force at neck bearing: } F_n = C_R \quad \text{kN (tf Ltf)}$$

where C_R is as defined in 3-2-11/3 and ℓ_u is as defined in 3-2-11/7.5.2.

13.5.2 Balanced Rudder with Shoepiece Support

$$\text{Bearing force at rudder carrier: } P_u = \frac{M_n}{\ell_u} \quad \text{kN (tf Ltf)}$$

$$\text{Bearing force at neck bearing: } P_n = P_u \left(1 + \frac{\ell_u}{\ell_b} \right) + \frac{C_R}{\ell_b} \left(\frac{\ell_R}{2} + \ell_p \right) \quad \text{kN (tf Ltf)}$$

where

- ℓ_b = distance between the center of neck bearing support and the center of shoepiece support, as shown in 3-2-A2/Figure 2
- = $\ell_p + \ell_r + \ell_\ell$
- ℓ_p = distance between bottom of rudder blade and center of support of neck bearing
- ℓ_ℓ = distance between top of rudder blade and center of support of neck bearing

$$\text{Bearing force at shoepiece: } P_p = C_R + P_u - P_n \quad \text{kN (tf, Ltf)}$$

but not less than $0.5C_R$

$$\text{Shear force at neck bearing: } F_n = P_n - P_u \quad \text{kN (tf, Ltf)}$$

where C_R is as defined in 3-2-11/3.

15 Supporting and Anti-Lifting Arrangements

15.1 Rudder Stock and Pintle Bearings

15.1.1 Bearing Surfaces

Bearing surfaces for rudder stocks, shafts and pintles are to meet the following requirements:

- i) The length/diameter ratio (ℓ_b/d_ℓ) of the bearing surface is not to be greater than 1.2
- ii) The projected area of the bearing surface ($A_b = d_\ell \ell_b$) is not to be taken less than A_{bmin} ,

where

d_ℓ = outer diameter of the liner, in mm (in.)

ℓ_b = bearing length, in mm (in.)

$$A_{bmin} = k_1 \frac{P}{q_a} \quad \text{mm}^2 \text{ (in}^2\text{)}$$

k_1 = 1000 (2240)

P = bearing reaction force, in kN (tf, Ltf), as determined from 3-2-11/Table 5

q_a = allowable surface pressure, as indicated in 3-2-11/Table 6, depending on bearing material, in N/mm² (kgf/mm², psi)

TABLE 5
Bearing Reaction Force (2009)

<i>Bearing Type</i>	<i>P, Bearing Reaction Force kN (tf, Ltf)</i>
Pintle bearings	$P = B$ as defined in 3-2-11/13
Other bearings	Calculation of P is to be submitted. Guidelines for calculation can be found in Appendix 3-2-A2

TABLE 6
Bearing Pressure

<i>Bearing Material</i>	q_a †		
	<i>N/mm²</i>	<i>kgf/mm²</i>	<i>psi</i>
lignum vitae	2.5	0.25	360
white metal, oil lubricated	4.5	0.46	650
synthetic material with hardness between 60 and 70 Shore D *	5.5	0.56	800
steel § and bronze and hot-pressed bronze-graphite materials	7.0	0.71	1000

† Higher values than given in the table may be taken if they are verified by tests.

§ Stainless and wear-resistant steel in an approved combination with stock liner.

* Indentation hardness test at 23°C and with 50% moisture, according to a recognized standard. Synthetic bearing materials to be of approved type.

15.1.2 Bearing Clearance

With metal bearings, clearance is not to be less than $d_f/1000 + 1.0$ mm, ($d_f/1000 + 0.04$ in.) on the diameter. If non-metallic bearing material is applied, the bearing clearance is to be specially determined considering the material's swelling and thermal expansion properties. This clearance is in no case to be taken less than 1.5 mm (0.06 in.) on diameter.

15.3 Rudder Carrier

The weight of the rudder assembly is to be supported by a rudder carrier **mounted on the hull structure** designed for that purpose. At least half of the rudder carrier holding-down bolts are to be fitted bolts. Alternative means of preventing horizontal movement of the rudder carrier may be considered.

15.5 Anti-Lifting Devices

Means are to be provided to prevent accidental unshipping or undue movement of the rudder which may cause damage to the steering gear. There are to be at least two bolts in the joint of the anti-lifting ring.

17 Double Plate Rudder

17.1 Strength

The section modulus and web area of the rudder mainpiece are to be such that the stresses **indicated in the following Subparagraphs** are not exceeded.

In calculating the section modulus of the rudder, the effective width of side plating is to be taken as not greater than twice the athwartship dimension of the rudder. Welded or bolted cover plates on access openings to pintles are not to be considered effective in determining the section modulus of the rudder. Generous radii are to be provided at abrupt changes in section **where there are stress concentrations, including** in way of openings **and** cover plates.

Moments, **shear forces** and reaction forces are to be as given in 3-2-11/7.5 and 3-2-11/13.5.

17.1.1 Clear of Cut-outs

Allowable stresses for determining the rudder strength clear of cutouts are as follows:

Bending stress $\sigma_b = K_b/Q$ N/mm² (kgf/mm², psi)

Shear stress $\tau = K_\tau/Q$ N/mm² (kgf/mm², psi)

Equivalent stress $\sigma_e = \sqrt{\sigma_b^2 + 3\tau^2} = K_e/Q$ N/mm² (kgf/mm², psi)

where

	<i>SI units</i>	<i>MKS units</i>	<i>US units</i>
K_σ	110	11.2	15,900
K_τ	50	5.1	7,300
K_e	120	12.2	17,400

$Q = 1.0$ for ordinary strength hull steel

$=$ as defined in 3-2-1/7.5 for higher strength steel plate

17.1.2 In Way of Cutouts

Allowable stresses for determining the rudder strength in way of cutouts (see 3-2-11/Figure 4) are as follows:

Bending stress $\sigma_b = K_\sigma/Q$ N/mm² (kgf/mm², psi)

Shear stress $\tau = K_\tau/Q$ N/mm² (kgf/mm², psi)

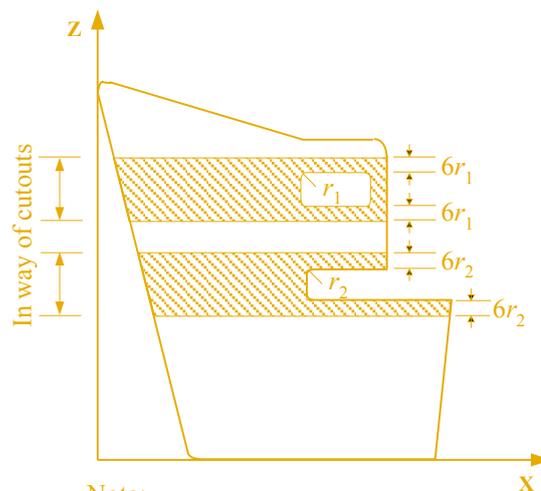
Equivalent stress $\sigma_e = \sqrt{\sigma_b^2 + 3\tau^2} = K_e/Q$ N/mm² (kgf/mm², psi)

where

	SI units	MKS units	US units
K_σ	75	7.65	10,900
K_τ	50	5.1	7,300
K_e	100	10.2	14,500

Q = 1.0 for ordinary strength hull steel
 = as defined in 3-2-1/7.5 for higher strength steel plate

FIGURE 4 (2009)



Note:
 r_1 = corner radius of rudder plate in way of portable bolted inspection hole
 r_2 = corner radius of rudder plate

The mainpiece of the rudder is to be formed by the rudder side plating (but not more than the effective width indicated above) and vertical diaphragms extending the length of the rudder or the extension of the rudder stock or a combination of both.

For spade rudders, the section modulus at the bottom of the rudder is not to be less than one-third required section modulus of the rudder at the top of the rudder or at the center of the lowest pintle.

Where rudders have an unsymmetrical foil section (e.g., reaction rudder) details of the rudder are to be submitted.

Special attention is to be paid in design and construction of rudders with slender foil sections in the vicinity of their trailing edge (e.g., hollow foil sections, fishtail foil sections). Where the width of the rudder blade at the aftermost vertical diaphragm is equal or less than 1/6 of the trailing edge length measured between the diaphragm and the trailing edge, vibration analysis of the rudder blade is also to be submitted for review.

17.3 Side, Top and Bottom Plating

The plating thickness is not to be less than that obtained from the following equation:

$$t = 0.0055s\beta \sqrt{k_1 d + (k_2 C_R / A)} \times \sqrt{Q} + k_3$$

where

Q	=	1.0 for ordinary strength hull steel
	=	as defined in 3-2-1/7.5 for higher strength steel plate
k_1	=	1.0 (1.0, 0.305)
k_2	=	0.1 (0.981, 10.7)
k_3	=	2.5 (2.5, 0.1)
d	=	summer load line draft of the vessel, in m (ft)
C_R	=	rudder force according to 3-2-1 1/3, in kN (tf, Ltf)
A	=	rudder area, in m ² (ft ²)
β	=	$\sqrt{1.1 - 0.5(s/b)^2}$; maximum 1.0 for $b/s \geq 2.5$
s	=	smaller unsupported dimension of plating, in mm (in.)
b	=	greater unsupported dimension of plating, in mm (in.)

The thickness of the rudder side or bottom plating is to be at least 2 mm (0.08 in.) greater than that required by 3-2-8/5 for deep tank plating in association with a head, h , measured to the summer load line.

17.5 Diaphragm Plates

Vertical and horizontal diaphragms are to be fitted within the rudder, effectively attached to each other and to the side plating. Vertical diaphragms are to be spaced approximately 1.5 times the spacing of horizontal diaphragms.

The thickness of diaphragm plates is not to be less than 70% of the required rudder side plate thickness or 8 mm (0.31 in.), whichever is greater. **Openings in diaphragms are not to exceed one half their depth.**

Welding is to be in accordance with **Section 2-4-1** of the *ABS Rules for Materials and Welding (Part 2)* and Section 3-2-16 of these Rules. Where inaccessible for welding inside the rudder, it is recommended that diaphragms be fitted with flat bars and the side plating be connected to these flat bars by continuous welds or by 75 mm (3 in.) slot welds spaced at 150 mm (6 in.) centers. The slots are to be fillet welded around the edges and filled with a suitable compound.

17.7 Watertightness

The rudder is to be watertight and is to be tested in accordance with 3-7-1/Table 1.

19 Single Plate Rudders

19.1 Mainpiece Diameter

The mainpiece diameter is calculated according to 3-2-11/7.3. For spade rudders, the lower third may be tapered down to 0.75 times stock diameter at the bottom of the rudder.

19.3 Blade Thickness

The blade thickness is not to be less than that obtained from the following equation:

$$t_b = 0.0015sV_R + 2.5 \quad \text{mm}$$

$$t_b = 0.0015sV_R + 0.1 \quad \text{in.}$$

where

s = spacing of stiffening arms, in mm (in.), not to exceed 1000 mm (39 in.)

V_R = speed, as defined in 3-2-11/3

19.5 Arms

The thickness of the arms is not to be less than the blade thickness obtained in 3-2-11/19.3. The section modulus of each set of arms about the axis of the rudder stock is not to be less than that obtained from the following equation:

$$SM = 0.0005sC_1^2V_R^2Q \quad \text{cm}^3$$

$$SM = 0.0000719sC_1^2V_R^2Q \quad \text{in}^3$$

where

C_1 = horizontal distance from the aft edge of the rudder to the centerline of the rudder stock, in m (ft)

s and V_R are as defined in 3-2-11/19.3.

Q is as defined in 3-2-11/17.3.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

APPENDIX **2 Guidelines for Calculating Bending Moment and Shear Force in Rudders and Rudder Stocks**

1 Application

Bending moments, shear forces and reaction forces of rudders, stocks and bearings may be calculated according to this Appendix for the types of rudders indicated. Moments and forces on rudders of different types or shapes than those shown are to be calculated using alternative methods and will be specially considered.

3 Spade Rudders

3.1 Rudder

3.1.1 Shear Force

Lateral shear force, $V(z)$, at a horizontal section of the rudder z meters (feet) above the bottom of ℓ_R is given by the following equation:

$$V(z) = \frac{zC_R}{A} \left[c_\ell + \frac{z}{2\ell_R} (c_u - c_\ell) \right] \quad \text{kN (tf, Ltf)}$$

where

z = distance from the bottom of ℓ_R to the horizontal section under consideration, in m (ft)

C_R = rudder force, as defined in 3-2-11/3, in kN (tf, Ltf)

A = rudder blade area, in m^2 (ft^2)

c_ℓ , c_u and ℓ_R are dimensions as indicated in 3-2-A2/Figure 1, in m (ft).

3.1.2 Bending Moment

Bending moment, $M(z)$, at a horizontal section z meters (feet) above the baseline of the rudder is given by the following equation:

$$M(z) = \frac{z^2 C_R}{2A} \left[c_\ell + \frac{z}{3\ell_R} (c_u - c_\ell) \right] \quad \text{kN-m, (tf-m, Ltf-ft)}$$

where z , C_R , A , c_ℓ , c_u and ℓ_R are as defined in 3-2-A2/3.1.1.

3.3 Lower Stock

3.3.1 Shear Force

Lateral shear force, V_ℓ , at any section of the lower stock between the top of the rudder and the neck bearing, is given by the following equation:

$$V_\ell = C_R \quad \text{kN (tf, Ltf)}$$

where C_R is as defined in 3-2-11/3.

3.3.2 Bending Moment at Neck Bearing

The bending moment in the rudder stock at the neck bearing, M_n , is given by the following equation:

$$M_n = C_R \left[\ell_\ell + \frac{\ell_R (2c_\ell + c_u)}{3(c_\ell + c_u)} \right] \quad \text{kN-m (tf-m, Ltf-ft)}$$

where

$$C_R = \text{rudder force, as defined in 3-2-11/3}$$

c_ℓ , c_u , ℓ_ℓ and ℓ_R are dimensions, as indicated in 3-2-A2/Figure 1, in m (ft).

3.5 Moment at Top of Upper Stock Taper

The bending moment in the upper rudder stock at the top of the taper, M_t , is given by the following equation:

$$M_t = C_R \left[\ell_\ell + \frac{\ell_R (2c_\ell + c_u)}{3(c_\ell + c_u)} \right] \times \left[\frac{(\ell_u + \ell_R + \ell_\ell - z_t)}{\ell_u} \right] \quad \text{kN-m (tf-m, Ltf-ft)}$$

where

$$z_t = \text{distance from the rudder baseline to the top of the upper rudder stock taper, in m (ft)}$$

$$C_R = \text{rudder force, as defined in 3-2-11/3}$$

c_ℓ , c_u , ℓ_ℓ , ℓ_u and ℓ_R are dimensions as indicated in 3-2-A2/Figure 1, in m (ft).

3.7 Bearing Reaction Forces

Reaction forces at the bearings are given by the following equations:

$$P_u = \text{reaction force at the upper bearing}$$

$$= - \frac{M_n}{\ell_u} \quad \text{kN (tf, Ltf)}$$

$$P_n = \text{reaction force at the neck bearing}$$

$$= C_R + \frac{M_n}{\ell_u} \quad \text{kN (tf, Ltf)}$$

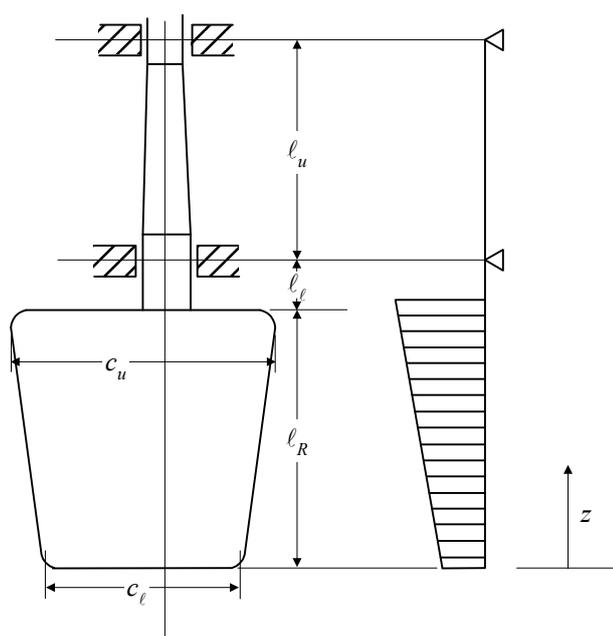
where

M_n = bending moment at the neck bearing, as defined in 3-2-A2/3.3.2

C_R = rudder force, as defined in 3-2-11/3.

ℓ_u is as indicated in 3-2-A2/Figure 1, in m (ft).

**FIGURE 1
Spade Rudder**



5 Rudders Supported by Shoepiece

5.1 Shear Force, Bending Moment and Reaction Forces

Shear force, bending moment and reaction forces may be calculated according to the model given in 3-2-A2/Figure 2.

$$w_R = \text{rudder load per unit length}$$

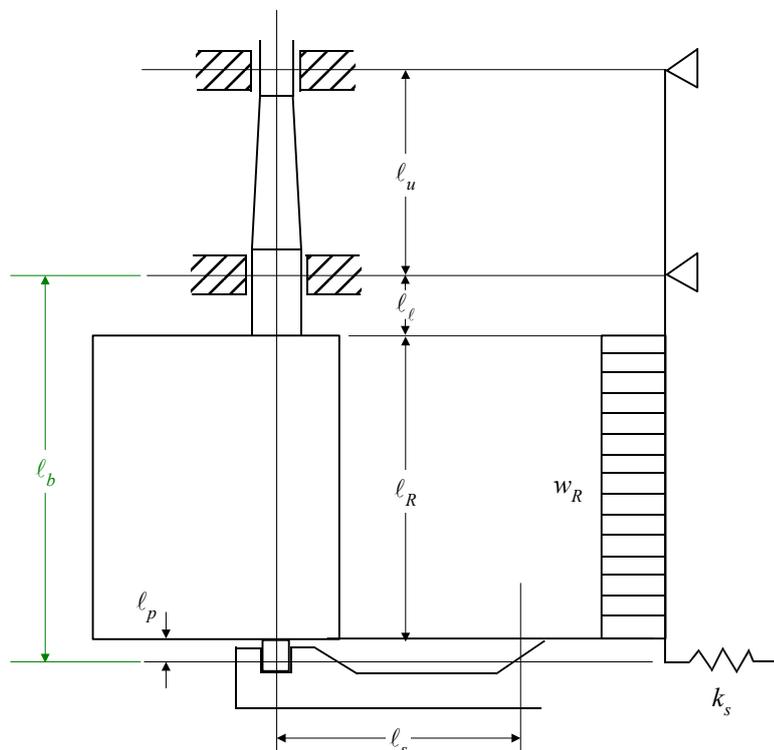
$$= \frac{C_R}{\ell_R} \quad \text{kN/m (tf/m, Ltf/ft)}$$

where

- C_R = rudder force, as defined in 3-2-11/3
- k_s = spring constant reflecting support of the shoepiece
 - = $\frac{n_s I_s}{\ell_s^3}$ kN/m (tf/m, Ltf/ft)
- n_s = 6.18 (0.630, 279)
- I_s = moment of inertia of shoepiece about the vertical axis, in cm^4 (in^4)
- I_u = moment of inertia of the rudder stock above the neck bearing, in cm^4 (in^4)
- I_ℓ = moment of inertia of the rudder stock below the neck bearing, in cm^4 (in^4)
- I_R = moment of inertia of the rudder about the longitudinal axis, in cm^4 (in^4)
- I_p = moment of inertia of the pintle, in cm^4 (in^4)

ℓ_ℓ , ℓ_s , ℓ_R and ℓ_u are dimensions as indicated in 3-2-A2/Figure 2, in m (ft).

FIGURE 2
Rudder Supported by Shoepiece



7 Rudders Supported by a Horn with One Pintle (2009)

7.1 Shear Force, Bending Moment and Reaction Forces

Shear force, bending moment and reaction forces are to be assessed by the simplified beam model shown in 3-2-A2/Figure 3.

w_{R1} = rudder load per unit length above pintle

$$= \frac{C_{R1}}{\ell_{R1}} \quad \text{kN/m (tf/m, Ltf/ft)}$$

w_{R2} = rudder load per unit length below pintle

$$= \frac{C_{R2}}{\ell_{R2}} \quad \text{kN/m (tf/m, Ltf/ft)}$$

where

C_{R1} = rudder force, as defined in 3-2-11/3.3

C_{R2} = rudder force, as defined in 3-2-11/3.3

k_h = spring constant reflecting support of the horn

$$= \frac{1}{\frac{\ell_h^3}{n_b I_h} + \frac{\sum \left(\frac{s_i}{t_i} \right) e^2 \ell_h}{n_t a^2}} \quad \text{kN/m (tf/m, Ltf/ft)}$$

n_b = 4.75 (0.485, 215)

n_t = 3.17 (0.323, 143)

a = mean area enclosed by the outside lines of the rudder horn, in cm² (in²)

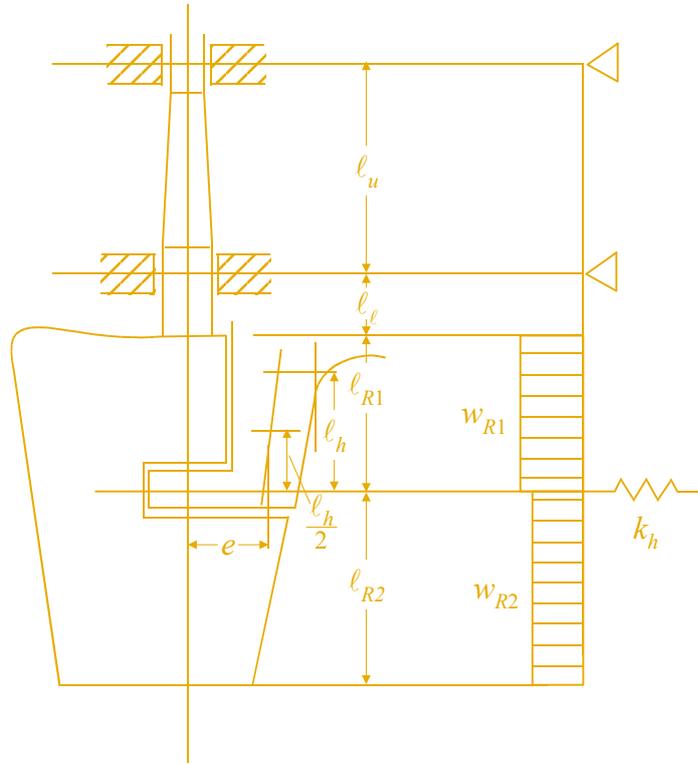
s_i = the girth length of each segment of the horn of thickness t_i , in cm (in.)

t_i = the thickness of each segment of horn outer shell of length s_i , in cm (in.)

I_h = moment of inertia of horn section at ℓ_h about the longitudinal axis, in cm⁴ (in⁴)

e , ℓ_h , ℓ_{R1} and ℓ_{R2} are dimensions as indicated in 3-2-A2/Figure 3, in m (ft).

FIGURE 3
Rudder Supported by a Horn with One Pintle (2009)



PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **12 Protection of Deck Openings**

1 General

All openings in decks are to be framed to provide efficient support and attachment for the ends of the deck beams. The proposed arrangement and details for all hatchways are to be submitted for approval.

3 Additional Requirements for Bulk Carriers, Ore Carriers and Combination Carriers (2004)

On all bulk carriers, ore carriers and combination carriers, in addition to all relevant requirements in this Section, all cargo hold hatch covers, hatch coamings and closing arrangements for cargo hold hatches in position 1, as defined in 3-2-12/5.1, are to meet the requirements in 5C-4-2/13.3 of the *Steel Vessel Rules*.

5 Positions and Design Pressures (1 January 2005)

5.1 Positions of Deck Openings (1 January 2005)

For the purpose of the Rules, two positions of deck openings are defined as follows.

Position 1 Upon exposed freeboard and raised quarter decks, and upon exposed superstructure decks situated forward of a point located $L_f/4$ from the forward end of L_f .

Position 2 Upon exposed superstructure decks situated abaft $L_f/4$ from the forward end of L_f and located at least one standard height of superstructure above the freeboard deck. Upon exposed superstructures decks situated forward of a point located $L_f/4$ from the forward end of L_f and located at least two standards heights of superstructure above the freeboard deck.

5.3 Design Pressures (1 January 2005)

The design pressures are not to be taken as less than the following. Values at intermediate lengths are to be determined by interpolation.

5.3.1 Cargo Hatch Covers in Position 1

$$p = R \{ 15.8 + (L_f/N)[1 - (5/3)(x/L_f)] - 3.6x/L_f \} \quad \text{kN/m}^2 \text{ (tf/m}^2, \text{ Ltf/ft}^2)$$

For a position 1 hatchway located at least one superstructure standard height higher than the freeboard deck:

$$p = 3.3 - 1.3x \frac{(90 - L_{f1})}{66} \quad \text{kN/m}^2 \text{ (tf/m}^2, \text{ Ltf/ft}^2)$$

where

- p_{FP} = pressure at the forward perpendicular
- = $23.8 + a(L_f - 24)$ kN/m² for L_f in meters
- = $2.43 + a(L_f - 24)$ tf/m² for L_f in meters
- = $0.222 + a(L_f - 79)$ Ltf/ft² for L_f in feet
- a = 0.332 (0.0338, 0.000942) kN/m² (tf/m², Ltf/ft²), for type B freeboard ships
- L_f = freeboard length, in m (ft), as defined in 3-1-1/3.3
- L_{f1} = freeboard length, in m (ft), as defined in 3-1-1/3.3, but is not to be taken as greater than 90 m (295 ft) and less than 24 m (80 ft)
- x = distance, in m (ft), from the mid length of the hatch cover under examination to the forward end of L_f or $0.25L_f$, whichever is less.
- R = 1.0 (0.102, 0.00932)
- N = 3 (3, 9.84)

5.3.2 Cargo Hatch Covers in Position 2

Where a position 2 hatchway is located at least one superstructure standard height higher than the freeboard deck, the design pressures are as follows:

$$\begin{aligned} p &= 25.5 - 0.142(100 - L_f) && \text{kN/m}^2 \\ &= 2.6 - 0.0145(100 - L_f) && \text{tf/m}^2 \\ &= 0.238 - 0.00041(328 - L_f) && \text{Ltf/ft}^2 \end{aligned}$$

7 Hatchway Coamings, Companionway Sills and Access Sills

7.1 Coaming and Sill Heights

The heights above deck of coamings of hatchways secured weathertight by tarpaulins and battening devices, and sills of companionways and access openings, are to be not less than given in 3-2-12/Table 1. Where hatch covers are made of steel or other equivalent material and made tight by means of gaskets and clamping devices, these heights may be reduced or the coamings omitted entirely, provided that the safety of the vessel is not thereby impaired in any sea conditions. Sealing arrangements are to be weathertight if coaming is fitted, and watertight for flush covers.

TABLE 1
Coaming and Sill Heights

L equal to or over 24 meters (79 feet) in length

	<i>Position 1</i>	<i>Position 2</i>
Hatch Coamings	600 mm (23.5 in.)	450 mm (17.5 in.)
Companionway Sills	600 mm (23.5 in.)	380 mm (15 in.)
Access Sills	380 mm (15 in.)	380 mm (15 in.)

L under 24 meters (79 feet) in length

	<i>Position 1</i>	<i>Position 2</i>
Hatch Coamings and Companionways	450 mm (17.5 in.)	300 mm (12 in.)
Access Sills	380 mm (15 in.)	300 mm (12 in.)

Notes:

- 1 Coaming and sill heights may be reduced on vessels which have freeboard in excess of the minimum geometric freeboard and/or a superstructure deck with height of deck in excess of the standard height of a superstructure.
- 2 For vessels with $L < 24$ m (79 ft), the coaming/sill height should be as indicated above, unless otherwise specifically requested by Flag Administration.

7.3 Coaming Plates

Coaming plates are not to be less in thickness than that obtained from the following equation:

$$t = 0.05L + 7 \text{ mm}$$

$$t = 0.0006L + 0.27 \text{ in.}$$

where

t = thickness, in mm (in.)

L = length of vessel, in m (ft), as defined in 3-1-1/3, but need not exceed 76 m (250 ft)

7.5 Coaming Stiffeners

Horizontal stiffeners are to be fitted on coamings 450 mm (17.5 in.) or greater in height. The breadth of the stiffeners is not to be less than that obtained from the following equation:

$$b = 1.67L + 50 \text{ mm}$$

$$b = 0.02L + 2 \text{ in.}$$

where

b = breadth, in mm (in.)

L = length of vessel, in m (ft), as defined in 3-1-1/3, but need not exceed 76 m (250 ft)

Efficient brackets or stays are to be fitted from the stiffeners to the deck at intervals of not more than 3 m (10 ft).

Where exposed coamings are 760 mm (30 in.) or more in height, the arrangement of the stiffeners and brackets or stays is to provide equivalent support. Where end coamings are protected, the arrangement of the stiffeners and brackets or stays may be modified.

Where chocks are provided on the coaming to limit the horizontal movement of hatch covers, the strength of the coaming and deck structure is to be adequate to withstand the load on these chocks. Similar consideration is to be given to pads supporting the weight from hatch covers.

7.7 Continuous Longitudinal Hatch Coamings

Where strength deck longitudinal hatch coamings of length greater than $0.14L$ are effectively supported by longitudinal bulkheads or deep girders, they are, in general, to be longitudinally stiffened. The coaming thickness is to be not less than required by 3-2-3/3, and the longitudinal stiffeners not less than required by 3-2-6/1.3 for strength deck longitudinal beams where s is the stiffener spacing, ℓ is the distance between coaming brackets, and h is as given in 3-2-6/1.3.2. Special consideration will be given to the coaming scantlings where adequate buckling strength is shown to be otherwise provided.

9 Hatchways Closed by Portable Covers and Secured Weathertight by Tarpaulins and Battening Devices

9.1 Pontoon Covers

9.1.1 Scantlings (*1 January 2005*)

Where steel pontoon covers are used, the maximum allowable stress and deflection under the design pressures in 3-2-12/5.3, and the minimum required top plate thickness are as follows.

Maximum allowable stress	$0.68Y$
Maximum allowable deflection	0.0044 times the span
Top plate thickness	$0.01s$, but not less than 6 mm (0.24 in.)

where

Y	=	specified minimum upper yield point strength of the materials, in N/mm ² (kgf/mm ² , psi)
s	=	stiffener spacing, in mm (in.)

Covers are to be assumed to be simply supported.

Where the cross section of hatch cover stiffeners is not constant along the span, Appendix 3-2-A3 may be used to determine required scantlings.

9.1.2 Cleats

Cleats are to be set to fit the taper of the wedges. They are to be at least 65 mm (2.5 in.) wide and spaced not more than 600 mm (23.5 in.) center to center. The cleats along each side or end are to be not more than 150 mm (6 in.) from the hatch corners.

9.1.3 Wedges

Wedges are to be of tough wood with a taper of not more than 1 in 6 and are to be not less than 13.0 mm (0.50 in.) thick at the toes.

9.1.4 Battening Bars

Battening bars are to be provided for properly securing the tarpaulins. They are to have a width of 64 mm (2.5 in.) and a thickness of not less than 9.5 mm (0.375 in.).

9.1.5 Tarpaulins

At least two thoroughly waterproofed tarpaulins of ample strength are to be provided for each exposed hatchway. The material is to be guaranteed free from jute and is to be of an approved type. Synthetic fabrics which have been demonstrated to be equivalent will be specially approved.

9.1.6 Security of Hatch Covers

For all hatchways in Position 1 or 2, steel bars or other equivalent means are to be provided in order to efficiently and independently secure each section of hatch covers after the tarpaulins are battened down. Hatch covers of more than 1.5 m (4.9 ft) in length are to be secured by at least two such securing appliances.

9.3 Wooden Hatch Covers

9.3.1 Hatch Boards

Wood hatch covers on exposed hatchways are to have a finished thickness not less than 60 mm (2.375 in.) where the span is not more than 1.5 m (4.9 ft); the wood is to be of satisfactory quality, straight-grained, reasonably free from knots, sap and shakes, and is to be examined before being coated. Hatch rests are to be beveled where necessary, so as to provide a solid bearing surface.

9.3.2 Portable Beams (1 January 2005)

Where portable beams for supporting wooden hatch boards are made of steel, the maximum allowable stress and deflection under the design loads in 3-2-12/5.3 are as follows:

Maximum allowable stress	0.68Y
Maximum allowable deflection	0.0044 times the span

Where Y is as defined in 3-2-12/9.1.1.

Where the cross section of portable beams is not constant along the span, Appendix 3-2-A3 may be used to determine required beam scantlings.

9.3.3 Closing/Securing Arrangements

Closing arrangements are to be in accordance with 3-2-12/9.1.2 through 3-2-12/9.1.6.

9.3.4 Carriers and Sockets

Carriers or sockets for portable beams are to be of substantial construction, and are to provide means for the efficient fitting and securing of the beams. Where rolling types of beams are used, the arrangements are to ensure that the beams remain properly in position when the hatchway is closed. The bearing surface is not to be less than 75 mm (3 in.) in width, measured along the axis of the beam, unless the carriers are of an interlocking type with the beam ends. Carriers for beams are to extend to the deck level or the coamings are to be fitted with stiffeners or external brackets in way of each beam.

9.5 Steel Hatch Covers

9.5.1 Scantlings (1 January 2005)

Where steel hatch covers are fitted, the maximum allowable stress and deflection under the design loads in 3-2-12/5.3 and the minimum top plate thickness are as follows:

Maximum allowable stress	0.8Y, and not exceed the critical buckling strength in compression
Maximum allowable deflection	0.0056 times the span
Top plate thickness	0.01s, but not less than 6 mm (0.24 in.)

Where Y and s are as defined in 3-2-12/9.1.1.

Covers are to be assumed to be simply supported.

Where the cross section of hatch over stiffeners is not constant along the span, Appendix 3-2-A3 may be used to determine required scantlings.

9.5.2 Closing Arrangements

Closing arrangements are to be in accordance with 3-2-12/9.1.2 through 3-2-12/9.1.6.

9.7 Bearing Surface

The width of each bearing surface for hatchway covers is to be at least 65 mm (2.5 in.)

9.9 Materials Other Than Steel

The strength and stiffness of covers made of materials other than steel are to be equivalent to those of steel and will be subject to special consideration.

11 Hatchways Closed by Covers of Steel Fitted with Gaskets and Clamping Devices

11.1 Strength of Covers (1 January 2005)

11.1.1 Scantlings

Where weathertight covers are steel, the maximum allowable stress and deflection under the design loads in 3-2-12/5.3 and the minimum top plate thickness are as follows.

Maximum allowable stress	$0.8Y$, and not to exceed the critical buckling strength in compression
Maximum allowable deflection	0.0056 times the span
Top plate thickness	$0.01s$, but not less than 6 mm (0.24 in.)

Where Y and s are as defined in 3-2-12/9.1.1.

The following corrosion margin is to be incorporated into each strength member of the hatch cover:

- Single skin hatch covers, a corrosion addition $t_s = 2.0$ mm (*) for all plating and stiffeners.
- Double skin hatch covers, a corrosion addition $t_s = 1.5$ mm (*) for top and bottom plating and $t_s = 1.0$ mm for the internal structure.

(*) Corrosion addition $t_s = 1.0$ mm for the hatch covers in way of cellular cargo holds intended for containers.

The value for cargo hatch covers for bulk carriers, ore carriers and combination carriers is given in 5C-3-4/19.3.1(a) of the *Steel Vessel Rules*.

Where the cross section of hatch cover stiffeners is not constant along the span, Appendix 3-2-A3 may be used to determine required scantlings.

11.3 Means for Securing Weathertightness (1 January 2005)

11.3.1 General

The means for securing and maintaining weathertightness are to be such that the tightness can be maintained in any sea conditions. The arrangements and the strength of these means of closing and securing of the covers to the anticipated sea loads are to comply with the following requirements. Where it is intended to carry cargoes on the covers, the securing means are also take into consideration these loads, including dynamic effects. Strength calculations for the means of securing hatch covers carrying cargoes are to be submitted for review. The covers are to be hose-tested in position under a water pressure of at least 2.1 bar (2.1 kgf/cm², 30 psi) at the time of construction and, if considered necessary, at subsequent surveys.

11.3.2 Securing Arrangements

11.3.2(a) Securing Device. Hatch cover panels are to be secured by appropriate devices (bolts, wedges or similar) suitably spaced alongside the coamings and between panels.

Arrangement and spacing are to be determined with due attention to the weather-tightness, the type and the size of the hatch cover, as well as the stiffness of the cover edges between the securing devices.

Securing devices are to be of reliable construction and securely attached to the hatchway coamings, decks or covers. Individual securing devices on each cover are to have approximately the same stiffness characteristics.

Where rod cleats are fitted, resilient washers or cushions are to be incorporated.

Where hydraulic cleating is adopted, a positive means is to be provided to ensure that it remains mechanically locked in the closed position in the event of failure of the hydraulic system.

11.3.2(b) Sectional Area. Subject to 3-2-12/11.3.2(c), the net sectional area of each securing device is not to be less than:

$$A = c_{sd} a/f \quad \text{cm}^2 \text{ (in}^2\text{)}$$

where

$$c_{sd} = 1.4 \text{ (1.4, 0.066)}$$

$$a = \text{spacing, in m (ft), of securing devices, not to be taken less than 2 m (6.6 ft)}$$

$$f = (\sigma_Y/Y)^e$$

$$Y = 235 \text{ N/mm}^2 \text{ (24 kgf/mm}^2\text{, 34,000 psi)}$$

$$\sigma_Y = \text{specified minimum upper yield stress, in N/mm}^2 \text{ (kgf/mm}^2\text{, psi), of the steel, not to be taken greater than 70\% of the ultimate tensile strength.}$$

$$e = 0.75 \quad \text{for } \sigma_Y > 235 \text{ N/mm}^2 \text{ (24 kgf/mm}^2\text{, 34,000 psi)}$$

$$= 1.0 \quad \text{for } \sigma_Y < 235 \text{ N/mm}^2 \text{ (24 kgf/mm}^2\text{, 34,000 psi)}$$

Rods or bolts are to have a net diameter not less than 19 mm (0.75 in.) for hatchways exceeding 5 m² (54 ft²) in area.

11.3.2(c) *Packing Line Pressure.* Between cover and coaming and at cross-joints, a packing line pressure sufficient to obtain weathertightness is to be maintained by the securing devices.

For packing line pressures exceeding 5 N/mm² (0.51 kgf/mm², 28.6 psi), the cross section area of the securing device is to be increased in direct proportion. The packing line pressure is to be specified.

11.3.2(d) *Edge Stiffness.* The cover edge stiffness is to be sufficient to maintain adequate sealing pressure between securing devices. The moment of inertia, I , of edge elements is not to be less than:

$$I = c_i p a^4 \quad \text{cm}^4 \text{ (in}^4\text{)}$$

where

$$c_i = 6 \text{ (58.8, 0.000218)}$$

$$p = \text{packing line pressure, in N/mm}^2 \text{ (kgf/mm}^2\text{, psi), minimum 5 N/mm}^2 \text{ (0.51 kgf/mm}^2\text{, 28.6 psi).}$$

$$a = \text{spacing, in m (ft), of securing devices.}$$

11.3.3 Stoppers

11.3.3(a) *Forces.* All hatch covers are to be fitted with stoppers to limit horizontal movement of the cover against the forces caused by the following pressures:

i) *Longitudinal pressure on fore end of cover:*

No. 1 hatch cover:

- Where a forecastle in accordance with 5C-3-1/7 of the *Steel Vessel Rules* is not fitted:

$$230 \text{ kN/m}^2 \text{ (23.5 tf/m}^2\text{, 2.14 Ltf/ft}^2\text{)}$$

- Where a forecastle in accordance with 5C-3-1/7 of the *Steel Vessel Rules* is fitted:

$$175 \text{ kN/m}^2 \text{ (17.8 tf/m}^2\text{, 1.63 Ltf/ft}^2\text{)}$$

Other hatch covers: 175 kN/m² (17.8 tf/m², 1.63 Ltf/ft²).

ii) *Transverse pressure on side of cover:*

All hatch covers: 175 kN/m² (17.8 tf/m², 1.63 Ltf/ft²).

11.3.3(b) *Allowable Stresses.* The equivalent stress:

i) In stoppers and their supporting structures, and

ii) Calculated in the throat of the stopper welds

is not to exceed $0.8\sigma_y$ under the above pressures.

11.3.4 Materials and Welding

Stoppers and securing devices are to be manufactured of materials and corresponding welding procedures and consumables, in accordance with applicable requirements of the *ABS Rules for Materials and Welding (Part 2)*.

13 Hatchways Closed by Portable Covers in Lower Decks or within Fully Enclosed Superstructures

13.1 General

The following scantlings are intended for conventional type covers. Those for covers of special types are to be specially considered.

13.3 Portable Beams and Wood Covers

Portable beams supporting lower deck hatch covers on which cargo is stowed are to have a section modulus, SM , of not less than that obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 1.18$$

$$h = \text{tween-deck height in m (ft). When a design load is specified, } h \text{ is to be taken as } p/n, \text{ where } p \text{ is the specified design pressure, in kN/m}^2 \text{ (kgf/m}^2, \text{ lbf/ft}^2), \text{ and } n \text{ is defined as 7.05 (715, 45).}$$

$$s = \text{spacing of the portable beams, in m (ft)}$$

$$\ell = \text{length of the portable beam, in m (ft)}$$

The depth of the portable beam is not to be less than 4% of its unsupported span.

Wood covers are not to be less than 63.5 mm (2.5 in.) thick where the spacing of the portable beams does not exceed 1.52 m (5 ft). Where the height to which the cargo may be loaded on top of the cover exceeds 2.59 m (8.5 ft), or where the spacing of the portable beams exceeds 1.52 m (5 ft), the thickness of the wood covers is to be suitably increased.

13.5 Steel Covers

The thickness of the plating for steel covers is not to be less than required for platform decks in enclosed cargo spaces, as obtained from 3-2-3/1. A stiffening bar is to be fitted around the edges, as required, to provide the necessary rigidity to permit the covers being handled without deformation. The effective depth of the framework is normally to be not less than 4% of its unsupported length. The stiffeners in association with the plating to which they are attached are to have section modulus, SM , as determined by the following equation:

$$SM = 7.8hs\ell^2 \text{ cm}^3$$

$$SM = 0.0041hs\ell^2 \text{ in}^3$$

where

$$h = \text{tween-deck height, in m (ft). When a design load is specified, } h \text{ is to be taken as } p/n, \text{ where } p \text{ is the specified design pressure in kN/m}^2 \text{ (kgf/m}^2, \text{ lbf/ft}^2) \text{ and } n \text{ is defined as 7.05 (715, 45).}$$

$$s = \text{spacing of the stiffeners, in m (ft)}$$

$$\ell = \text{length of the stiffener, in m (ft)}$$

13.7 Wheel Loading

Where provision is to be made for the operation or stowage of vehicles having rubber tires, the thickness of the hatch cover plating is to be not less than that obtained from 3-2-3/3, for platform deck plating, except that the thickness of plate panels adjacent to the edges of the covers is to be at least 15% greater than obtained from 3-2-3/7.

14 Small Hatches on the Exposed Fore Deck (2004)

14.1 Application

This subsection is applicable to vessels with length, L , (as defined in 3-1-1/3.1) between 80 meters (263 feet) and 90 meters (295 feet).

The requirements of this subsection apply to all small hatches [opening normally 2.5 m^2 (27 ft^2) or less] located on the exposed fore deck within the forward $0.25L$, where the deck in way of the hatch is less than $0.1L$ or 22 m (72.2 ft) above the summer load line, whichever is less.

Hatches designed for emergency escape need not comply with 3-2-12/14.5i), 3-2-12/14.5ii), the third paragraph of 3-2-12/14.7 and 3-2-12/14.9.

14.3 Strength

For small rectangular steel hatch covers, the plate thickness, stiffener arrangement and scantlings are to be in accordance with 3-2-12/Table 2 and 3-2-12/Figure 1. Stiffeners, where fitted, are to be aligned with the metal-to-metal contact points required in 3-2-12/14.7 (see also 3-2-12/Figure 1). Primary stiffeners are to be continuous. All stiffeners are to be welded to the inner edge stiffener (see 3-2-12/Figure 2).

The upper edge of the hatchway coaming is to be suitably reinforced by a horizontal section, normally not more than 170 to 190 mm (6.9 to 7.5 in.) from the upper edge of the coaming.

For small hatch covers of circular or similar shape, the cover plate thickness and reinforcement is to provide strength and stiffness equivalent to the requirements for small rectangular hatches.

For small hatch covers constructed of materials other than steel, the required scantlings are to provide strength and stiffness equivalent to 235 N/mm^2 (24 kgf/mm^2 , 34,000 psi) yield strength steel.

14.5 Primary Securing Devices

The primary securing devices are to be such that their hatch covers can be secured in place and made weather-tight by means of a mechanism employing any one of the following methods:

- i) Butterfly nuts tightening onto forks (clamps), or
- ii) Quick acting cleats, or
- iii) A central locking device.

Dogs (twist tightening handles) with wedges are not acceptable.

14.7 Requirements for Primary Securing

The hatch cover is to be fitted with a gasket of elastic material. This is to be designed to allow a metal-to-metal contact at a designed compression and to prevent over compression of the gasket by green sea forces that may cause the securing devices to be loosened or dislodged. The metal-to-metal contacts are to be arranged close to each securing device, in accordance with 3-2-12/Figure 1, and of sufficient capacity to withstand the bearing force.

The primary securing method is to be designed and manufactured such that the designed compression pressure is achieved by one person without the need of any tools.

For a primary securing method using butterfly nuts, the forks (clamps) are to be of robust design. They are to be designed by means of curving the forks upward and a raised surface on the free end or a similar method to minimize the risk of butterfly nuts being dislodged while in use. The plate thickness of unstiffened steel forks is not to be less than 16 mm ($5/8$ in.). An example arrangement is shown in 3-2-12/Figure 2.

For small hatch covers located on the exposed deck forward of the fore-most cargo hatch, the hinges are to be fitted such that the predominant direction of green sea will cause the cover to close, which means that the hinges are normally to be located on the fore edge.

On small hatches located between the main hatches, for example, between Nos. 1 and 2, the hinges are to be placed on the fore edge or outboard edge, whichever is practicable, for protection from green water in beam sea and bow quartering conditions.

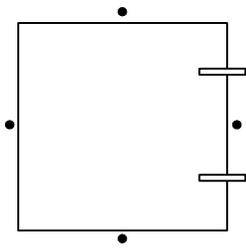
14.9 Secondary Devices

Small hatches on the fore deck are to be fitted with an independent secondary securing device e.g., by means of a sliding bolt, a hasp or a backing bar of slack fit, which is capable of keeping the hatch cover in place, even in the event that the primary securing device became loosened or dislodged. It is to be fitted on the side opposite to the hatch cover hinges.

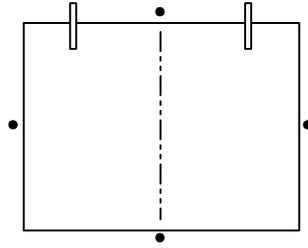
TABLE 2
Scantlings for Small Steel Hatch Covers on the Fore Deck

Nominal Size (mm × mm)	Cover Plate Thickness (mm)	Primary Stiffeners	Secondary Stiffeners
		Flat Bar (mm × mm); number	
630 × 630	8	---	---
630 × 830	8	100 × 8; 1	---
830 × 630	8	100 × 8; 1	---
830 × 830	8	100 × 10; 1	---
1030 × 1030	8	120 × 12; 1	80 × 8; 2
1330 × 1330	8	150 × 12; 2	100 × 10; 2

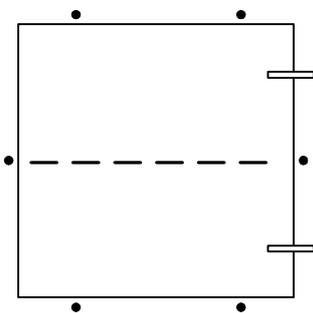
FIGURE 1
Arrangement of Stiffeners



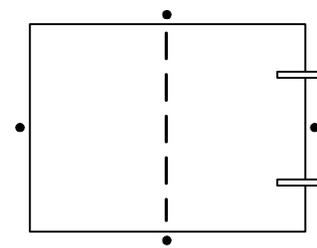
Nominal size 630 × 630



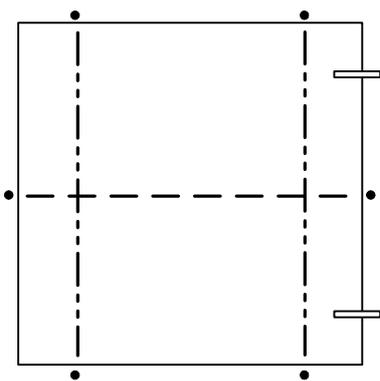
Nominal size 630 × 830



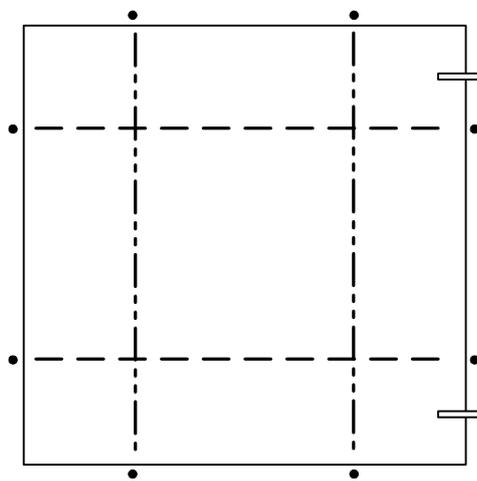
Nominal size 830 × 830



Nominal size 830 × 630



Nominal size 1030 × 1030



Nominal size 1330 × 1330

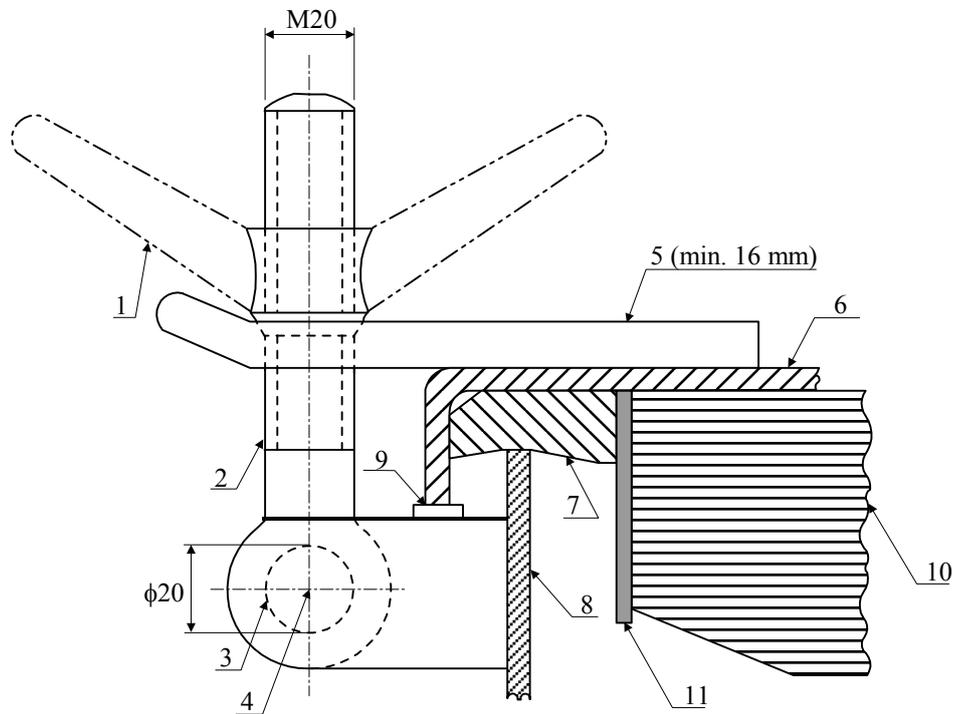
— Hinge

• Securing device/metal to metal contact

— — — Primary stiffener

· · · Secondary stiffener

FIGURE 2
Example of Primary Securing Method



- 1: butterfly nut
 2: bolt
 3: pin
 4: center of pin
 5: fork (clamp) plate
 6: hatch cover
 7: gasket
 8: hatch coaming
 9: bearing pad welded on the bracket of a toggle bolt for metal to metal contact
 10: stiffener
 11: inner edge stiffener
- (Note: Dimensions in millimeters)

15 Hatchways within Open Superstructures

Hatchways within open superstructures are to be considered as exposed.

17 Hatchways within Deckhouses

Hatchways within deckhouses are to have coamings and closing arrangements as required in relation to the protection afforded by the deckhouse from the standpoint of its construction and the means provided for the closing of all openings into the house.

19 Container Loading

Where it is intended to carry containers on steel hatch covers, the exact locations of the container pads and the maximum total static load on the pads are to be indicated on the plans. Where the pads are not in line with the supporting structures, headers are to be provided to transmit the loads to these members. Each member intended to support containers is to have a section modulus, SM , in cm^3 (in^3), not less than that obtained from the following equation:

$$SM = M/f$$

where

M = Maximum bending moment due to maximum static container loading, in kN-cm, (kgf-cm, Ltf-in)

f = permissible maximum bending stress, not to exceed $0.46Y$

Y = specified yield point or yield strength, in kN/cm^2 (kgf/cm^2 , Ltf/in^2), but is not to exceed 72% of the specified minimum tensile strength.

In determining the maximum bending moment, members are to be considered simply-supported.

The net sectional area of the web of the member, in cm^2 (in^2), including effective brackets, where applicable, is to be not less than that obtained from the following equation:

$$A = F/q$$

where

F = shearing force at the point under consideration, in kN (kgf, Ltf)

q = allowable average shear stress in the web, not to exceed $0.34Y$

The hatch cover is to be so designed as to limit deflection to 0.0028 times the span under the static container load.

21 Machinery Casings

21.1 Arrangement

Machinery-space openings in Position 1 or 2 are to be framed and efficiently enclosed by steel casings of ample strength, and, wherever practicable, those in freeboard decks are to be within superstructures or deckhouses. Openings in exposed casings are to be fitted with doors complying with the requirements of 3-2-9/5.1. The sills are to be in accordance with 3-2-12/7.1 for companionways. Other openings in such casings are to be fitted with equivalent covers, permanently attached. Stiffeners are to be spaced at not more than 760 mm (30 in.)

21.3 Exposed Casings on Freeboard or Raised Quarter Decks

Exposed casings on freeboard or raised quarter decks are to have plating not less in thickness than that obtained from 3-2-9/3 or the following equation, whichever is greater:

$$t = 0.0164L + 6 \text{ mm}$$

$$t = 0.0002L + 0.24 \text{ in.}$$

where

t = thickness, in mm (in.)

L = length of vessel, in m (ft), as defined in 3-1-1/3.

Stiffeners are to be at least as effective as those required for watertight bulkheads where ℓ is the tween deck height and h is $\ell/2$.

21.5 Exposed Casings on Superstructure Decks

Exposed casings on superstructure decks are to have plating not less in thickness than that obtained from the following equation:

$$t = 0.033L + 3.5 \text{ mm}$$

$$t = 0.0004L + 0.14 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in 3-1-1/3, but is not to be taken as less than 30.5 m (100 ft).}$$

Stiffeners in association with the plating to which they are attached are to have section modulus, SM , as obtained from the following equation:

$$SM = 7.8chs\ell^2 \text{ cm}^3$$

$$SM = 0.0041chs\ell^2 \text{ in}^3$$

where

$$c = 0.25$$

$$s = \text{spacing of stiffeners, in m (ft)}$$

$$h = \text{height of casing, in m (ft)}$$

$$\ell = \text{length, between supports, of the stiffeners, in m (ft)}$$

21.7 Casings within Open Superstructures

Casings within open superstructures are to be of similar scantlings to those obtained from 3-2-12/21.5 for exposed casings on superstructure decks.

21.9 Casings within Enclosed Superstructures, Deckhouses, or below Freeboard Decks

Casings within enclosed superstructures or in decks below freeboard deck where cargo is carried are to have plating not less in thickness than that obtained from the following equation:

$$t = 0.022L + 3.8 \text{ mm}$$

$$t = 0.00027L + 0.15 \text{ in.}$$

where

$$t = \text{thickness, in mm (in.)}$$

$$L = \text{length of vessel, in m (ft), as defined in 3-1-1/3, but is not to be taken as less than 30.5 m (100 ft).}$$

Stiffeners are to be fitted in line with the beams and are to have section modulus, SM , as required for exposed casings by 3-2-12/21.5, but the coefficient in the formula may be 0.14 instead of 0.25 and h is the tween-deck height.

23 Miscellaneous Openings in Freeboard and Superstructure Decks

23.1 Manholes and Scuttles

Manholes and flush scuttles in Position 1 or 2 within superstructures other than enclosed superstructures are to be closed by substantial covers capable of being made watertight. Unless secured by closely spaced bolts, the covers are to be permanently attached.

23.3 Other Openings

Openings in freeboard decks other than hatchways, machinery-space openings, manholes and flush scuttles are to be protected by an enclosed superstructure, or by a deckhouse or companionway of equivalent strength and weathertightness. Any such opening in an exposed superstructure deck or in the top of a deckhouse on the freeboard deck which gives access to a space below the freeboard deck or a space within an enclosed superstructure is to be protected by an efficient deckhouse or companionway. Doorways in such deckhouses or companionways are to be fitted with doors complying with the requirements given in 3-2-9/5.1.

23.5 Escape Openings

The closing appliances of escape openings are to be readily operable from each side.

23.7 Chain Pipe Opening (*1 July 2003*)

For vessels with freeboard length, L_f , (as defined in 3-1-1/3.3) greater than 24 meters (79 feet), chain pipes through which anchor cables are led are to be provided with permanently attached closing appliances to minimize the ingress of water. A canvas cover with appropriate lashing arrangement will be acceptable for this purpose. A cement and wire mesh arrangement is not permitted.

The arrangement on vessels that are not subject to the International Convention on Load Lines or its Protocol may be specially considered.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

APPENDIX **3 Portable Beams and Hatch Cover Stiffeners of Variable Cross Section**

1 Application

For portable beams and hatch cover stiffeners with free ends and varying cross section along their span, the section modulus, SM , and inertia, I , at the midspan required by 3-2-12/9.3.2, 3-2-12/9.5.1 and 3-2-12/11.1.1 may be obtained from the following equations:

$$SM = \frac{C_1 K_1 p s \ell^2}{\sigma_a} \text{ cm}^3 \text{ (in}^3\text{)}$$

$$I = C_2 K_2 p s \ell^3 \text{ cm}^4 \text{ (in}^4\text{)}$$

where

$$C_1 = 125 \text{ (125, 1.5)}$$

$$C_2 = 2.87 \text{ (28.2, } 2.85 \times 10^{-5}\text{) for 3-2-12/9.1.1 and 3-2-12/9.3.2}$$

$$= 2.26 \text{ (22.1, } 2.24 \times 10^{-5}\text{) for 3-2-12/9.5.1 and 3-2-12/11.1}$$

$$K_1 = 1 + \frac{3.2\alpha - \gamma - 0.8}{7\gamma + 0.4}, \text{ but not less than 1.0}$$

$$\alpha = \text{length ratio}$$

$$= \ell_1 / \ell$$

$$\gamma = SM \text{ ratio}$$

$$= SM_1 / SM$$

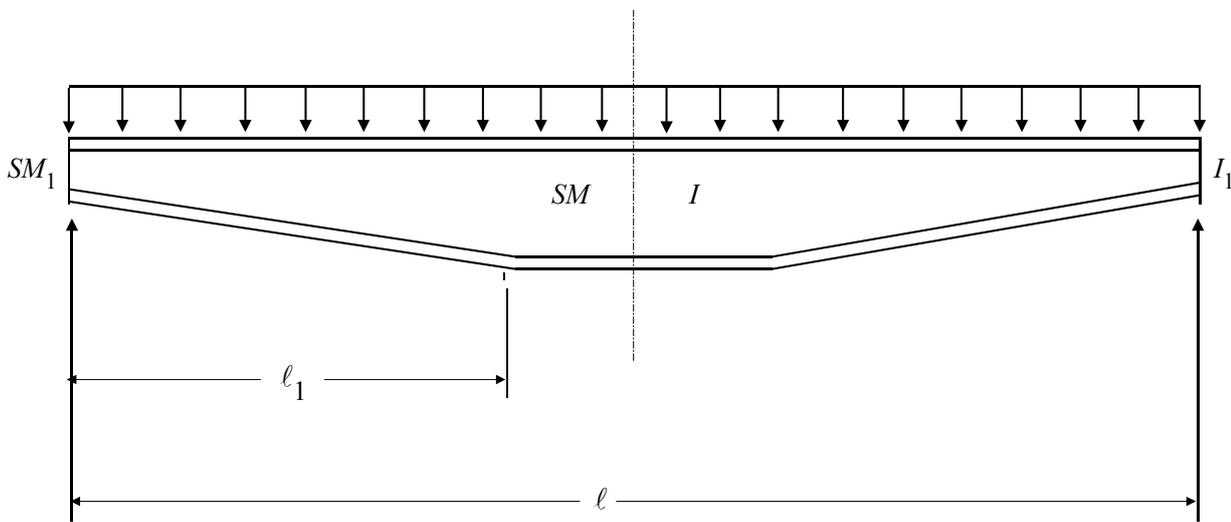
ℓ_1 , ℓ , SM_1 and SM are as indicated in 3-2-A3/Figure 1

$$\sigma_a = \text{allowable stress given in 3-2-12/9.1.1, 3-2-12/9.3.2, 3-2-12/9.5.1, and 3-2-12/11.1, in kN/mm}^2 \text{ (kgf/mm}^2\text{, psi)}$$

$$K_2 = 1 + 8\alpha^3 \frac{(1-\beta)}{(0.2 + 3\sqrt{\beta})}, \text{ but not less than 1.0}$$

- β = ratio of the moments of inertia, I_1 and I , at the locations indicated in 3-2-A3/Figure 1
 = I_1/I
- p = design load given in 3-2-12/5.3, in kN/m^2 (tf/m², psi)
- s = spacing of beams or stiffeners, in m (ft)
- ℓ = span of free ended constructional elements, in m (ft)

FIGURE 1
***SM* and *I* of Construction Elements**



PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **13 Protection of Shell Openings**

1 Cargo, Gangway, or Fueling Ports

1.1 Construction

Cargo, gangway or fueling ports in the sides of vessels are to be strongly constructed and capable of being made thoroughly watertight. Where frames are cut in way of such ports, web frames are to be fitted on the sides of the openings, and suitable arrangements are to be provided for the support of the beams over the openings. Thick shell plates or doublers are to be fitted, as required, to compensate for the openings. The corners of the openings are to be well rounded. Waterway angles and scuppers are to be provided on the decks in way of ports in cargo spaces below the freeboard deck or in cargo spaces within enclosed superstructures to prevent the spread of any leakage water over the decks.

Indicators showing whether the ports in the side shell below the freeboard or superstructure deck are secured closed or open are to be provided on the navigation bridge.

Where allowed by 3-2-13/1.3, cargo ports or similar openings located with their lower edge below the line defined in 3-2-13/1.3 are to be fitted with a second door of equivalent strength and watertightness with a leakage detection device for the compartment between the doors. The drain from this compartment is to be led to the bilge with a screwdown valve operable from an accessible location.

In general, all outer doors are to open outwards.

1.3 Location

The lower edges of cargo, gangway or fueling-port openings are not to be below a line parallel to the freeboard deck at side having as its lowest point the designed load waterline or upper edge of the uppermost load line.

Cargo ports or similar openings may be located with their lower edge below the line defined above, provided they meet the additional construction requirements of 3-2-13/1.1.

3 Bow Doors, Inner Doors, Side Shell Doors and Stern Doors (1998)

3.1 General (2005)

Where bow doors of the visor or side-opening type are fitted leading to complete or long forward enclosed superstructure or to long superstructures with closing appliances to the satisfaction of the Administration, bow doors and inner doors are to meet the requirements of this Section. Hull supporting structure in way of the bow doors is to be able to withstand the loads imposed by the bow door securing and supporting devices without exceeding the allowable stresses for those devices, both given in this Section.

Side shell doors fitted abaft the collision bulkhead and stern doors leading into enclosed spaces are to meet the requirements of this Section.

3.3 Arrangement

As far as practicable, bow doors and inner doors are to be arranged so as to preclude the possibility of the bow door causing structural damage to the inner door or to the collision bulkhead in the case of damage to or detachment of the bow door.

3.3.1 Bow Doors

Bow doors are to be situated above the freeboard deck, except that where a watertight recess fitted for arrangement of ramps or other related mechanical devices is located forward of the collision bulkhead and above the deepest waterline, the bow doors may be situated above the recess.

3.3.2 Inner Doors

An inner door is to be fitted in the extension of the collision bulkhead required by 3-2-7/3.1.1. A vehicle ramp made watertight and conforming to 3-2-7/3.1.1 in the closed position may be accepted for this purpose.

3.3.3 Side Shell and Stern Doors (1998)

Stern doors for passenger vessels are to be situated above the freeboard deck. Stern doors for ro-ro cargo vessels and all side shell doors need not be situated above the freeboard deck.

5 Securing, Locking and Supporting of Doors

5.1 Definitions

5.1.1 Securing Device

A device used to keep the door closed by preventing it from rotating about its hinges or its pivoted attachments to the vessel.

5.1.2 Supporting Device

A device used to transmit external or internal loads from the door to a securing device and from the securing device to the vessel's structure, or a device other than a securing device, such as a hinge, stopper or other fixed device that transmits loads from the door to the vessel's structure.

5.1.3 Locking device

A device that locks a securing device in the closed position.

7 Securing and Supporting Devices

7.1 Bow Doors

Means are to be provided to prevent lateral or vertical movement of the bow doors when closed. Means are also to be provided for mechanically fixing the door in the open position.

Means of securing and supporting the door are to maintain equivalent strength and stiffness of the adjacent structure.

7.1.1 Clearance and Packing

The maximum design clearance between the door and securing/supporting devices is not to exceed 3 mm (0.12 in.). Where packing is fitted, it is to be of a comparatively soft type and the supporting forces are to be carried by the steel structure only.

7.1.2 Visor Door Arrangement

The pivot arrangement is to be such that the visor is self closing under external loads. The closing moment, M_y , as defined in 3-2-13/19.5.1, is not to be less than M_{yo} , as given by the following equation:

$$M_{yo} = Wc + 0.1 \sqrt{a^2 + b^2} \sqrt{F_x^2 + F_z^2}$$

Where W , a , b , c , F_x and F_z are as defined in 3-2-13/19. In addition, the arrangement of the door is to be such that the reaction forces of pin or wedge supports at the base of the door do not act in the forward direction when the door is loaded in accordance with 3-2-13/19.5.4.

7.3 Side Shell and Stern Doors (1998)

Means are to be provided to prevent lateral or vertical movement of the side shell or stern doors when closed. Means are also to be provided for mechanically fixing the doors in the open position.

The means of securing and supporting the doors are to have strength and stiffness equivalent to the adjacent structure.

Clearance and packing for side shell and stern doors are to be in accordance with 3-2-13/7.1.1.

9 Securing and Locking Arrangement

9.1 General

Securing devices are to be provided with a mechanical locking arrangement (self locking or separate arrangement), or are to be of the gravity type.

9.3 Operation

Securing devices are to be simple to operate and readily accessible. The opening and closing systems as well as the securing and locking devices are to be interlocked in such a way that they can only operate in the proper sequence.

9.3.1 Hydraulic Securing Devices

Where hydraulic securing devices are applied, the system is to be mechanically lockable in the closed position. In the event of a loss of hydraulic fluid, the securing devices are to remain locked.

The hydraulic system for securing and locking devices is to be isolated from other hydraulic circuits when in the closed position

9.3.2 Remote Control (1998)

Where bow doors and inner doors give access to a vehicle deck, or where side shell doors or stern doors are located partially or totally below the freeboard deck with a clear opening area greater than 6 m² (65 ft²), an arrangement for remote control from a position above the freeboard deck is to be provided, allowing closing and opening of the doors and associated securing and locking of every door. The operating panels for doors are to be accessible to authorized persons only. A notice plate giving instructions to the effect that all securing devices are to be closed and locked before leaving harbor is to be placed at each operating panel and is to be supplemented by warning indicator lights, as required by 3-2-13/9.5.1.

9.5 Indication/Monitoring (1998)

The following requirements for indicators, water leakage protection and door surveillance are required for vessels fitted with bow doors and inner doors. The requirements also apply to vessels fitted with side shell doors or stern doors in the boundary of special category spaces or ro-ro spaces through which such spaces may be flooded.

The requirements are not applicable to ro-ro cargo vessels where no part of the side shell doors or stern doors is located below the uppermost waterline and the area of the door opening is not greater than 6 m² (65 ft²).

9.5.1 Indicators (2005)

The indicator system is to be designed on the fail safe principle and in accordance with the following. See 3-2-13/9.5.1(e).

9.5.1(a) Location and Type (1998). Separate indicator lights are to be provided on the navigation bridge and on each operating panel to show that the doors are closed and that their locking devices are properly positioned.

The indication panel on the navigation bridge is to be equipped with a mode selection function "harbor/sea voyage", arranged so that an audible and visible alarm is given on the navigation bridge if, in the sea voyage condition, the doors are not closed, or any of the securing devices are not in the correct position.

Indication of the open/closed position of every door and every securing and locking device is to be provided at the operating panels.

9.5.1(b) Indicator Lights. Indicator lights are to be designed so that they cannot be manually turned off. The indication panel is to be provided with a lamp test function.

9.5.1(c) Power Supply. The power supply for the indicator system is to be independent of the power supply for operating and closing the doors and is to be provided with a backup power supply from the emergency source of power or other secure power supply, e.g., UPS.

9.5.1(d) Protection of Sensors. Sensors are to be protected from water, ice formation and mechanical damage.

9.5.1(e) Fail Safe Principle. The alarm/indicator system is considered designed on a fail-safe principle when the following are provided, as applicable.

- i) The indicator panel is provided with:
- A power failure alarm
 - An earth failure alarm
 - A lamp test
 - Separate indication for door closed, door locked, door not closed and door not locked.

- ii) Limit switches electrically closed when the door is closed (when more limit switches are provided, they may be connected in series)
- iii) Limit switches electrically closed when securing arrangements are in place (when more limit switches are provided, they may be connected in series)
- iv) Two electrical circuits (also in one multicore cable), one for the indication of door closed/not closed and the other for door locked/not locked.
- v) In the case of dislocation of limit switches, indication to show: not closed/not locked/securing arrangements not in place, as appropriate.

9.5.2 Water Leakage Protection (2005)

A drainage system is to be arranged in the areas between the bow door and ramp or, where no ramp is fitted, between the bow door and inner door. The system is to be equipped with an audible alarm function to the navigation bridge being set off when the water levels in these areas exceed 0.5 m (1.6 ft) or the high water level alarm, whichever is the lesser. See 3-2-13/9.5.1(e).

For vessels fitted with bow and inner doors, a water leakage detection system with audible alarm and television surveillance is to be arranged to provide an indication to the navigation bridge and to the engine control room of leakage through the inner door. See 3-2-13/9.5.1(e).

For passenger vessels fitted with side shell or stern doors, a water leakage detection system with audible alarm and television surveillance is to be arranged to provide an indication to the navigation bridge and to the engine control room of leakage through any of the doors.

For cargo vessels fitted with side shell or stern doors, a water leakage detection system with audible alarm is to be arranged to provide an indication to the navigation bridge of leakage through any of the doors. See 3-2-13/9.5.1(e).

9.5.3 Door Surveillance (2005)

Between the bow door and the inner door, a television surveillance system is to be fitted with a monitor on the navigation bridge and in the engine control room. The system is to monitor the position of doors and a sufficient number of their securing devices. Special consideration is to be given for the lighting and contrasting color of objects under surveillance.

11 Watertightness

11.1 Bow Doors

Bow doors are to be so fitted as to ensure tightness consistent with operational conditions and to give effective protection to the inner doors.

11.3 Inner Doors

Inner doors forming part of the extension of the collision bulkhead are to be weathertight over the full height of the cargo space and arranged with fixed sealing supports on the aft side of the doors.

11.5 Side Shell and Stern Doors (1998)

Side shell doors and stern doors are to be so fitted as to ensure water tightness.

13 Bow Door Scantlings

13.1 General

Bow doors are to be framed and stiffened so that the whole structure is equivalent to the unpierced bulkhead when closed.

13.3 Primary Structure (2005)

Scantlings of primary members are to be designed so that the allowable stresses indicated in 3-2-13/25.1 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/19.1. Normally, simple beam theory may be applied to determine the bending stresses. Members are to be considered to have simply supported end connections.

13.5 Secondary Stiffeners

Secondary stiffeners are to be supported by primary members constituting the main stiffening of the door. The section modulus, SM , of secondary stiffeners is to be as required by 3-2-5/1.1 and 3-2-5/5.3. In addition, stiffener webs are to have a net sectional area not less than that obtained from the following equation:

$$A = VQ/10 \text{ cm}^2 \quad (A = VQ \text{ cm}^2, A = VQ/6.5 \text{ in}^2)$$

where

V = shear force, in kN (tf, Ltf), in the stiffener calculated using the uniformly distributed external pressure, P_{eb} , given in 3-2-13/19.1

Q = as defined in 3-2-1/7.5

13.7 Plating

The thickness of bow door plating is to be not less than that required for side shell plating at the same location.

13.9 Securing and Supporting Devices (2005)

Scantlings of securing and supporting devices are to be designed so that the allowable stresses indicated in 3-2-13/25.1 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/19.3. All load transmitting elements in the design load path from the door through securing and supporting devices into the vessel structure, including welded connections, are to meet the strength standards required for securing and supporting devices. These elements include pins, support brackets and back-up brackets. Where fitted, threaded bolts are not to carry support forces, and the maximum tensile stress in way of the threads is not to exceed the allowable stress given in 3-2-13/25.5.

In determining the required scantlings, the door is to be assumed to be a rigid body. Only those active supporting and securing devices having an effective stiffness in the relevant direction are to be included and considered when calculating the reaction forces on the devices. Small or flexible devices, such as cleats, intended to provide load compression of the packing material, are not to be included in the calculations.

13.9.1 Bearing Pressure

The bearing pressure on steel to steel bearings is to be calculated by dividing the design force by the projected bearing area, and is not to exceed the allowable stress given in 3-2-13/25.3.

13.9.2 Redundancy

In addition to the above requirements, the arrangement of the securing and supporting devices is to be designed with redundancy, such that in the event of failure of any single securing or supporting device, the stresses in the remaining devices do not exceed the allowable stresses indicated in 3-2-13/25.1 by more than 20% under the above loads.

13.9.3 Visor Door Securing and Supporting Devices

Securing and supporting devices, excluding the hinges, are to be capable of resisting the vertical design force given in 3-2-13/19.5.3 without stresses exceeding the allowable stresses in 3-2-13/25.1.

Two securing devices are to be provided at the lower part of the door, each capable of providing the full reaction force required to prevent opening of the door without stresses exceeding the allowable stresses indicated in 3-2-13/25.1. The opening moment, M_o , to be balanced by this force is as given in 3-2-13/19.5.2.

13.9.4 Side-opening Door Thrust Bearing

A thrust bearing is to be provided in way of girder ends at the closing of the two doors, and is to prevent one door from shifting towards the other one under the effect of unsymmetrical pressure. Securing devices are to be fitted to secure sections of thrust bearing to one another.

13.11 Visor Door Lifting Arms and Supports

Where visor type bow doors are fitted, calculations are to be submitted, verifying that lifting arms and their connections to the door and vessel structure are adequate to withstand the static and dynamic forces applied during the lifting and lowering operations under a wind pressure of at least 1.5 kN/m² (0.15 tf/m², 0.014 Ltf/ft²).

15 Inner Door Scantlings

15.1 General

Scantlings of inner doors are to meet the requirements of this subsection. In addition, where inner doors are used as vehicle ramps, scantlings are not to be less than that required for vehicle decks in Sections 3-2-6 and 3-2-3.

15.3 Primary Structure

Scantlings of primary members are to be designed so that the allowable stresses indicated in 3-2-13/25.1 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/21.1.

15.5 Securing and Supporting Devices

Scantlings of securing and supporting devices are to be designed so that the allowable stresses indicated in 3-2-13/19 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/17.5. Where fitted, threaded bolts are not to carry support forces, and the maximum tensile stress in way of the threads is not to exceed the allowable stress given in 3-2-13/25.5.

The bearing pressure on steel to steel bearings is to be calculated by dividing the design force by the projected bearing area, and is not to exceed the allowable stress given in 3-2-13/25.3.

17 Side Shell Door and Stern Door Scantlings (1998)

17.1 General

Scantlings of side shell doors or stern doors are to meet the requirements of this subsection. In addition, where the doors are used as vehicle ramps, scantlings are not to be less than that required for vehicle decks in Sections 3-2-6 and 3-2-3.

17.3 Primary Structure (2005)

Scantlings of primary members are to be designed so that the allowable stresses indicated in 3-2-13/25.1 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/23. Normally, simple beam theory may be applied to determine the bending stresses. Members are to be considered to have simply supported end connections.

17.5 Secondary Stiffeners

Secondary stiffeners are to be supported by primary members constituting the main stiffening of the door. The section modulus, SM , of secondary stiffeners is to be not less than that required by Section 3-2-5 for frames in the same location. In addition, the net sectional area of stiffener webs is to be in accordance with 3-2-13/13.5, using the external pressure, p_e , given in 3-2-13/23.

17.7 Plating

The thickness of side or stern door plating is to be not less than that required for side shell plating at the same location.

17.9 Securing and Supporting Devices

Scantlings of securing and supporting devices are to be designed so that the allowable stresses indicated in 3-2-13/25.1 are not exceeded when the structure is subjected to the design loads indicated in 3-2-13/23. All load transmitting elements in the design load path from the door through securing and supporting devices into the vessel structure, including welded connections, are to meet the strength standards required for securing and supporting devices. Where fitted, threaded bolts are not to carry support forces, and the maximum tensile stress in way of the threads is not to exceed the allowable stress given in 3-2-13/25.5.

In determining the required scantlings, the door is to be assumed to be a rigid body. Only those active supporting and securing devices having an effective stiffness in the relevant direction are to be included and considered when calculating the reaction forces on the devices. Small or flexible devices, such as cleats, intended to provide compression load on the packing material are not to be included in the calculations.

17.9.1 Bearing Pressure

The bearing pressure on steel to steel bearings is to be calculated by dividing the design force by the projected bearing area, and is not to exceed the allowable stress given in 3-2-13/25.3.

17.9.2 Redundancy

In addition to the above requirements, the arrangement of the securing and supporting devices is to be designed with redundancy, such that in the event of a failure of any single securing or supporting device, the stresses in the remaining devices do not exceed the allowable stresses indicated in 3-2-13/25.1 by more than 20% under the above loads.

19 Bow Door Design Loads

19.1 External Pressure

The design external pressure, P_{eb} , is to be taken as indicated by the following equation. P_{eb} for vessels engaged in restricted service will be specially considered.

$$P_{eb} = nc(0.22 + 0.15 \tan \beta)(0.4V_d \sin \alpha + 0.6\sqrt{kL})^2 \text{ kN/m}^2 \text{ (tf/m}^2, \text{ Ltf ft}^2\text{)}$$

where

$$n = 2.75 \text{ (0.280, 0.0256)}$$

$$c = 0.0125L \quad \text{for } L < 80 \text{ m}$$

$$= 0.00385L \quad \text{for } L < 260 \text{ ft}$$

$$= 1.0 \quad \text{for } L \geq 80 \text{ m (260 ft)}$$

$$L = \text{length of vessel as defined in 3-1-1/3, in m (ft)}$$

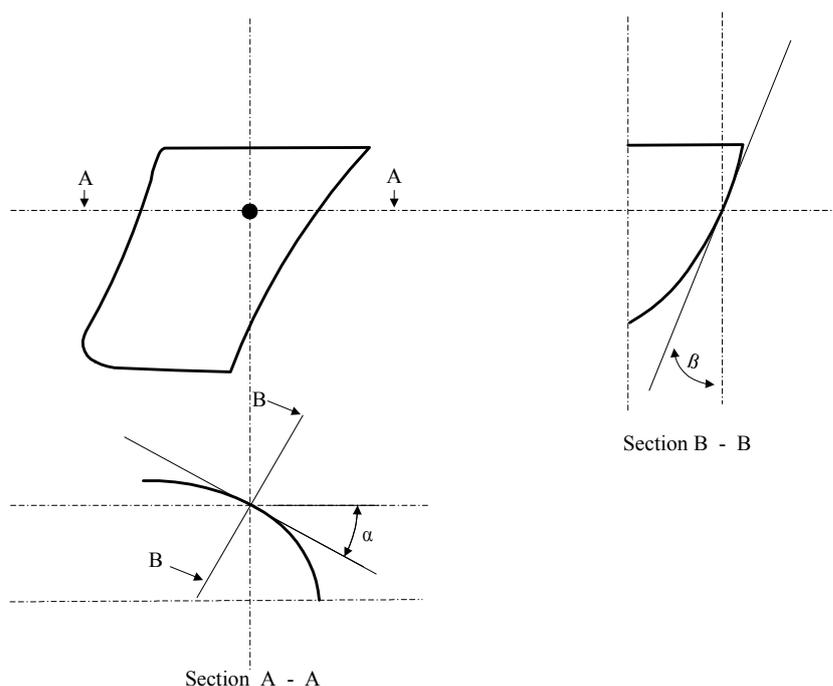
$$\beta = \text{flare angle at the point to be considered, defined as the angle between a vertical line and the tangent to the side shell plating measured in a vertical plane normal to the horizon tangent to the shell plating. See 3-2-13/Figure 1.}$$

$$\alpha = \text{entry angle at the point to be considered, defined as the angle between a longitudinal line parallel to the centerline and the tangent to the shell plating in a horizontal plane. See 3-2-13/Figure 1.}$$

$$k = 1.0 \text{ (1.09, 0.305)}$$

$$V_d = \text{vessel design speed, as defined in 3-2-11/3}$$

FIGURE 1
Entry and Flare Angles



19.3 External Forces (2005)

The design external forces considered in determining scantlings of securing and supporting devices of bow doors are not to be taken less than those given by the following equations:

$$F_x = P_{em} A_x$$

$$F_y = P_{em} A_y$$

$$F_z = P_{em} A_z$$

where

F_x = the design external force in the longitudinal direction, in kN (tf, Ltf)

F_y = the design external force in the horizontal direction, in kN (tf, Ltf)

F_z = the design external force in the vertical direction, in kN (tf, Ltf)

A_x = area, in m² (ft²), of the transverse vertical projection of the door between the levels of the bottom of the door and the top of the upper deck bulwark or between the bottom of the door and the top of the door, whichever is the lesser. Where the flare angle of the bulwark is at least 15° less than the flare of the adjacent shell plating, the bulwark may be excluded and the distance may be measured from the bottom of the door to the upper deck or to the top of the door, whichever is the lesser.

A_y = area, in m² (ft²), of the longitudinal vertical projection of the door between the levels of the bottom of the door and the top of the upper deck bulwark or between the bottom of the door and the top of the door, whichever is the lesser. Where the flare angle of the bulwark is at least 15° less than the flare of the adjacent shell plating, the bulwark may be excluded and the distance may be measured from the bottom of the door to the upper deck or to the top of the door, whichever is the lesser.

A_z = area, in m² (ft²), of the horizontal projection of the door between the levels of the bottom of the door and the top of the upper deck bulwark or between the bottom of the door and the top of the door, whichever is the lesser. Where the flare angle of the bulwark is at least 15° less than the flare of the adjacent shell plating, the bulwark may be excluded and the distance may be measured from the bottom of the door to the upper deck or to the top of the door, whichever is the lesser.

P_{em} = bow door pressure, P_b , determined using α_m and β_m in place of α and β .

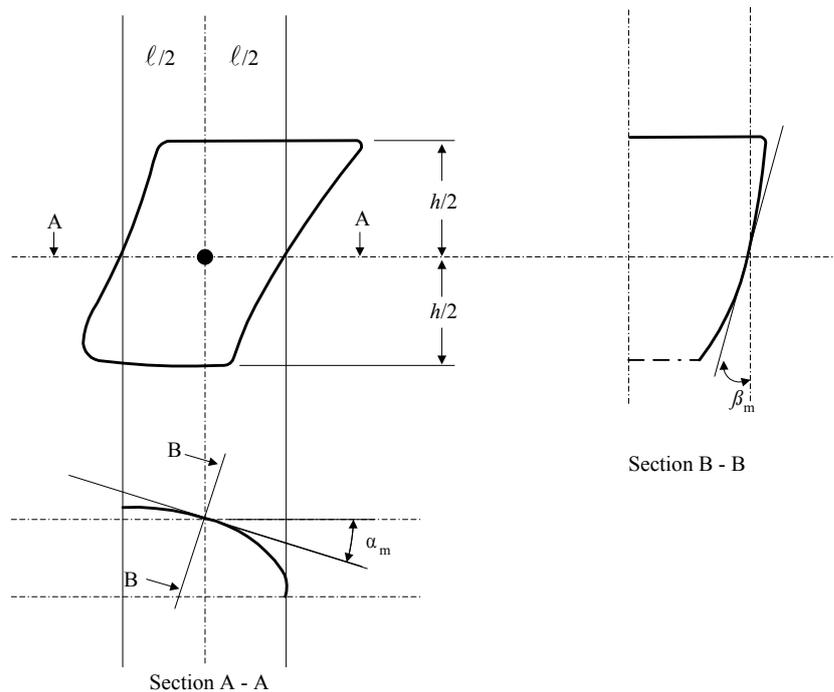
β_m = flare angle measured at a point on the bow door $\ell/2$ aft of the stem line on a plane $h/2$ above the bottom of the door, as shown in 3-2-13/Figure 2.

α_m = entry angle measured at the same point as β_m . See 3-2-13/Figure 2.

h = height, in m (ft), of the door between the levels of the bottom of the door and the upper deck or between the bottom of the door and the top of the door, whichever is less.

ℓ = length, in m (ft), of the door at a height, $h/2$, above the bottom of the door.

FIGURE 2
Definition of α_m and β_m



19.5 Visor Door Forces, Moments and Load Cases

19.5.1 Closing Moment

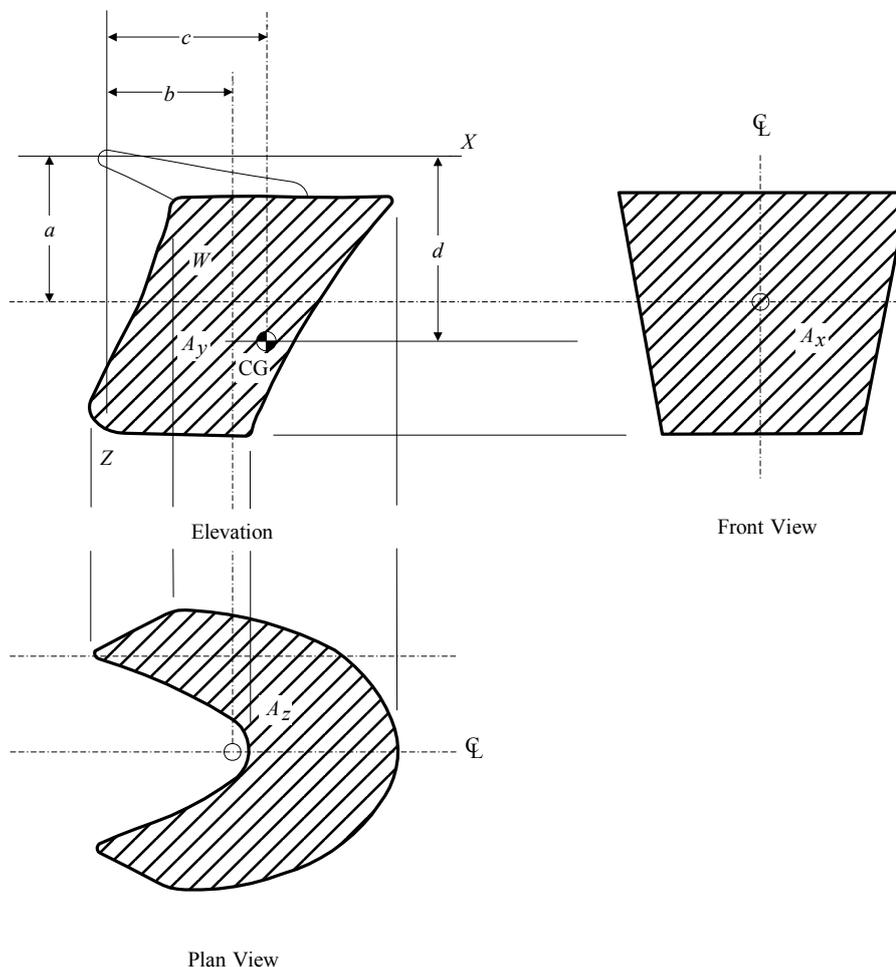
For visor doors, the closing moment, M_y , is to be taken as indicated by the following equation:

$$M_y = F_x a + Wc - F_z b \quad \text{kN-m (tf-m, Ltf-ft)}$$

where F_x and F_z are defined in 3-2-13/19.3

- W = weight of the visor door, in kN (tf, Ltf)
- a = vertical distance, in m (ft), from the visor pivot to the centroid of the transverse vertical projected area of the visor door. See 3-2-13/Figure 3.
- b = horizontal distance, in m (ft), from visor pivot to the centroid of the horizontal projected area of the visor door. See 3-2-13/Figure 3.
- c = horizontal distance, in m (ft), from the visor pivot to the center of gravity of the visor. See 3-2-13/Figure 3.

FIGURE 3
Visor Type Bow Door



19.5.2 Opening Moment

The opening moment, M_o , is to be taken as indicated by the following equation:

$$M_o = Wd + 5A_x a \text{ kN-m} \quad (M_o = Wd + 0.5A_x a \text{ tf-m}, M_o = Wd + 0.047A_x a \text{ Ltf-ft})$$

where

d = vertical distance, in m (ft), from the hinge axis to the center of gravity of the door.

W , A_x and a are as indicated above.

19.5.3 Vertical Design Force

The vertical design force is to be taken as $F_z - W$, where F_z is as defined in 3-2-13/19.3 and W is as defined in 3-2-13/19.5.1.

19.5.4 Combined Load Case 1

The visor doors are to be evaluated under a load of F_x , F_z and W acting simultaneously with F_x and F_z acting at the centroid of their respective projected areas.

19.5.5 Combined Load Case 2

The visor doors are to be evaluated under a load of $0.7F_y$ acting on each side separately, together with $0.7F_x$, $0.7F_z$ and W . F_x , F_y and F_z are to be taken as acting at the centroid of their respective projected areas.

19.7 Side-Opening Door Load Cases

19.7.1 Combined Load Case 1

Side opening doors are to be evaluated under a load of F_x , F_y , F_z and W acting simultaneously with F_x , F_y and F_z acting at the centroid of their respective projected areas.

19.7.2 Combined Load Case 2

Side opening doors are to be evaluated under a load of $0.7F_x$, $0.7F_z$ and W acting on both doors simultaneously and $0.7F_y$ acting on each door separately.

21 Inner Door Design Loads

21.1 External Pressure

The design external pressure is to be taken as the greater of P_{ei} or P_h , as given by the following equations:

$$P_{ei} = 0.45L \text{ kN/m}^2 \text{ (0.046L tf/m}^2, 0.042L \text{ Ltf/ft}^2)$$

$$P_h = 10h \text{ kN/m}^2 \text{ (1.0h tf/m}^2, 0.029h \text{ Ltf/ft}^2)$$

where

L is as defined in 3-1-1/3.

h = the distance, in m (ft), from the load point to the top of the cargo space.

21.3 Internal Pressure

The design internal pressure, P_i , is to be taken as not less than 25 kN/m^2 (2.5 tf/m^2 , 0.23 Ltf/ft^2).

23 Side Shell and Stern Doors (1998)

23.1 Design Forces for Primary Members

The design force, in kN (tf, Ltf), for primary members is to be the greater of the following:

$$\text{External force: } F_e = Ap_e$$

$$\text{Internal force: } F_i = F_o + W$$

23.3 Design Forces for Securing or Supporting Devices of Doors Opening Inwards

The design force, in kN (tf, Ltf), for securing or supporting devices of doors opening inwards is to be the greater of the following:

$$\text{External force: } F_e = Ap_e + F_p$$

$$\text{Internal force: } F_i = F_o + W$$

23.5 Design Forces for Securing or Supporting Devices of Doors Opening Outwards

The design force, in kN (tf, Ltf), for securing or supporting devices of doors opening outwards is to be the greater of the following:

External force: $F_e = Ap_e$

Internal force: $F_i = F_o + W + F_p$

where

A = area, in m² (ft²), of the door opening

W = weight of the door, in kN (tf, Ltf)

F_p = total packing force, in kN (tf, Ltf). Packing line pressure is normally not to be taken less than 5.0 N/mm (0.51 kg/mm, 28.6 lbf/in).

F_o = the greater of F_c and kA , in kN (tf, Ltf)

k = 5 (0.51, 0.047)

F_c = accidental force, in kN (tf, Ltf), due to loose cargo, etc., to be uniformly distributed over the area A and not to be taken less than 300 kN (30.6 tf, 30.1 Ltf). For small doors such as bunker doors and pilot doors, the value of F_c may be appropriately reduced. However, the value of F_c may be taken as zero, provided an additional structure such as an inner ramp is fitted which is capable of protecting the door from accidental forces due to loose cargoes.

p_e = external design pressure, in kN/m² (tf/m², Ltf/ft²), determined at the center of gravity of the door opening and not taken less than:

$$p_e = k_1 \quad \text{for } Z_G \geq d$$

$$p_e = k_2(d - Z_G) + k_1 \quad \text{for } Z_G < d$$

Moreover, for vessels fitted with bow doors, p_e for stern doors is not to be taken less than:

$$p_e = nc(0.8 + 0.6(k_3L)^{0.5})^2$$

For vessels fitted with bow doors and operating in restricted service, the value of p_e for stern doors will be specially considered.

k_1 = 25.0 (2.55, 0.233)

k_2 = 10.0 (1.02, 0.0284)

d = draft, in m (ft), as defined in 3-1-1/9

Z_G = height of the center of area of the door, in m (ft), above the baseline.

n = 0.605 (0.0616, 0.00563)

k_3 = 1.0 (1.0, 0.305)

c = 0.0125L for $L < 80$ m (260 ft)

= 1.0 for $L \geq 80$ m (260 ft)

L = length of vessel, as defined in 3-1-1/3, in m (ft)

25 Allowable Stresses

25.1 Primary Structure and Securing and Supporting Devices

The following stresses are not to be exceeded under the loads indicated above.

$$\text{Shear Stress: } \tau = 80/Q \text{ N/mm}^2 \text{ (} 8.2/Q \text{ kgf/mm}^2, 11600/Q \text{ psi)}$$

$$\text{Bending Stress: } \sigma = 120/Q \text{ N/mm}^2 \text{ (} 12.2/Q \text{ kgf/mm}^2, 17400/Q \text{ psi)}$$

$$\text{Equivalent Stress (} \sqrt{\sigma^2 + 3\tau^2} \text{): } \sigma_e = 150/Q \text{ N/mm}^2 \text{ (} 15.3/Q \text{ kgf/mm}^2, 21770/Q \text{ psi)}$$

where Q is defined in 3-2-1/7.5.

25.3 Steel Securing and Supporting Devices Bearing Stress

For steel to steel bearings in securing and supporting devices, the nominal bearing pressure is not to exceed $0.8\sigma_f$ where σ_f is the yield stress of the bearing material.

25.5 Tensile Stress on Threaded Bolts

The tensile stress in threaded bolts is not to exceed $125/Q \text{ N/mm}^2$ ($12.7/Q \text{ kgf/mm}^2$, $18000/Q \text{ psi}$).

27 Operating and Maintenance Manual

The following information is to be submitted for review.

27.1 Manual (1998)

An operating and maintenance manual for the doors is to be provided onboard and is to contain at least the following:

- Main particulars and design drawings
- Service conditions, e.g., service area restrictions, emergency operations, acceptable clearances for supports
- Maintenance and function testing
- Register of inspections and repairs

27.3 Operating Procedures (1998)

Documented operating procedures for closing and securing the doors are to be kept onboard and posted at an appropriate location.

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PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **14 Bulwarks, Rails, Freeing Ports, Portlights, Windows, Ventilators, Tank Vents, and Overflows**

1 Bulwarks and Guard Rails

1.1 Height

The height of bulwarks and guard rails on exposed freeboard and superstructure decks is to be at least 1 m (39.5 in.). Where this height would interfere with the normal service or operation of a vessel, a lesser height may be approved if adequate protection is provided. Where approval of a lesser height is requested, justifying information is to be submitted.

1.3 Strength of Bulwarks

Bulwarks are to be of ample strength in proportion to their height and efficiently stiffened at the upper edge. The bulwark plating is to be kept clear of the sheerstrake and the lower edge effectively stiffened. For vessels under 61 m (200 ft) in length, the bulwark plating on freeboard decks is to be of a thickness adequate for the intended service of the vessel. For vessels 61 m (200 ft) in length and over, the bulwark plating on freeboard decks is not to be less than 6.5 mm (0.25 in.) in thickness. Bulwarks are to be supported by efficient stays. Stays on freeboard decks are to be spaced not more than 1.83 m (6 ft) apart and are to be efficiently attached to the bulwarks and deck plating. Where it is intended to carry timber deck cargoes, the bulwark stays are to be not over 1.52 m (5 ft) apart and have increased attachment to deck and bulwark. Gangways and other openings in bulwarks are to be kept well away from breaks of superstructures, and heavy plates are to be fitted in way of mooring pipes.

1.5 Guard Rails (1998)

1.5.1

Fixed, removable or hinged stanchions are to be fitted at approximately 1.5 m (5 ft) apart. Removable or hinged stanchions are to be capable of being locked in the upright position.

1.5.2 (2007)

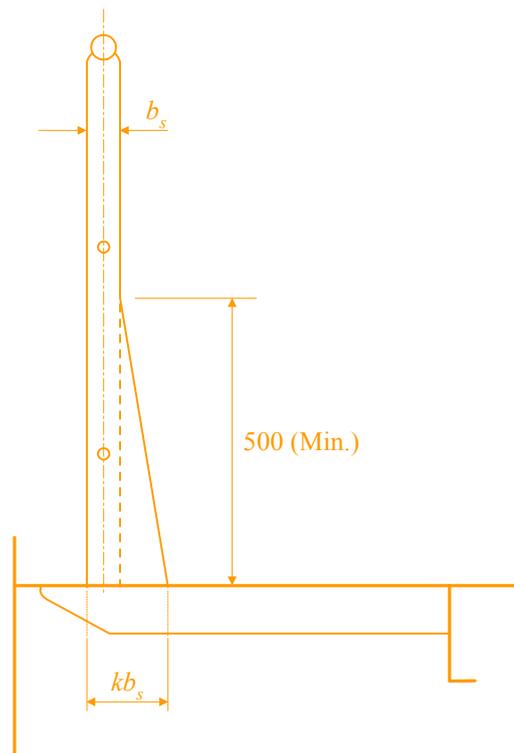
At least every third stanchion is to be supported by a bracket or stay. **Where the arrangements would interfere with the safe traffic of persons on board, the following alternative arrangements of stanchions may be acceptable:**

- i) At least every third stanchion is to be of increased breadth, $kb_s = 2.9b_s$ at the attachment of stanchion to the deck, or,
 - ii) At least every second stanchion is to be of increased breadth, $kb_s = 2.4b_s$ at the attachment of stanchion to the deck, or,
 - iii) Every stanchion is to be of increased breadth, $kb_s = 1.9b_s$ at the attachment of stanchion to the deck.
- where, b_s is the breadth of normal stanchion according to the recognized design standard. (see 3-2-14/Figure 1)

In any arrangement of *i)*, *ii)* or *iii)* above, the following details are to be complied with:

- iv) Flat steel stanchion required by *i)*, *ii)* or *iii)* above is to be aligned with member below deck unless the deck plating thickness exceeds 20 mm (0.79 in.)
- v) The supporting member of the stanchion is to be of 100 × 12 mm (4.0 × 0.5 in.) flat bar welded to deck by double continuous fillet weld with minimum leg size of 7.0 mm ($5/16$ in.) or specified by the recognized design standard.

FIGURE 1
Guardrail Stanchion (2007)



1.5.3

The opening below the lowest course is not to exceed 230 mm (9 in.). The distance between the remaining courses is not to be more than 380 mm (15 in.).

1.5.4

For vessels with rounded gunwales, stanchions are to be placed on the flat of the deck.

3 Access and Crew Protection (*1 July 1998*)

3.1 General

Satisfactory means in the form of guard rails, lifelines, gangways or underdeck passages, etc., are to be provided for the protection of the crew in getting to and from their quarters, the machinery space and all other parts used in the necessary work of the vessel.

3.3 Access to Bow on Tankers

Tankers, including oil carriers, fuel oil carriers, gas carriers and chemical carriers, are to be provided with means to enable the crew to gain safe access to the bow, even in severe weather conditions.

TABLE 1
Acceptable Arrangement for Access (1 July 1998)

Type of vessel	Locations of access in Vessel	Assigned Summer Freeboard	Acceptable arrangements according to type of freeboard assigned			
			Type A	Type B-100	Type B-60	Type B & B+
All Vessels other than Oil Tankers*, Chemical Tankers* and Gas Carriers*	1.1: Access to Midship Quarters	≤ 3000 mm (≤ 118 in.)	a	a	a	a
	1.1.1: Between poop and bridge, or		b	b	b	b
	1.1.2: Between poop and deckhouse containing living accommodation, or navigation equipment, or both.	> 3000 mm (> 118 in.)	e	e	c(1)	c(1)
					e	c(2)
					f(1)	c(4)
			a	a	a	d(1)
			b	b	b	d(2)
			e	e	c(1)	d(3)
					c(2)	e
					e	f(1)
				f(1)	f(2)	
				f(2)	f(4)	
	1.2: Access to Ends	≤ 3000 mm (≤ 118 in.)	a	a	a	
	1.2.1: Between poop and bow (if there is no bridge), or		b	b	b	
	1.2.2: Between bridge and bow, or	> 3000 mm (> 118 in.)	c(1)	c(1)	c(1)	
			e	c(2)	c(2)	
	1.2.3: Between a deckhouse containing living accommodation or navigation equipment, or both, and bow, or	> 3000 mm (> 118 in.)	f(1)	e	e	
				f(1)	f(1)	
	1.2.4: In the case of a flush deck vessel, between crew accommodation and the forward and after ends of vessel.	> 3000 mm (> 118 in.)		f(2)	f(2)	
			a	a	a	
b			b	b		
c(1)			c(1)	c(1)		
		d(1)	c(2)	c(2)		
		e	d(1)	c(4)		
		f(1)	d(2)	d(1)		
			e	d(2)		
			f(1)	d(3)		
				e		
				f(1)		
				f(2)		
				f(4)		
Oil Tankers*, Chemical Tankers* and Gas Carriers*	2.1: Access to Bow	≤ (A _f + H _s)**	e f(1) f(5)			
	2.1.1: Between poop and bow, or					
	2.1.2: Between a deckhouse containing living accommodation or navigation equipment, or both, and bow, or					
	2.1.3: In the case of a flush deck vessel, between crew accommodation and the forward end of vessel.	> (A _f + H _s)**	e f(1) f(2)			

* Oil Tanker, Chemical Tanker and Gas Carrier as defined in SOLAS: II-1/2.12, VII/8.2 and VII/11.2, respectively.

** A_f: the minimum summer freeboard calculated as type A ship, regardless of the type freeboard actually assigned.

H_s: the standard height of superstructure, as defined in ICLL Regulation 33.

I. Construction Keys (a) through (f)

- (a) A well lighted and ventilated underdeck passageway with clear opening at least 0.8 m (2.6 ft) in width and 2.0 m (6.6 ft) in height, providing access to the locations in question and located as close as practicable to the freeboard deck.
- (b) A permanently constructed gangway fitted at or above the level of the superstructure deck on or as near as practicable to the center line of the vessel, providing a continuous platform of a non-slip surface at least 0.6 m (2 ft) in width, with a foot-stop and guard rails extending on each side throughout its length. Guard rails are to be as required in 3-2-14/3.1 and 3-2-14/1.5, except that stanchions are to be fitted at intervals not more than 1.5 m (5 ft).

TABLE 1 (continued)
Acceptable Arrangement for Access (1 July 1998)

- (c) A permanent walkway at least 0.6 m (2 ft) in width, fitted at freeboard deck level, consisting of two rows of guard rails with stanchions spaced not more than 3 m (10 ft). The number of courses of rails and their spacing are to be as required in 3-2-14/1.5. On Type B ships, hatchway coamings not less than 0.6 m (2 ft) in height may be regarded as forming one side of the walkway, provided that two rows of guard rails are fitted between the hatchways.
- (d) A 10 mm (0.4 in.) minimum diameter wire rope lifeline supported by stanchions about 10 m (33 ft) apart, or
 A single hand rail or wire rope attached to hatch coamings, continued and adequately supported between hatchways.
- (e) A permanently constructed gangway fitted at or above the level of the superstructure deck on or as near as practicable to the center line of the vessel:
 - located so as not to hinder easy access across the working areas of the deck;
 - providing a continuous platform at least 1.0 m (3.3 ft) in width*;
 - constructed of fire-resistant and non-slip material;
 - fitted with guard rails extending on each side throughout its length. Guard rails are to be as required in 3-2-14/3.1 and 3-2-14/1.5, except that stanchions are to be fitted at intervals not more than 1.5 m (5 ft);
 - provided with a foot stop on each side;
 - having openings, with ladders where appropriate, to and from the deck. Openings are to be not more than 40 m (131 ft) apart;
 - having shelters of substantial construction set in way of the gangway at intervals not exceeding 45 m (148 ft) if the length of the exposed deck to be traversed exceeds 70 m (230 ft). Every such shelter is to be capable of accommodating at least one person and be so constructed as to afford weather protection on the forward, port and starboard sides.
- (f) A permanent and efficiently constructed walkway fitted at freeboard deck level on or as near as practicable to the center line of the vessel having the same specifications as those for a permanent gangway listed in (e)*, except for foot-stops. On Type B ships certified for the carriage of liquids in bulk, the hatch coamings may be accepted as forming one side of the walkway, provided a combined height of hatch coaming and hatch cover in the closed condition is not less than 1 m (3.3 ft) and two rows of guard rails are fitted between the hatchways.

(*) For tankers less than [100 m (328 ft)] in length, the minimum width of the gangway platform or deck level walkway fitted in accordance with arrangement (e) or (f), respectively, may be reduced to 0.6 m (2 ft).

II. Transverse Location Keys (1) through (5) - for Construction (c), (d) and (f) where specified in the Table

- (1) At or near the center line of vessel; or fitted on hatchways at or near the center line of vessel.
- (2) Fitted on each side of the vessel.
- (3) Fitted on one side of the vessel, provision being made for fitting on either side.
- (4) Fitted on one side only.
- (5) Fitted on each side of the hatchways as near to the center line as practicable.

III. Notes:

- 1. In all cases where wire ropes are fitted, adequate devices are to be provided to enable maintaining their tautness.
- 2. A means of passage over obstructions, if any, such as pipes or other fittings of a permanent nature is to be provided.
- 3. Generally, the width of the gangway or walkway should not exceed 1.5 m (5 ft).

5 Freeing Ports

5.1 Basic Area

Where bulwarks on freeboard decks form wells, ample provision is to be made for rapidly freeing the decks of water and for draining them. The minimum freeing-port area on each side of the vessel for each well 20 m (66 ft) or less in length is to be obtained from the following equation:

$$A = 0.7 + 0.035\ell \text{ m}^2 \qquad A = 7.6 + 0.115\ell \text{ ft}^2$$

Where the bulwark length exceeds 20 m (66 ft):

$$A = 0.07\ell \text{ m}^2 \qquad A = 0.23\ell \text{ ft}^2$$

where

A = freeing-port area, in m^2 (ft^2)

ℓ = bulwark length in m (ft), but need not exceed $0.7L$

If a bulwark is more than 1.2 m (3.9 ft) in height, the freeing-port area is to be increased by 0.004 m^2 per meter (0.04 ft^2 per foot) of length of well for each 0.1 m (1 ft) difference in height. If a bulwark is less than 0.9 m (3 ft) in height, the freeing port area may be decreased by the same ratio. In vessels with no sheer, the calculated area is to be increased by 50%. Where sheer is less than standard, the percentage is to be obtained by interpolation.

5.3 Trunks, Deckhouses, and Hatchway Coamings

Where a vessel is fitted with a trunk on the freeboard deck, and open rails are not fitted in way of the trunk for at least one-half its length, or where continuous or substantially continuous hatchway side coamings are fitted or long deckhouse exist between detached superstructures, the minimum area of freeing-port openings is to be obtained from the following table.

<i>Breadth of trunk, deckhouse or hatchway in relation to breadth of vessel</i>	<i>Area of freeing ports in relation to total area of bulwarks</i>
40% or less	20%
75% or more	10%

The area of freeing ports at intermediate breadths is to be obtained by linear interpolation.

5.5 Superstructure Decks

Where bulwarks on superstructure decks form wells, the bulwarks are to comply with 3-2-14/5.1, except that the minimum freeing-port area on each side of the vessel for each well is to be one-half of the area obtained in 3-2-14/5.1 and 3-2-14/5.3.

5.7 Open Superstructures

In vessels having superstructures that are open at either end or both ends, adequate provisions for freeing the spaces within such superstructures are to be provided. The arrangements will be subject to special approval.

5.9 Details of Freeing Ports

The lower edges of the freeing ports are to be as near the deck as practicable. Two-thirds of the required freeing-port area is to be provided in the half of the well nearest the lowest point of the sheer curve. Freeing-port openings are to be protected by rails or bars in such a manner that the maximum clear vertical or horizontal space is 230 mm (9 in.). Where shutters are fitted, ample clearance is to be provided to prevent them from jamming. Hinges are to have pins and bearings of corrosion-resistant material and, in general, the hinges are to be located at the top of the shutter. If the shutters are equipped with securing appliances, the appliances are to be of approved construction.

7 Portlights

7.1 Application (1 July 1998)

This subsection applies to passenger vessels and cargo vessels with the keel laid or in similar stage of construction on or after 1 July 1998. As such, any reference to bulkhead/freeboard deck means bulkhead deck in the case of passenger vessels and freeboard deck in the case of cargo vessels.

7.3 Location (1 July 1998)

The lower edges of the portlight sills are not to be below a line drawn parallel to the bulkhead/freeboard deck at side having as its lowest point either 2.5% of the breadth of the vessel or 500 mm (19.5 in.) above the designed load waterline, whichever is the greater distance.

In addition, portlights are not to be fitted in spaces which are used exclusively for the carriage of cargo.

7.5 Construction (1 July 1998)

7.5.1 General

Portlights to spaces below the bulkhead/freeboard deck or to spaces within enclosed superstructures and deckhouses protecting openings leading to below the bulkhead/freeboard deck are to be fitted with efficient hinged, inside deadlights arranged so that they can be effectively closed and secured watertight. The portlights, together with their glasses and deadlights, are to comply with a recognized standard. They are to have strong frames (other than cast iron) and opening-type portlights are to have noncorrosive hinge pins.

7.5.2 Non-opening Type

Where vessels are subject to damaged stability requirements of 3-3-1/3.3, portlights found to be situated below a final damage equilibrium waterline are to be of non-opening type.

7.5.3 Locked Type

Portlights, where permitted in 3-2-14/7.5.2 to be of opening type, are to be of such construction as will prevent unauthorized opening where:

- i) The sills of which are below the bulkhead/freeboard deck, as permitted in 3-2-14/7.3, or
- ii) Fitted in spaces used alternatively for the carriage of cargo or passengers.

7.5.4 Automatic Ventilating Type

Automatic ventilating portlights are not to be fitted in the shell plating below the bulkhead/freeboard deck without special approval.

9 Windows

9.1 Construction

Windows should generally not be fitted in deckhouses or the end bulkheads of superstructures in Position 1.

Windows to spaces within enclosed superstructure and deckhouses are to be fitted with strong, steel deadlight covers.

Windows in other locations may be fitted without deadlight covers, depending upon the arrangement of the vessel. Window frames are to be metal or other approval material and effectively secured to the adjacent structure. Windows are to have a minimum of a 1/4-inch radius at all corners. The glazing is to be set into the frames in a suitable, approved packing or compound. Special consideration is to be given to angled house fronts.

The thickness of the window is not to be less than that obtained from 3-2-14/9.1.1, 3-2-14/9.1.2 or 3-2-14/9.1.3 below, whichever is greatest.

9.1.1

$$t = s \sqrt{\frac{pk}{1000\sigma_a}} \text{ mm} \qquad t = s \sqrt{\frac{pk}{\sigma_a}} \text{ in.}$$

9.1.2

$$t = s \sqrt[3]{\frac{pk_1}{20E}} \text{ mm} \qquad t = s \sqrt[3]{\frac{pk_1}{0.02E}} \text{ in.}$$

9.1.3 Minimum Tempered Monolithic Glass Thicknesses:

$$t = 9.5 \text{ mm (0.37 in.) for front windows}$$

$$t = 6.5 \text{ mm (0.25 in.) for side and end windows}$$

where

$$t = \text{required window thickness, in mm (in.)}$$

$$s = \text{lesser dimension of window, in mm (in.)}$$

$$h = \text{pressure head, in m (ft), given in 3-2-9/3.3}$$

$$p = 9.8h \text{ kN/m}^2 \text{ (0.44}h \text{ psi)}$$

$$k = \text{factor given in 3-2-14/Table 2}$$

$$k_1 = \text{factor given in 3-2-14/Table 2}$$

$$\sigma_a = 0.30\sigma_f$$

$$\sigma_f = \text{material flexural strength; see 3-2-14/Table 3}$$

$$E = \text{material flexural modulus; see 3-2-14/Table 3}$$

TABLE 2

ℓ/s	k	k_1
>5	0.750	0.142
5	0.748	0.142
4	0.741	0.140
3	0.713	0.134
2	0.610	0.111
1.8	0.569	0.102
1.6	0.517	0.091
1.4	0.435	0.077
1.2	0.376	0.062
1	0.287	0.044

ℓ = greater dimension of window panel, in mm (in.)

s = lesser dimension of window panel, in mm (in.)

TABLE 3

<i>Glazing</i>	<i>Flexural Strength</i>	<i>Flexural Modulus</i>
Tempered Monolithic	119 Mpa (17,200 psi)	73,000 MPa (10,600,000 psi)
Laminated Glass	69 Mpa (10,000 psi)	2,620 MPa (380,000 psi)
Polycarbonate*	93 Mpa (13,500 psi)	2,345 MPa (340,000 psi)
Acrylic (poly methyl methacrylate)*	110 MPa (16,000 psi)	3,000 MPa (435,000 psi)

* Indicated values are for reference. Aging effects are to be considered for design.

9.3 Testing

All windows and portlights are to be hose-tested after installation.

11 Ventilators, Tank Vents, and Overflows (2004)

11.1 General (2004)

Ventilators are to comply with the requirements of 3-2-14/11.3. Tank vents and overflows are to comply with the requirements in 3-2-14/11.5. In addition, for those located on the fore deck of vessels with length, L , (as defined in 3-1-1/3.1) between 80 meters (263 feet) and 90 meters (295 feet), the requirements given in 3-2-14/11.7 are to be complied with.

11.3 Ventilators (2004)

11.3.1 Coaming Construction

Ventilators on exposed freeboard decks, superstructure deck or deckhouses are to have coamings of steel or equivalent material. Coaming plate thicknesses are to be obtained from the following equation.

$$t = 0.01d + 5.5 \text{ mm}$$

$$t = 0.01d + 0.22 \text{ in.}$$

where

t = thickness of coaming in mm (in.)

d = diameter of ventilator in mm (in.), but not less than 200 mm (7.5 in.)

The maximum coaming plate thickness required is 10 mm (0.40 in.). The coamings are to be effectively secured to the deck. Coamings which are more than 900 mm (35.5 in.) high and which are not supported by adjacent structures are to have additional strength and attachment. Ventilators passing through superstructures, other than enclosed superstructures, are to have substantially constructed coamings of steel at the freeboard deck.

11.3.2 Coaming Height

Ventilators in Position 1 are to have coamings at least 900 mm (35.5 in.) high. Ventilators in Position 2 are to have coamings at least 760 mm (30 in.) high. For definitions of Position 1 and Position 2, see 3-2-12/5. Coaming heights may be reduced on vessels which have freeboard in excess of the minimum geometric freeboard and/or a superstructure deck with the height of the deck in excess of the standard height of a superstructure.

11.3.3 Means for Closing Ventilators

Except as provided below, ventilator openings are to be provided with efficient, permanently attached closing appliances. In vessels measuring 24 m (79 ft) or more in length (as defined in the International Convention on Load Lines, 1966), ventilators in Position 1, the coamings of which extend to more than 4.5 m (14.8 ft) above the deck and in Position 2, the coamings of which extend to more than 2.3 m (7.5 ft) above the deck, need not be fitted with closing arrangements.

These coaming height requirements may be modified in vessels measuring less than 24 m (79 ft) in length.

11.5 Tank Vents and Overflows (2004)

Tank vents and overflows are to be in accordance with the requirements of 4-4-3/9 and 4-4-3/11 of these Rules and, where applicable, the requirements given below in 3-2-14/11.7.

11.7 Ventilators, Tank Vents and Overflows on the Fore Deck (2004)

11.7.1 Application

The requirements of this paragraph apply to all ventilators, tank vents and overflows located on the exposed fore deck within the forward $0.25L$ on vessels with length, L , (as defined in 3-1-1/3.1) between 80 meters (263 feet) and 90 meters (295 feet) and where the height of the exposed deck in way of the item is less than $0.1L$ or 22 meters (72 ft) above the summer load waterline, whichever is the lesser.

11.7.2 Applied Loading to the Air Pipes and Ventilators

11.7.2(a) Pressure. The pressures, p , in kN/m^2 (tf/m^2 , Ltf/ft^2), acting on air pipes, ventilator pipes and their closing devices, may be calculated from:

$$p = f \rho V^2 C_d C_s C_p \quad \text{kN/m}^2 \text{ (tf/m}^2, \text{ Ltf/ft}^2\text{)}$$

where:

f = 0.5 (0.05, 0.0156)

ρ = density of sea water, 1.025 t/m^3 (1.025 t/m^3 , 0.0286 Lt/ft^3)

V = velocity of water over the fore deck, 13.5 m/sec (44.3 ft/sec)

- C_d = shape coefficient
 = 0.5 for pipes
 = 1.3 for pipes or ventilator heads in general
 = 0.8 for pipes or ventilator heads of cylindrical form with its axis in the vertical direction
- C_s = slamming coefficient, 3.2
- C_p = protection coefficient:
 = 0.7 for pipes and ventilator heads located immediately behind a breakwater or forecastle
 = 1.0 elsewhere including immediately behind a bulwark

11.7.2(b) *Force.* Forces acting in the horizontal direction on the pipe and its closing device may be calculated from the above pressure using the largest projected area of each component.

11.7.3 Strength Requirements for Ventilators, Tank Vents and Overflows and their Closing Devices

11.7.3(a) *Bending Moment and Stress.* Bending moments and stresses in air pipes and ventilator pipes are to be calculated at critical positions; at penetration pieces, at weld or flange connections, at toes of supporting brackets. Bending stresses in the net section are not to exceed $0.8Y$, where Y is the specified minimum yield stress or 0.2% proof stress of the steel at room temperature. Irrespective of corrosion protection, a corrosion addition to the net section of 2.0 mm (0.08 in.) is then to be applied.

11.7.3(b) Tank Vents and Overflows

- i) For standard tank vents and overflows of 760 mm (30 in.) height closed by heads of not more than the tabulated projected area, pipe thicknesses and bracket heights are specified in 3-2-14/Table 4. Where brackets are required, three or more radial brackets are to be fitted.
- ii) Brackets are to be of gross thickness of 8 mm (0.32 in.) or more, of minimum length of 100 mm (4.0 in.) and height according to 3-2-14/Table 4, but need not extend over the joint flange for the head. Bracket toes at the deck are to be suitably supported.
- iii) For other configurations, loads according to 3-2-14/11.7.2 are to be applied and means of support determined in order to comply with the requirements above. Brackets, where fitted, are to be of suitable thickness and length according to their height.
- iv) Final (gross) pipe thickness is not to be taken less than as indicated in 4-4-3/9.3.
- v) The minimum internal diameter of the air pipe or overflow is not to be less than 65 mm.

11.7.3(c) Ventilators

- i) For standard ventilators of 900 mm (35.4 in.) height closed by heads of not more than the tabulated projected area, pipe thicknesses and bracket heights are specified in 3-2-14/Table 5. Brackets, where required, are to be as specified in 3-2-14/11.7.3(b)iii).
- ii) For ventilators of height greater than 900 mm (35.4 in.), brackets or alternative means of support are to be provided. Coamings are not to be taken less than as indicated in 3-2-14/11.3 nor in 3-2-14/Table 4.

11.7.3(d) *Components and Connections.* All component parts and connections of the tank vents and overflows or ventilators are to be capable of withstanding the loads defined in 3-2-14/11.7.2.

11.7.3(e) *Rotary Heads.* Rotating type mushroom ventilator heads are not to be used for application in this location.

TABLE 4
760 mm (30 in.) High Tank Vents and Overflows
Thickness and Bracket Standards (2004)

Nominal Pipe Size		Minimum Fitted Gross Thickness		Maximum Projected Area of Head		Height ⁽¹⁾ of Brackets	
A	B	mm	in.	cm ²	in ²	mm	in.
65	2½	6.0	---	---	---	480	18.9
80	3	6.3	0.25	---	---	460	18.1
100	4	7.0	0.28	---	---	380	15.0
125	5	7.8	0.31	---	---	300	11.8
150	6	8.5	0.33	---	---	300	11.8
175	7	8.5	0.33	---	---	300	11.8
200	8	8.5 ⁽²⁾	0.33 ⁽²⁾	1900	295	300	11.8
250	10	8.5 ⁽²⁾	0.33 ⁽²⁾	2500	388	300 ⁽²⁾	11.8 ⁽²⁾
300	12	8.5 ⁽²⁾	0.33 ⁽²⁾	3200	496	300 ⁽²⁾	11.8 ⁽²⁾
350	14	8.5 ⁽²⁾	0.33 ⁽²⁾	3800	589	300 ⁽²⁾	11.8 ⁽²⁾
400	16	8.5 ⁽²⁾	0.33 ⁽²⁾	4500	698	300 ⁽²⁾	11.8 ⁽²⁾

Notes:

- 1 Brackets [see 3-2-14/11.7.3(b)] need not extend over the joint flange for the head.
- 2 Brackets are required where the as fitted (gross) thickness is less than 10.5 mm (0.41 in.), or where the tabulated projected head area is exceeded.

Note: For other air pipe heights, the relevant requirements of 3-2-14/11.7.3 are to be applied.

TABLE 5
900 mm (35.4 in.) High Ventilator
Thickness and Bracket Standards (2004)

Nominal Pipe Size		Minimum Fitted Gross Thickness		Maximum Projected Area of Head		Height ⁽¹⁾ of Brackets	
A	B	mm	in.	cm ²	in ²	mm	in.
80	3	6.3	0.25	-	-	460	18.1
100	4	7.0	0.28	-	-	380	15.0
150	6	8.5	0.33	-	-	300	11.8
200	8	8.5	0.33	550	85	-	-
250	10	8.5	0.33	880	136	-	-
300	12	8.5	0.33	1200	186	-	-
350	14	8.5	0.33	2000	310	-	-
400	16	8.5	0.33	2700	419	-	-
450	18	8.5	0.33	3300	511	-	-
500	20	8.5	0.33	4000	620	-	-

Note: For other ventilator heights, the relevant requirements of 3-2-14/11.7.3 are to be applied.

PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **15 Ceiling, Sparring, and Protection of Steel**

1 Ceiling

In cargo holds of vessels with single bottoms, close ceiling is to be fitted on the floors and up to the upper turns of the bilges. The ceiling on the floors is to be laid in portable sections or other convenient arrangements are to be made for removal for cleaning, painting or inspection. The ceiling may be omitted where the bottom is filled with cement to the tops of the floors.

In cargo holds of vessels with double bottoms, close ceiling is to be fitted from the outboard edges of the double bottoms up to the upper turns of the bilges. Under all cargo hatches, either ceiling is to be fitted or the thickness of the inner bottom is to be increased by 2 mm (0.08 in.). Ceiling fitted at the bilges is to be removable for cleaning, painting or inspection. Ceiling fitted on the inner bottom plating either is to be laid on battens for drainage or is to be bedded in a suitable composition.

The thickness of wood ceiling is not to be less than 25 mm (1 in.) in vessels 9 m (30 ft) in length, not less than 50 mm (2 in.) in vessels between 20 m (65 ft) and 61 m (200 ft) in length, not less than 57 mm (2.25 in.) in vessels 61 to 76 m (200 to 250 ft) in length, nor less than 63 mm (2.5 in.) in vessels over 76 m (250 ft) in length. Between 9 m (30 ft) and 20 m (65 ft) in length, the thicknesses may be determined by interpolation.

3 Sparring

In spaces intended to carry general cargo, sparring, where fitted, is to be arranged between the bilge ceiling and the beam brackets. In vessels over 20 m (65 ft) in length, sparring is not to provide less protection to the framing than would be obtained from wood battens 40 mm (1.625 in.) thick, 140 mm (5.5 in.) wide, and spaced 380 mm (15 in.) center to center. In vessels 9 m (30 ft) in length, the thickness of wood battens may be reduced to 20 mm (0.8 in.). Between 9 m (30 ft) and 20 m (65 ft) in length, the thicknesses may be proportioned. Sparring is to be portable and fitted in cleats or in portable frames. If sparring is not fitted, the notation **NS** will be entered in the *Record*, indicating no sparring.

5 Protection of Steel Work

5.1 All Spaces

Unless otherwise approved, all steel work is to be suitably coated with paint or equivalent.

5.3 Salt Water Ballast Space

Tanks or holds for salt water ballast are to have a corrosion-resistant hard type coating such as epoxy or zinc on all structural surfaces. Where a long retention of salt water is expected due to the type of vessel or unit, special consideration for the use of inhibitors or sacrificial anodes may be given.

5.5 Oil Spaces

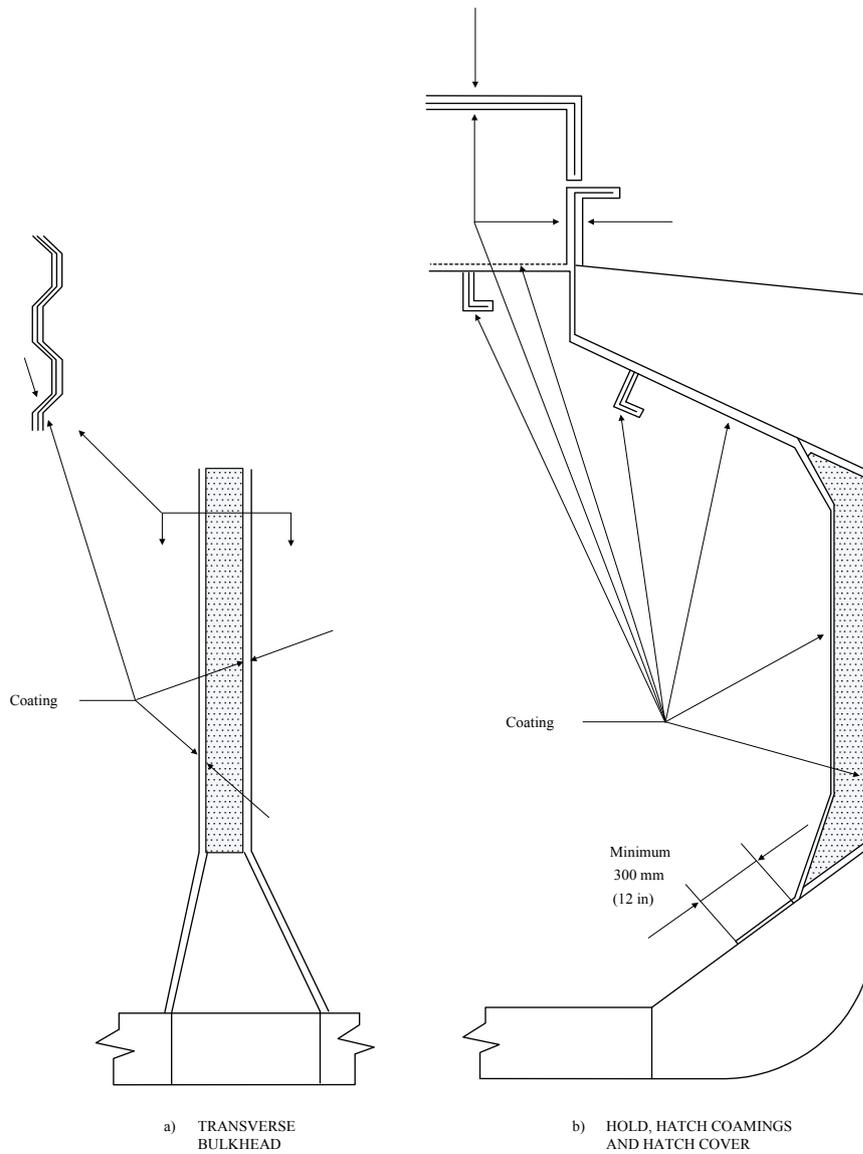
Tanks intended for oil or the holds of combination carriers intended for the carriage of dry bulk cargoes and oil cargoes need not be coated unless required by 3-2-15/5.7.

5.7 Cargo Holds on Bulk Carriers (including Combination Carriers) (1998)

All internal and external surfaces of hatch coamings and hatch covers, and all internal surfaces of cargo holds, excluding the flat tank top areas and the hopper tank sloping plating up to approximately 300 mm (12 in.) below the side shell frame end brackets, are to have an epoxy or equivalent coating applied in accordance with the manufacturer's recommendations. The internal surface of the cargo hold includes those surfaces of stiffening members of the top wing tank bottom, where fitted on the hold side, and deck plating and associated beams, girders, etc. facing holds such as those between the main hatchways. See 3-2-15/Figure 1.

In the selection of coatings, due consideration is to be given by the Owner to the intended cargoes and conditions expected in service.

FIGURE 1
Extent of Coatings (1998)



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PART

3

CHAPTER **2 Hull Structures and Arrangements**

SECTION **16 Weld Design**

1 Fillet Welds

1.1 General

1.1.1 Plans and Specifications

The actual sizes of fillet welds are to be indicated on detail drawings or on a separate welding schedule and submitted for approval in each individual case.

1.1.2 Workmanship

Completed welds are to be to the satisfaction of the attending Surveyor. The gaps between the faying surfaces of members being joined should be kept to a minimum. Where the opening between members being joined exceeds 2.0 mm ($1/16$ in.) and is not greater than 5 mm ($3/16$ in.), the weld leg size is to be increased by the amount of the opening in excess of 2.0 mm ($1/16$ in.). Where the opening between members is greater than 5 mm ($3/16$ in.), corrective procedures are to be specially approved by the Surveyor.

1.1.3 Special Precautions

Special precaution such as the use of preheat or low-hydrogen electrodes or low-hydrogen welding processes may be required where small fillets are used to attach heavy plates or sections. When heavy sections are attached to relatively light plating, the weld size may be required to be modified.

1.3 Tee Connections

1.3.1 Size of Fillet Welds

Frames, beams, bulkheads stiffeners, floors and intercostals, etc. are to have at least the disposition and sizes of intermittent or continuous fillet welds, as required by 3-2-16/Table 1. Where it is desirable to substitute continuous welding for intermittent welding, as given in 3-2-16/Table 1, a reduction from the required size of fillet may be allowed if equivalent strength is provided.

1.3.2 Intermittent Welding at Intersection

Where beams, stiffeners, frames, etc., are intermittently welded and pass through slotted girders, shelves or stringers, there is to be a pair of matched intermittent welds on each side of each such intersection and the beams, stiffeners and frames are to be efficiently attached to the girders, shelves and stringers.

1.3.3 Welding of Longitudinal to Plating

Welding of longitudinals to plating is to have double continuous welds at the ends and in way of transverses equal in length to the depth of the longitudinal. For deck longitudinals only, a matched pair of welds is required at the transverses.

1.3.4 Stiffeners and Webs to Hatch Covers

Unbracketed stiffeners and webs of hatch covers are to be welded continuously to the plating and to the face plate for a length at ends equal to the end depth of the member.

1.5 Tee Type End Connections

Tee type end connections, where fillet welds are used, are to have continuous welds on each side. In general, the sizes of the welds, w , are not to be less than $3/4$ times the thickness of the member being attached, but in special cases where heavy members are attached to relatively light plating, the sizes may be modified. In certain cases, only the webs of girders, beams and stiffeners need be attached. In such cases, it is recommended that the unattached face plates or flanges be cut back.

1.7 Tee Joints at Boundary Connections

Tee joints at boundary connections of bulkheads, decks, inner bottoms, etc. are to have continuous welding on both sides where the thinner of the plates is 12.5 mm ($1/2$ in.) thick or greater. In general, the size of the welds, w , is to be such that the two together are not less than the thickness of the thinner plate plus 1.5 mm ($1/16$ in.). Where the thickness of the thinner plate is less than 12.5 mm ($1/2$ in.), the attachment may be made by a continuous weld on one side 1.5 mm ($1/16$ in.) less than the thickness of the thinner plate with intermittent welding on the opposite side of the size required by 3-2-16/Table 1 for stiffeners to deep tank bulkheads, except in way of tanks where equivalent continuous welds are to be used.

1.9 Ends of Unbracketed Stiffeners

Unbracketed stiffeners of shell, watertight and oiltight bulkheads and house fronts are to have double continuous welds for one-tenth of their length at each end.

Unbracketed stiffeners of nontight structural bulkheads, deckhouse sides and after ends are to have a pair of matched intermittent welds at each end.

1.11 Reduced Weld Size

Reduction in fillet weld sizes, except for slab longitudinals of thickness greater than 25 mm (1.0 in.), may be specially approved by the Surveyor, in accordance with either 3-2-16/1.11.1 or 3-2-16/1.11.2, provided the requirements of 3-2-16/1.3 are satisfied.

1.11.1 Controlled Gaps

Where quality control facilitates working to a gap between members being attached of 1 mm (0.04 in.) or less, a reduction in fillet weld leg size w of 0.5 mm (0.02 in.) may be permitted.

1.11.2 Deep Penetration Welds

Where automatic double continuous fillet welding is used and quality control facilitates working to a gap between members being attached of 1 mm (0.04 in.) or less, a reduction in fillet weld leg size of 1.5 mm ($1/16$ in.) may be permitted, provided that the penetration at the root is at least 1.5 mm ($1/16$ in.) into the members being attached.

1.13 Lapped Joints

Lapped joints are generally to have overlaps of not less width than twice the thinner plate thickness plus 25 mm (1 in.).

1.13.1 Overlapped End Connections

Overlapped end connections of longitudinal strength members within the midship $0.4L$ are to have continuous fillet welds on both edges, each equal in size w to the thickness of the thinner of the two plates joined. All other overlapped end connections are to have continuous welds on each edge of sizes w such that the sum of the two is not less than 1.5 times the thickness of the thinner plate.

1.13.2 Overlapped Seams

Overlapped seams are to have welds on both edges of the sizes required by 3-2-16/1.7 for tee-connections at boundaries.

1.15 Plug Welds or Slot Welds

Plug welds or slot welds may be specially approved for particular applications. Where used in the body of doublers and similar locations, such welds may be spaced about 305 mm (12 in.) between centers in both directions.

3 Full or Partial Penetration Corner or Tee Joints

A full or partial penetration weld may be required for highly stressed (75% or more of the yield) critical (e.g., oil/water boundary) joints.

The designer is to give consideration to minimizing the possibility of lamellar tearing in such joints. Ultrasonic inspection of the plate in way of the connection may be required prior to and after fabrication to assure the absence of possible laminations and lamellar tearing.

5 Alternatives

The foregoing are considered minimum requirements for electric-arc welding in hull construction, but alternate methods, arrangements and details will be considered for approval. Fillet weld sizes may be determined from structural analyses based on sound engineering principles, provided they meet the overall strength standards of the Rules.

TABLE 1
Weld Sizes and Spacing – Millimeters

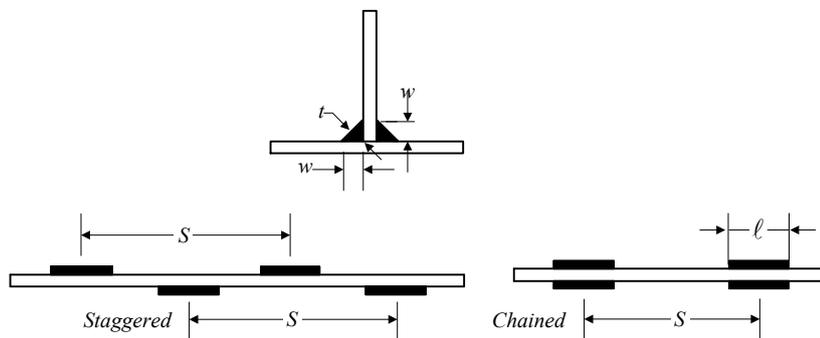
For weld requirements for thicknesses intermediate to those shown in the table, use the nearest thickness shown in the table.

Where beams, stiffeners, frames, etc., are intermittently welded and pass through slotted girders, shelves or stringers, there is to be a pair of matched intermittent welds on each side of each such intersection and the beams, stiffeners and frames are to be efficiently attached to the girders, shelves and stringers.

For slab longitudinals, the attachment is to be made by double continuous fillet welds of a size w which is 0.3 times the thickness of the thinner plate, but need not be greater than 8.0 mm.

Where automatic double continuous fillet welding is provided, a reduction in fillet size of 1.5 mm will be permitted, provided that the specified size of fillet in 3-2-16/Table 1 is 6.5 mm or greater, the gap between the members does not exceed 1.0 mm and the penetration at the root is at least 1.5 mm into the member being attached. This reduction does not apply for slab longitudinals.

For double continuous welding as an alternative to intermittent welding, see 3-2-16/1.3.1.



	Weld size for lesser thickness of members joined, mm							
	5	6.5	8	9.5	11	12.5	14.5	16
Nominal leg size of fillet w	3	5	6.5	6.5	8	8	8	8
Nominal throat size of fillet t	2	3.5	4.5	4.5	5.5	5.5	5.5	5.5
Length of fillet weld	40	65	75	75	75	75	75	75

Structural Items	Spacing of Welds S , mm							
Single-Bottom Floors To center keelson <i>Note:</i> Connections elsewhere to take same weld as floors in double bottom	In accordance with 3-2-16/1.7							
Double-Bottom Floors To shell in aft peaks of vessels having high power and fine form	—	—	150	125	150	150	150	125
To shell flat of bottom forward (fore-end strengthening) and in peaks	—	—	250	225	250	250	225	200
To shell elsewhere	*300	*300	300	275	300	275	250	250
Solid floors to center vertical keel plate in engine room, under boiler bearers, wide-spaced floors with longitudinal frames	In accordance with 3-2-16/1.7							
Solid floors to center vertical keel plate elsewhere, and open-floor brackets to center vertical keel	*250	*250	250	225	250	225	200	175
Solid floors and open-floor brackets to margin plate	In accordance with 3-2-16/1.7							
To inner bottom in engine room	In accordance with 3-2-16/1.7							
To inner bottom at forward end (fore-end strengthening)	*275	*275	275	250	275	250	225	200
To inner bottom elsewhere	*300	*300	300	275	300	275	250	250
Wide spaced with longitudinal framing to shell and inner bottom	In accordance with 3-2-16/1.7							
Solid floor stiffeners at watertight or oiltight boundaries	300	300	300	275	300	275	250	250
Watertight and oiltight periphery connections of floors throughout double bottom	In accordance with 3-2-16/1.7							

TABLE 1 (continued)
Weld Sizes and Spacing – Millimeters

	<i>Weld size for lesser thickness of members joined, mm</i>							
	5	6.5	8	9.5	11	12.5	14.5	16
Nominal leg size of fillet <i>w</i>	3	5	6.5	6.5	8	8	8	8
Nominal throat size of fillet <i>t</i>	2	3.5	4.5	4.5	5.5	5.5	5.5	5.5
Length of fillet weld	40	65	75	75	75	75	75	75
<i>Structural Items</i>	<i>Spacing of Welds S, mm</i>							
Center Girder								
Nontight to inner-bottom or center strake in way of engine and to shell or bar keel	In accordance with 3-2-16/1.7							
Nontight to inner-bottom or center strake clear of engine	150	150	150	125	150	125	125	125
Watertight or oiltight to inner bottom, rider plate, shell or bar keel	In accordance with 3-2-16/1.7							
Intercostals								
Intercostals and continuous longitudinal girders to shell on flat bottom forward (fore-end strengthening) and to inner bottom in way of engines	—	150	150	125	150	125	125	‡Dbl. Cont.
Intercostals and continuous longitudinal girders to shell and inner bottom elsewhere and to floors	*275	*275	275	250	275	250	225	225
Watertight and oiltight periphery connections of longitudinal girders in double bottom	In accordance with 3-2-16/1.7							
Frames								
To shell in aft peaks of vessels having high power and fine form	—	—	150	125	150	150	150	125
To shell for 0.125 <i>L</i> forward and in peaks	—	—	250	225	250	250	225	225
To shell elsewhere—See Note 1	*300	*300	300	275	300	275	250	250
Unbracketed to inner bottom	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Frame brackets to frames, decks and inner bottom	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Longitudinals to shell and inner bottom	*300	*300	300	275	300	275	250	250
Longitudinals to shell on flat of bottom forward (fore-end strengthening)	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
Girders and Webs								
To shell and to bulkheads or decks in tanks	—	200	225	200	225	200	175	150
To bulkheads or decks elsewhere	—	—	250	225	250	225	200	175
Webs to face plate where area of face plate is 64.5 sq. cm. or less	*250	*250	300	275	300	275	250	250
Webs to face plate area of face plate exceeds 64.5 sq. cm	—	—	250	225	250	225	200	175
Bulkheads								
Peripheries of swash bulkheads	—	200	225	200	225	200	175	150
Peripheries of nontight structural bulkheads	—	225	250	225	250	225	200	175
Peripheries of deep tank or watertight bulkheads	In accordance with 3-2-16/1.7							
Stiffeners to deep tank bulkheads – See Note 1	—	*300	300	275	300	275	250	250
Stiffeners to ordinary watertight bulkheads and deckhouse fronts – See Note 1	—	*300	300	275	300	275	250	250
Stiffeners to nontight structural bulkheads; stiffeners on deckhouse sides and after ends – See Note 2	*300	*300	*‡300	300	‡300	300	300	250
Stiffener brackets to beams, decks, etc.	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Decks								
Peripheries of platform decks and nontight flats								
Upper Weld	Cont.	Cont.	‡Cont.	Cont.	‡Cont.	Cont.	Cont.	†Cont.
Lower Weld	300	300	‡300	300	‡300	300	300	250

TABLE 1 (continued)
Weld Sizes and Spacing – Millimeters

	<i>Weld size for lesser thickness of members joined, mm</i>							
	5	6.5	8	9.5	11	12.5	14.5	16
Nominal leg size of fillet <i>w</i>	3	5	6.5	6.5	8	8	8	8
Nominal throat size of fillet <i>t</i>	2	3.5	4.5	4.5	5.5	5.5	5.5	5.5
Length of fillet weld	40	65	75	75	75	75	75	75
<i>Structural Items</i>	<i>Spacing of Welds S, mm</i>							
Decks (continued)								
Peripheries of strength decks, exposed decks, and all watertight or oiltight decks, tunnels and flats	In accordance with 3-2-16/1.7							
Beams (transverse or longitudinal) to decks	*300	*300	300	275	300	275	250	250
Beams knees to beams and frames	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Hatch coamings to exposed decks	— — — In accordance with 3-2-16/1.7							
Transverses or deep beams to decks in tanks	—	200	225	200	225	200	175	150
Transverse or deep beams to deck elsewhere	—	—	250	225	250	225	200	175
Foundations								
To top plates, shell or inner bottom for main engines and major auxiliaries	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
To top plates, shell or inner bottom for boilers and other auxiliaries	In accordance with 3-2-16/1.7							
Additional Welding for Vessels Classed “Oil Carrier” (See Note 4)								
Girders and Webs								
Centerline girder to shell	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Centerline girder to deck	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	Dbl. Cont.
Bulkhead webs to plating	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
To face plates	—	150	150	150	150	125	125	‡Dbl. Cont.
Transverses								
Bottom transverses to shell	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Side, deck and bulkhead transverses to plating	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
To face plates	—	150	150	150	150	125	125	‡Dbl. Cont.

See general notes at beginning of table

Notes

- 1 Unbracketed stiffeners of shell, watertight and oiltight bulkheads and house fronts are to have double continuous welds for one-tenth of their length at each end.
 - 2 Unbracketed stiffeners of nontight structural bulkheads, deckhouse sides and after ends are to have a pair of matched intermittent welds at each end.
 - 3 Where the symbol, “—” (dash), is shown in place of the spacing of intermittent fillet welds, it is to indicate that the corresponding thickness is not anticipated for that particular structural member.
 - 4 The welding of longitudinals may be as required under frames or decks above. In addition, they are to have double continuous welds at the ends and in way of transverses equal in length to the depth of the longitudinal. For deck longitudinals, only a matched pair of welds is required at the transverses. For slab longitudinals, the attachment is to be made by double continuous fillet welds of a size *w* which is 0.3 times the thickness of the thinner plate, but need not be greater than 8.0 mm.
- ‡ Nominal size of fillet *w* may be reduced 1.5 mm.
† Nominal size of fillet *w* is increased 1.5 mm.
* Fillet welds are to be staggered.

TABLE 1
Weld Sizes and Spacing – Inches

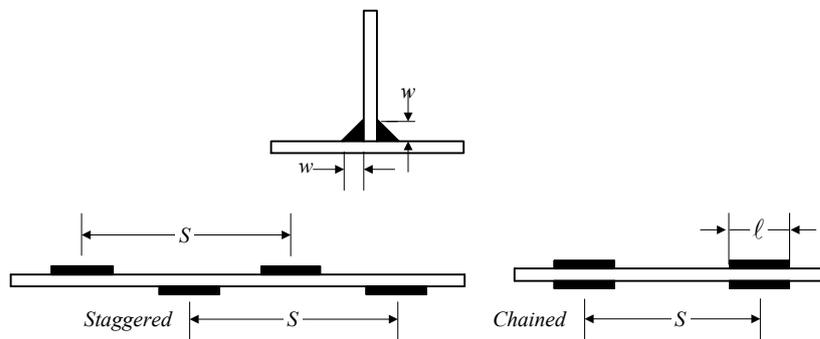
For weld requirements for thicknesses intermediate to those shown in the table, use the nearest thickness shown in the table.

Where beams, stiffeners, frames, etc., are intermittently welded and pass through slotted girders, shelves or stringers, there is to be a pair of matched intermittent welds on each side of each such intersection, and the beams, stiffeners and frames are to be efficiently attached to the girders, shelves and stringers.

For slab longitudinals, the attachment is to be made by double continuous fillet welds of a size w which is 0.3 times the thickness of the thinner plate, but need not be greater than $5/16$ in.

Where automatic double continuous fillet welding is provided, a reduction in fillet size of $1/16$ in. will be permitted, provided that the specified size of fillet in 3-2-16/Table 1 is $1/4$ in. or greater, the gap between the members does not exceed 0.04 in. and the penetration at the root is at least $1/16$ in. into the member being attached. This reduction does not apply for slab longitudinals.

For double continuous welding as an alternative to intermittent welding, see 3-2-16/1.3.1.



	Leg size for lesser thickness of members joined, in.							
	0.19	0.25	0.32	0.38	0.44	0.50	0.57	0.63
Nominal leg size of fillet w	$1/8$	$3/16$	$1/4$	$1/4$	$5/16$	$5/16$	$5/16$	$5/16$
Length of fillet weld	$1\frac{1}{2}$	$2\frac{1}{2}$	3	3	3	3	3	3

Structural Items	Spacing of Welds S , in.							
Single-Bottom Floors To center keelson <i>Note:</i> Connections elsewhere to take same weld as floors in double bottom	In accordance with 3-2-16/1.7							
Double-Bottom Floors To shell in aft peaks of vessels having high power and fine form	—	—	6	5	6	6	6	5
To shell flat of bottom forward (fore-end strengthening) and in peaks	—	—	10	9	10	10	9	8
To shell elsewhere	*12	*12	12	11	12	11	10	10
Solid floors to center vertical keel plate in engine room, under boiler bearers, wide-spread floors with longitudinal frames	In accordance with 3-2-16/1.7							
Solid floors to center vertical keel plate elsewhere, and open-floor brackets to center vertical keel	*10	*10	10	9	10	9	8	7
Solid floors and open-floor brackets to margin plate	In accordance with 3-2-16/1.7							
To inner bottom in engine room	In accordance with 3-2-16/1.7							
To inner bottom at forward end (fore-end strengthening)	*11	*11	11	10	11	10	9	8
To inner bottom elsewhere	*12	*12	12	11	12	11	10	10
Wide spaced with longitudinal framing to shell and inner bottom	In accordance with 3-2-16/1.7							
Solid floor stiffeners at watertight or oiltight boundaries	—	12	12	11	12	11	10	10
Watertight and oiltight periphery connections of floors throughout double bottom	In accordance with 3-2-16/1.7							

TABLE 1 (continued)
Weld Sizes and Spacing – Inches

	<i>Leg size for lesser thickness of members joined, in.</i>							
	<i>0.19</i>	<i>0.25</i>	<i>0.32</i>	<i>0.38</i>	<i>0.44</i>	<i>0.50</i>	<i>0.57</i>	<i>0.63</i>
Nominal leg size of fillet <i>w</i>	1/8	3/16	1/4	1/4	5/16	5/16	5/16	5/16
Length of fillet weld	1 1/2	2 1/2	3	3	3	3	3	3
<i>Structural Items</i>	<i>Spacing of Welds S, in.</i>							
Center Girder								
Nontight to inner-bottom or center strake in way of engine and to shell or bar keel	In accordance with 3-2-16/1.7							
Nontight to inner-bottom or center strake clear of engine	6	6	6	5	6	5	5	5
Watertight or oiltight to inner bottom, rider plate, shell or bar keel	In accordance with 3-2-16/1.7							
Intercostals								
Intercostals and continuous longitudinal girders to shell on flat bottom forward (fore-end strengthening) and to inner bottom in way of engines	—	6	6	5	6	5	5	‡Dbl. Cont.
Intercostals and continuous longitudinal girders to shell and inner bottom elsewhere and to floors	*11	*11	11	10	11	10	9	9
Watertight and oiltight periphery connections of longitudinal girders in double bottom	In accordance with 3-2-16/1.7							
Frames								
To shell in aft peaks of vessels having high power and fine form	—	—	6	5	6	6	6	5
To shell for 0.125 <i>L</i> forward and in peaks	—	—	10	9	10	10	9	9
To shell elsewhere—See Note 1	*12	*12	12	11	12	11	10	10
Unbracketed to inner bottom	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Frame brackets to frames, decks and inner bottom	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Longitudinals to shell and inner bottom	*12	*12	12	11	12	11	18	18
Longitudinals to shell on flat of bottom forward (fore-end strengthening)	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
Girders and Webs								
To shell and to bulkheads or decks in tanks	—	8	9	8	9	8	7	6
To bulkheads or decks elsewhere	—	—	10	9	10	9	8	7
Webs to face plate where area of face plate is 10 sq. in. or less	*10	*10	12	11	12	11	10	10
Webs to face plate area of face plate exceeds 10 sq. in.	—	—	10	9	10	9	8	7
Bulkheads								
Peripheries of swash bulkheads	—	8	9	8	9	8	7	6
Peripheries of nontight structural bulkheads	—	9	10	9	10	9	8	7
Peripheries of deep tank or watertight bulkheads	In accordance with 3-2-16/1.7							
Stiffeners to deep tank bulkheads – See Note 1	—	*12	12	11	12	11	10	10
Stiffeners to ordinary watertight bulkheads and deckhouse fronts – See Note 1	—	*12	12	11	12	11	10	10
Stiffeners to nontight structural bulkheads; stiffeners on deckhouse sides and after ends – See Note 2	*12	*12	*‡12	12	‡12	12	12	10
Stiffener brackets to beams, decks, etc.	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Decks								
Peripheries of platform decks and nontight flats								
Upper Weld	Cont.	Cont.	‡Cont.	Cont.	‡Cont.	Cont.	Cont.	†Cont.
Lower Weld	12	12	‡12	12	‡12	12	12	10

TABLE 1 (continued)
Weld Sizes and Spacing – Inches

	<i>Leg size for lesser thickness of members joined, in.</i>							
	0.19	0.25	0.32	0.38	0.44	0.50	0.57	0.63
Nominal leg size of fillet <i>w</i>	1/8	3/16	1/4	1/4	5/16	5/16	5/16	5/16
Length of fillet weld	1 1/2	2 1/2	3	3	3	3	3	3
<i>Structural Items</i>	<i>Spacing of Welds S, in.</i>							
Decks (continued)	In accordance with 3-2-16/1.7							
Peripheries of strength decks, exposed decks, and all watertight or oiltight decks, tunnels and flats								
Beams (transverse or longitudinal) to decks	*12	*12	12	11	12	11	10	10
Beams knees to beams and frames	Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	‡Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.
Hatch coamings to exposed decks	—	—	—	In accordance with 3-2-16/1.7				
Transverses or deep beams to decks in tanks	—	8	9	8	9	8	7	6
Transverses or deep beams to decks elsewhere	—	—	10	9	10	9	8	7
Foundations	In accordance with 3-2-16/1.7							
To top plates, shell or inner bottom for main engines and major auxiliaries	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
To top plates, shell or inner bottom for boilers and other auxiliaries	In accordance with 3-2-16/1.7							
Additional Welding for Vessels Classed “Oil Carrier” (See Note 4)								
Girders and Webs	In accordance with 3-2-16/1.7							
Centerline girder to shell	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Centerline girder to deck	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	Dbl. Cont.
Bulkhead webs to plating	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
To face plates	—	6	6	6	6	5	5	‡Dbl. Cont.
Transverses	In accordance with 3-2-16/1.7							
Bottom transverses to shell	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.	†Dbl. Cont.
Side, deck and bulkhead transverses to plating	—	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.	Dbl. Cont.
To face plates	—	6	6	6	6	6	6	‡Dbl. Cont.

See general notes at beginning of table

Notes

- 1 Unbracketed stiffeners of shell, watertight and oiltight bulkheads and house fronts are to have double continuous welds for one-tenth of their length at each end.
 - 2 Unbracketed stiffeners of nontight structural bulkheads, deckhouse sides and after ends are to have a pair of matched intermittent welds at each end.
 - 3 Where the symbol, “—” (dash), is shown in place of the spacing of intermittent fillet welds, it is to indicate that the corresponding thickness is not anticipated for that particular structural member.
 - 4 The welding of longitudinals may be as required under frames or decks above. In addition, they are to have double continuous welds at the ends and in way of transverses equal in length to the depth of the longitudinal. For deck longitudinals only, a matched pair of welds is required at the transverses. For slab longitudinals, the attachment is to be made by double continuous fillet welds of a size *w* which is 0.3 times the thickness of the thinner plate, but need not be greater than 1/16 in.
- ‡ Nominal size of fillet *w* may be reduced 1/16 in.
 † Nominal size of fillet *w* is increased 1/16 in.
 * Fillet welds are to be staggered.

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PART

3

CHAPTER 3 Subdivision and Stability

SECTION 1 General Requirements

1 General

Vessels of the following categories are to have subdivision and stability in accordance with the criteria as shown.

3 Criteria

3.1 Intact Stability

All vessels which have a length of 24 m (79 ft) or over, as defined in the International Convention on Load Lines, are to have intact stability guidance as required by Regulation 10 of the International Convention on Load Lines. The following criteria may be used for classification purposes:

- Cargo vessels of all sizes with or without deck cargo: IMO Resolution A167/A206 with A562

In case the above criteria are not applicable to a particular vessel, the intact stability will be reviewed by the Bureau in accordance with other recognized criteria appropriate to the vessel's type, size and intended service.

Tankers for which the request for class for new construction is received on or after 1 July 1997 are to meet the requirements in Appendix 3-3-A1, "Intact Stability of Tankers During Liquid Transfer Operations."

3.3 Subdivision and Damage Stability

Vessels of applicable size, type and service are to have subdivision and damage stability as required by the International Convention for the Safety of Life at Sea, 1974, as amended as follows:

- Passenger vessel Regulation II-1/4 through 8 (Section 3 of the *ABS Guide for Building and Classing Passenger Vessels*)
- Gas Carrier IGC Code (Section 5C-8-2 of the *ABS Rules for Building and Classing Steel Vessels*)
- Chemical carrier IBC Code (Section 5C-9-2 of the *ABS Rules for Building and Classing Steel Vessels*)
- Offshore support vessel see 5-10-2/3 of these Rules
- (1 July 1998) Cargo vessel of 80 m (262 ft) or more in subdivision length Regulation II-1/25-1 through 25-8

5 Review Procedures

5.1 Administration Review

When the vessel is issued an International Load Line Certificate, Passenger Ship Safety Certificate, Cargo Ship Safety Construction Certificate, International Certificate of Fitness for the Carriage of Liquefied Gases in Bulk or International Certificate of Fitness for Carriage of Dangerous Chemicals in Bulk by the flag Administration or its agent other than the Bureau, such Certificate will be accepted as evidence that the vessel has subdivision and stability in accordance with the criteria in the respective Convention.

Where the Administration undertakes the review of subdivision and stability and the Bureau is issuing the above Certificate, the acceptance of subdivision and stability by the Administration will be required before the certificate is issued.

5.3 Bureau Review

In all other cases, the information and calculations for subdivision and stability are to be submitted to the Bureau for review. Where the intact stability criteria are not applicable to a particular vessel, the review will be in accordance with other recognized criteria acceptable to the Bureau.

7 Damage Control plan for Dry Cargo Vessels

A plan for each deck and hold and boundaries of the watertight compartments, showing clearly the openings therein with the means of closure and position of any controls thereof, and the arrangements for the correction of any list due to flooding, is to be permanently exhibited or readily available on the navigation bridge. In addition, booklets containing the aforementioned information are to be made available to the officers of the vessel.

9 Onboard Computers for Stability Calculations (1 July 2007)

The use of onboard computers for stability calculations is not a requirement of class. However, if stability software is installed onboard vessels contracted on or after 1 July 2005, it should cover all stability requirements applicable to the vessel and is to be approved by the Bureau for compliance with the requirements of Appendix 3-3-A2, "Onboard Computers for Stability Calculations".

PART

3

CHAPTER **3 Subdivision and Stability**

APPENDIX **1 Intact Stability of Tankers During Liquid Transfer Operations**

1 General

1.1 Note

The following requirements for tankers (i.e., vessels designed to carry liquid in bulk) were developed from the IMO draft MSC Circular containing recommendations for existing oil tankers and the anticipated amendments to MARPOL for new tankers. The phenomenon of lolling is considered to be a safety issue for double hull tankers, as well as for other tankers having exceptionally wide cargo tanks (i.e. having cargo tank breadths greater than 60% of the vessel's maximum beam), which should be solved for vulnerable existing tankers and for new tankers now rather than be deferred until the proposed amendments to MARPOL enter into force. The solution should not be limited only to tankers subject to MARPOL.

1.3 Operations to be Addressed

Liquid transfer operations include cargo loading and unloading, lightering, ballasting and deballasting, ballast water exchange and tank cleaning operations.

3

Every tanker is to comply with the intact stability criteria specified in subparagraphs 3-3-A1/3.1 and 3-3-A1/3.3 for any operating draft reflecting actual, partial or full load conditions, including the intermediate stages of liquid transfer operations.

3.1

In port, the initial metacentric height, GM_0 , is not to be less than 0.15 m. Positive intact stability is to extend from the initial equilibrium position at which GM_0 is calculated over a range of at least 20 degrees to port and to starboard.

3.3

At sea, the intact stability criteria contained in paragraphs 3.1.2.1 to 3.1.2.4 of IMO Resolution A.749 (18), the Intact Stability Code, are applicable, or the criteria contained in the national requirements of the flag Administration are applicable if the national stability requirements provide at least an equivalent degree of safety.

5

For all loading conditions in port and at sea, including intermediate stages of liquid transfer operations, the initial metacentric height and the righting lever curve are to be corrected for the effect of free surfaces of liquids in tanks.

7

The intact stability criteria specified in 3-3-A1/3 is preferably to be met by design of the vessel, i.e., the design should allow for maximum free surface effects in all cargo, ballast and consumables tanks during liquid transfer operations.

9

If the intact stability criteria specified in 3-3-A1/3 are not met through design of the vessel alone, the Master is to be provided with clear instructions covering the operational restrictions and methods necessary to ensure compliance with these criteria during liquid transfer operations. These instructions should be simple and concise, and:

9.1

In a language understood by the officer-in-charge of transfer operations;

9.3

Require no more than minimal mathematical calculations by the officer-in-charge;

9.5

Indicate the maximum number of cargo and ballast tanks which may be slack under any possible condition of liquid transfer;

9.7

Provide pre-planned sequences of cargo/ballast transfer operations which indicate the cargo and ballast tanks which may be slack to satisfy the stability criteria under any specific condition of liquid transfer, including possible range of cargo densities. The slack tanks may vary during stages of the transfer operations and be any combination which satisfies the stability criteria;

9.9

Provide instructions for pre-planning other sequences of cargo/ballast transfer operations, including use of stability performance criteria in graphical or tabular form which enable comparisons of required and attained stability. These instructions for pre-planning other sequences, in relation to individual vessels, should take account of:

- i) The degree of criticality with respect to the number of tanks which can simultaneously have maximum free surface effects at any stage of liquid transfer operations;
- ii) The means provided to the officer-in-charge to monitor and assess the effects on stability and hull strength throughout the transfer operations;

- iii)* The need to give sufficient warning of an impending critical condition by reference to suitable margins (and the rate and direction of change) of the appropriate stability and hull strength parameters. If appropriate, the instructions should include safe procedures for suspending transfer operations until a suitable plan of remedial action has been evaluated;
- iv)* The use of on-line shipboard computer systems during all liquid transfer operations, processing cargo and ballast tank ullage data and cargo densities to continuously monitor the vessel's stability and hull strength and, when necessary, to provide effective warning of an impending critical situation, possibly automatic shut-down, and evaluation of possible remedial actions. The use of such systems is encouraged;

9.11

Provide for corrective actions to be taken by the officer-in-charge in case of unexpected technical difficulties with the recommended pre-planned transfer operations and in case of emergency situations. A general reference to the vessel's shipboard oil pollution emergency plan may be included; and

9.13

The instructions required above be prominently displayed:

- i)* In the approved trim and stability booklets;
- ii)* At the cargo/ballast transfer control station;
- iii)* In any computer software by which intact stability is monitored or calculations performed;
- iv)* In any computer software by which hull strength is monitored or calculations performed.

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PART

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CHAPTER **3 Subdivision and Stability**

APPENDIX **2 Onboard Computers for Stability Calculations** *(1 July 2007)*

1 General

1.1 Scope

The scope of stability calculation software is to be in accordance with the stability information as approved by the flag Administration or the Bureau on behalf of the flag Administration. The software is at least to include all information and perform all calculations or checks as necessary to ensure compliance with the applicable stability requirements.

Approved stability software is not a substitute for the approved stability information, and is used as a supplement to the approved stability information to facilitate stability calculations.

1.3 Design

The input/output information is to be easily comparable with approved stability information so as to avoid confusion and possible misinterpretation by the operator relative to the approved stability information.

An operation manual is to be provided for the onboard computer stability software.

The language in which the stability information is displayed and printed out as well as the operation manual is written is to be the same as used in the vessel's approved stability information. The primary language is to be English.

The onboard computer for stability calculations is vessel specific equipment and the results of the calculations are only applicable to the vessel for which it has been approved.

In case of modifications implying changes in the main data or internal arrangement of the vessel, the specific approval of any original stability calculation software is no longer valid. The software is to be modified accordingly and reapproved.

3 Calculation Systems

This Appendix covers either system, a passive system that requires manual data entry or an active system, which replaces the manual with the automatic entry with sensors reading and entering the contents of tanks, etc., provided the active system is in the off-line operation mode. However, an integrated system, which controls or initiates actions based on the sensor-supplied inputs is not within the scope of this Appendix.

5 Types of Stability Software

Three types of calculations performed by stability software are acceptable depending upon a vessel's stability requirements:

- Type 1 Software calculating intact stability only (for vessels not required to meet a damage stability criterion)
- Type 2 Software calculating intact stability and checking damage stability on basis of a limit curve (for vessels applicable to SOLAS Part B-1 damage stability calculations, etc.) or previously approved loading conditions
- Type 3 Software calculating intact stability and damage stability by direct application of preprogrammed damage cases for each loading condition (for some tankers etc.)

7 Functional Requirements

7.1 Calculation Program

The calculation program is to present relevant parameters of each loading condition in order to assist the Master in his judgment on whether the vessel is loaded within the approval limits. The following parameters are to be presented for a given loading condition:

- Deadweight data
- Lightship data
- Trim
- Draft at the draft marks and perpendiculars
- Summary of loading condition displacement, VCG, LCG and, if applicable, TCG
- Downflooding angle and corresponding downflooding opening
- Compliance with stability criteria: Listing of all calculated stability criteria, the limit values, the obtained values and the conclusions (criteria fulfilled or not fulfilled)

7.3 Direct Damage Stability Calculations

If direct damage stability calculations are performed, the relevant damage cases according to the applicable rules are to be pre-defined for automatic check of a given loading condition.

7.5 Warning

A clear warning is to be given on screen and in hard copy printout if any of the loading limitations are not complied with.

7.7 Data Printout

The data are to be presented on screen and in hard copy printout in a clear unambiguous manner.

7.9 Date and Time

The date and time of a saved calculation are to be part of the screen display and hard copy printout.

7.11 Information of Program

Each hard copy printout is to include identification of the calculation program with version number.

7.13 Units

Units of measurement are to be clearly identified and used consistently within a loading calculation.

9 Acceptable Tolerances

Depending on the type and scope of programs, the acceptable tolerances are to be determined differently, according to 3-3-A2/9.1 or 3-3-A2/9.3. In general, deviation from these tolerances is not to be accepted unless a satisfactory explanation for the difference is submitted for review and the same is satisfactorily confirmed by the Bureau that there would be no adverse effect on the safety of the vessel.

Examples of pre-programmed input data include the following:

- Hydrostatic data: Displacement, LCB, LCF, VCB, KMt and MCT vs. draft
- Stability data: KN or MS values at appropriate heel/trim angles vs. displacement, stability limits.
- Compartment data: Volume, LCG, VCG, TCG and FSM/Grain heeling moments vs. level of the compartment's contents.

Examples of output data include the following:

- Hydrostatic data: Displacement, LCB, LCF, VCB, KMt and MCT versus draft, as well as actual drafts, trim.
- Stability data: FSC (free surface correction), GZ-values, KG, GM, KG/GM limits, allowable grain heeling moments, derived stability criteria (e.g., areas under the GZ curve), weather criteria.
- Compartment data: Calculated Volume, LCG, VCG, TCG and FSM/Grain heeling moments vs. level of the compartment's contents

The computational accuracy of the calculation program results is to be within the acceptable tolerances specified in 3-3-A2/9.1 or 3-3-A2/9.3, of the results using an independent program or the approved stability information with identical input.

9.1 Calculation Program of the Approved Stability Information

Programs which use only pre-programmed data from the approved stability information as the basis for stability calculations are to have zero tolerances for the printouts of input data.

Output data tolerances are to be close to zero. However, small differences associated with calculation rounding or abridged input data are acceptable. Additionally differences associated with the use of hydrostatic and stability data for trims that differ from those in the approved stability information are acceptable subject to review by the Bureau.

9.3 Independent Program for Assessment of Stability

Programs which use hull form models as their basis for stability calculations are to have tolerances for the printouts of basic calculated data established against either data from the approved stability information or data obtained using the approval authority's model. Acceptable tolerances shall be in accordance with 3-3-A2/Table 1.

TABLE 1
Acceptable Tolerances

<i>Hull Form Dependent</i>	<i>Acceptable Tolerance ⁽¹⁾</i>
Displacement	2%
Longitudinal center of buoyancy, from AP	1%/50 cm max
Vertical center of buoyancy	1%/5 cm max
Transverse center of buoyancy	0.5% of B/5 cm max
Longitudinal center of flotation, from AP	1%/50 cm max
Moment to trim 1 cm	2%
Transverse metacentric height	1%/5 cm max
Longitudinal metacentric height	1%/50cm max
Cross curves of stability	5 cm
<i>Compartment Dependent</i>	
	<i>Acceptable Tolerance ⁽¹⁾</i>
Volume or deadweight	2%
Longitudinal center of gravity, from AP	1%/50 cm max
Vertical centre of gravity	1%/5 cm max
Transverse center of gravity	0.5% of B/5 cm max
Free surface moment	2%
Shifting moment	5%
Level of contents	2%
<i>Trim and Stability</i>	
	<i>Acceptable Tolerance ⁽¹⁾</i>
Drafts (forward, aft, mean)	1%/5 cm max
GMt	1%/5 cm max
GZ values	1%/5 cm max
FS correction	2%
Downflooding angle	2°
Equilibrium angles	1°
Distance to unprotected openings or margin line from WL, if applicable	±5%/5 cm
Areas under righting arm curve	5% or 0.0012 mrad

Notes:

- 1 Deviation in % = [(base value – applicant’s value)/base value] × 100.
- 2 Where the “base value” may be from the approved stability information or the results of master computer using an independent program.

11 Approval Procedure

11.1 Conditions of Approval of the Onboard Software for Stability Calculations

The onboard software used for stability calculations is subject to approval, which is to include:

- Verification of type approval, if any,
- Verification that the data used is consistent with the current condition of the vessel (see 3-3-A2/11.5),
- Verification and approval of the test conditions, and

- Verification that the software is appropriate for the type of vessel and stability calculations required.

The satisfactory operation of the software with the onboard computer(s) for stability calculations is to be verified by testing upon installation (see 3-3-A2/15). A copy of the approved test conditions and the operation manual for the computer/software are to be available onboard.

11.3 General Approval (optional)

Upon receipt of application for general approval of the calculation program, the Bureau may provide the applicant with test data consisting of two or more design data sets, each of which is to include a vessel's hull form data, compartmentation data, lightship characteristics and deadweight data, in sufficient detail to accurately define the vessel and its loading condition.

Acceptable hull form and compartmentation data may be in the form of surface coordinates for modeling the hull form and compartment boundaries (e.g., a table of offsets) or in the form of pre-calculated tabular data (e.g., hydrostatic tables, capacity tables) depending upon the form of data used by the software being submitted for approval. Alternatively, the general approval may be given based on at least two test vessels agreed upon between the applicant and the Bureau.

In general, the software is to be tested for two types of vessels for which approval is requested, with at least one design data set for each of the two types. Where approval is requested for only one type of vessel, a minimum of two data sets for different hull forms of that type of vessel are required to be tested.

For calculation software which is based on the input of hull form data, design data sets are to be provided for three types of vessels for which the software is to be approved, or a minimum of three data sets for different hull forms if approval is requested for only one type of vessel. Representative vessel types which require different design data sets due to their hull forms, typical arrangements, and nature of cargo include: tanker, bulk carrier, container carrier, and other dry cargo and passenger vessels.

The test data sets are to be used by the applicant to run the calculation program for the test vessels. The results obtained, together with the hydrostatic data and cross-curve data developed by the program, if appropriate are to be submitted to the Bureau for the assessment of the program's computational accuracy. The Bureau is to perform parallel calculations using the same data sets and a comparison of these results will be made against the applicant's submitted program's results.

11.5 Specific Approval

The Bureau is to verify the accuracy of the computational results and actual vessel data used by the calculation program for the particular vessel on which the program will be installed.

Upon receipt of application for data verification, the Bureau and the applicant are to agree on a minimum of four loading conditions, taken from the vessel's approved stability information, which are to be used as the test conditions.

For vessels carrying liquids in bulk, at least one of the conditions is to include partially filled tanks. For vessels carrying grain in bulk, one of the grain loading conditions is to include a partially filled grain compartment. Within the test conditions each compartment is to be loaded at least once. The test conditions normally are to cover the range of load drafts from the deepest envisaged loaded condition to the light ballast condition and are to include at least one departure and one arrival condition.

The Bureau is to verify that the following data, submitted by the applicant, is consistent with arrangements and most recently approved lightship characteristics of the vessel according to current plans and documentation on file with the Bureau, subject to possible further verification onboard:

- Identification of the calculation program including version number.
- Main dimensions, hydrostatic particulars and, if applicable, the vessel profile.

- The position of the forward and after perpendiculars, and if appropriate, the calculation method to derive the forward and after drafts at the actual position of the vessel's draft marks.
- Vessel lightweight and center of gravity derived from the most recently approved inclining experiment or light weight check.
- Lines plan, offset tables or other suitable presentation of hull form data if necessary for the Bureau to model the vessel.
- Compartment definitions, including frame spacing, and centers of volume, together with capacity tables (sounding/ullage tables), free surface corrections, if appropriate
- Cargo and Consumables distribution for each loading condition.

Verification by the Bureau does not absolve the applicant and shipowner of responsibility for ensuring that the information programmed into the onboard computer software is consistent with the current condition of the vessel.

13 Operation Manual

A simple and straightforward operation manual is to be provided, containing descriptions and instructions, as appropriate, for at least the following:

- Installation
- Function keys
- Menu displays
- Input and output data
- Required minimum hardware to operate the software
- Use of the test loading conditions
- Computer-guided dialogue steps
- List of warnings

15 Installation Testing

To ensure correct working of the computer after the final or updated software has been installed, it is the responsibility of the vessel's master to have test calculations carried out according to the following pattern in the presence of the Surveyor:

- From the approved test conditions at least one load case (other than lightship) is to be calculated.
Note: Actual loading condition results are not suitable for checking the correct working of the computer.
- Normally, the test conditions are permanently stored in the computer.

Steps to be performed:

- Retrieve the test load case and start a calculation run; compare the stability results with those in the documentation.
- Change several items of deadweight (tank weights and the cargo weight) sufficiently to change the draft or displacement by at least 10%. The results are to be reviewed to ensure that they differ in a logical way from those of the approved test condition.
- Revise the above modified load condition to restore the initial test condition and compare the results. The relevant input and output data of the approved test condition are to be replicated.

- Alternatively, one or more test conditions shall be selected and the test calculation performed by entering all deadweight data for each selected test condition into the program as if it were a proposed loading. The results shall be verified as identical to the results in the approved copy of the test conditions.

17 Periodical Testing

It is the responsibility of the vessel's master to check the accuracy of the onboard computer for stability calculations at each Annual Survey by applying at least one approved test condition.

If the Surveyor is not present for the computer check, a copy of the test condition results obtained by the computer check is to be retained onboard as documentation of satisfactory testing for the Surveyor's verification.

At each Special Periodical Survey, this checking for all approved test loading conditions is to be done in presence of the surveyor.

The testing procedure is to be carried out in accordance with 3-3-A2/15.

19 Other Requirements

The following features are to be provided to the software:

- Protection against unintentional or unauthorized modification of programs and data is to be provided.
- The program is to monitor operations and activate an alarm when the program is incorrectly or abnormally used.
- The program and any data stored in the system are to be protected from corruption by loss of power.
- Error messages with regard to limitations such as filling a compartment beyond capacity, or exceeding the assigned load line, etc. are to be included.

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CHAPTER

4 Fire Safety Measures

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CHAPTER 4 Fire Safety Measures

SECTION 1 Structural Fire Protection

1 General

1.1 SOLAS Application

For classification purposes, the fire and safety measures contained in the International Convention for the Safety of Life at Sea, 1974 (1974 SOLAS), as amended, are applicable to vessels of type, size and service coming under that Convention.

This section does not relax the requirements in other sections of the Rules.

Gross tonnage is to be taken as defined in 3-1-1/17.

1.3 Regulation

Regulation means the regulation contained in 1974 SOLAS, as amended. An abbreviated notation is used, e.g., Regulation II-2/5.2 means Regulation 5.2 of Chapter II-2.

1.5 Definitions

See Regulation II-2/3.

1.7 Materials Containing Asbestos (2005)

See Regulation II-1/3-5.

3 Passenger Vessels

For passenger vessels, the requirements of Section 4 of the *Passenger Vessel Guide* are applicable.

5 Cargo Vessels

5.1 All Vessels

For all cargo vessels as defined in Regulation II-2/3.7, the relevant requirements in Part B: Regulation 4, 5, 6; Part C: Regulations 7, 8, 9, 10, 11; Part D: Regulation 13; and Part G: Regulations 19 and 20, Chapter II-2 of 1974 SOLAS, as amended, are applicable.

5.3 Tankers

For tankers as defined in Regulations 3.48, Chapter II-2 of 1974 SOLAS, as amended, the following requirements are additional to 3-4-1/5.1.

5.3.1 Low Flash Point Cargoes

For tankers intended for the carriage of cargoes having flash point of 60°C (140°F) or less, the relevant requirements in Part A: Regulation 1; Part B: Regulation 4; Part C: Regulations 9, 10, 11; and Part E: Regulations 16, Chapter II-2 of 1974 SOLAS, as amended, are applicable. Furthermore, the requirements of Chapters 2, 14 and 15 of the Fire Safety Systems Code are also applicable.

5.3.2 High Flash Point Cargoes

For tankers intended for the carriage of cargoes having flash point above 60°C (140°F), the requirements in 3-4-1/5.1 are applicable, except that in lieu of the fixed fire extinguishing system required by Regulation II-2/10.7.1.3, they are to be fitted with a fixed deck foam system which is to comply with Chapter 14 of the Fire Safety Systems Code.

5.5 Vessels Carrying Chemicals or Liquefied Gases in Bulk

Vessels intending to carry chemicals or liquefied gases in bulk are to comply with the applicable requirements of Part 5C, Chapters 8 and 9 of the *Steel Vessel Rules*, as applicable, and the governing Administrative Regulations.

7 Review Procedures

7.1 Administration Review

When the vessel is issued a Passenger Ship Safety Certificate, Cargo Ship Safety Equipment Certificate or Cargo Ship Safety Construction Certificate by the flag Administration or its agent other than the Bureau, such Certificate will be accepted as evidence that the vessel is in accordance with the applicable criteria in 1974 SOLAS, as amended.

Where the Administration undertakes any part of the review and the Bureau is issuing the above Certificate, the acceptance by the Administration will be required before the certificate is issued.

Compliance with the Rule requirements, in addition to those in 1974 SOLAS, as amended, is to be verified by the Bureau.

7.3 Bureau Review

In all other cases, the required information and plans are to be submitted to the Bureau for review.

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CHAPTER **5 Equipment**

SECTION **1 Anchoring, Mooring and Towing Equipment**

1 General

All self-propelled vessels are to have anchors and chains. The anchors and their cables are to be connected and positioned ready for use. Means are to be provided for stopping each cable as it is paid out and the windlass is to be capable of heaving in either cable. Suitable arrangements are to be provided for securing the anchors and stowing the cables. Cables which are intended to form part of the equipment are not to be used as check chains when the vessel is launched. The inboard ends of the cables of the bower anchors are to be secured by efficient means.

3 Calculation of EN

3.1 Basic Equation

The basic Equipment Number (EN) is to be obtained from the following equation for use in determining required equipment.

$$EN = k\Delta^{2/3} + mBh + nA$$

where

- | | | |
|-----------------------|---|--|
| k | = | 1.0 (1.0, 1.012) |
| m | = | 2 (2, 0.186) |
| n | = | 0.1 (0.1, 0.00929) |
| Δ | = | molded displacement, in metric tons (long tons), to the summer load waterline |
| B | = | molded breadth, as defined in 3-1-1/5, in m (ft) |
| h | = | $a + h_1 + h_2 + h_3 + \dots$, as shown in 3-5-1/Figure 1. In the calculation of h , sheer, camber and trim may be neglected. |
| a | = | freeboard, in m (ft), from the summer load waterline amidships. |
| $h_1, h_2, h_3 \dots$ | = | height, in m (ft), on the center line of each tier of houses having a breadth greater than $B/4$ |

A = profile area, in m^2 (ft^2), of the hull, superstructure and houses above the summer load waterline which are within the Rule length. Superstructures or deck houses having a breadth at any point no greater than $0.25B$ may be excluded. Screens and bulwarks more than 1.5 m (4.9 ft) in height are to be regarded as parts of houses when calculating h and A .

3.3 Vessels of Unrestricted Service Having EN of 205 and Above

For vessels of unrestricted service having an EN of 205 or above in accordance with 3-5-1/3.1, the calculated EN is to be used in association with 3-5-1/Table 1. For vessels intended for towing service, see 3-5-1/3.5.

3.5 Vessels Having EN Less Than 205 or Vessels Intended for Towing Service

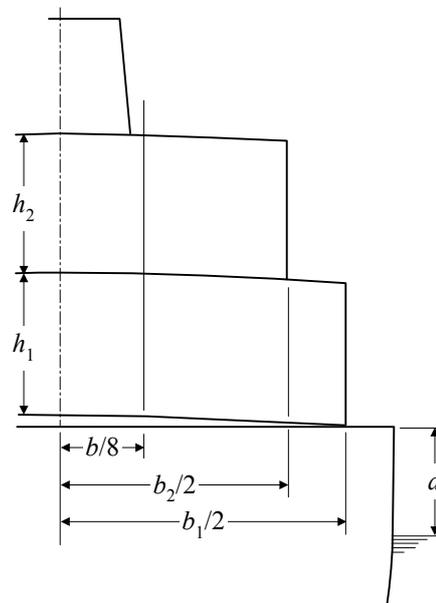
For vessels of unrestricted service having EN less than 205 calculated in accordance with 3-5-1/3.1 or for vessels intended for towing service, the EN for use with 3-5-1/Table 1 may be calculated in accordance with the following equation.

$$\text{Equipment Number} = k\Delta^{2/3} + m(Ba + \sum bh) + nA$$

Where k , m , n , Δ , B , a and A are as defined in 3-5-1/3.1 above and;

- b = breadth, in m (ft), of the widest superstructure or deckhouse on each tier.
- h = height, in m (ft), of each tier of deckhouse or superstructure having a width of $B/4$ or greater. In the calculation of h , sheer, camber and trim may be neglected. See 3-5-1/Figure 1.

FIGURE 1
Effective Heights of Deckhouses



5 Equipment with the Symbol Ⓔ

The equipment weight and size for all vessels with the symbol Ⓔ is to be in accordance with 3-5-1/Table 1 in association with EN calculated by 3-5-1/3.

7 Equipment without the Symbol Ⓔ

7.1 General

Where the symbol Ⓔ is not desired for vessels with EN calculated as permitted by 3-5-1/3.5, the equipment is to be in accordance with 3-5-1/Table 1, in association with the EN so calculated, but the following modifications may be accepted. See also 3-5-1/9.

7.3 Vessels intended for Limited Service

Vessels intended for limited service (see 1-1-3/7 of the *ABS Rules for Conditions of Classification (Part 1)*) and having their own moorage, e.g., ferries, launch, etc. with an equipment number less than 150, obtained from 3-5-1/3.5, are to have one anchor of the tabular weight and one-half the tabulated length of anchor chain in 3-5-1/Table 1. Alternatively, two anchors of one-half the tabular weight with the total length of anchor chain listed in 3-5-1/Table 1 may be fitted, provided both anchors are positioned and ready for use and the windlass is capable of heaving in either anchor.

7.5 Vessels Intended for Towing Service (2001)

Vessels intended for towing service are to have at least one anchor of one-half the tabular weight listed in 3-5-1/Table 1.

The towing winch can be used for releasing and heaving in the anchor, provided that the anchor is positioned ready for use and is capable of being quickly connected to the winch's wire rope.

9 Materials and Tests

Material and testing for anchors and chains on vessels receiving the Ⓔ symbol are to be in accordance with the requirements of Chapter 2 of the *ABS Rules for Materials and Welding (Part 2)* for the respective sizes of anchors and chains. See Sections 2-2-1 and 2-2-2 of the above referenced Part 2. Materials and tests for wire rope are to be in accordance with a national or other recognized standard.

Where the symbol Ⓔ is not desired in accordance with 3-5-1/7.1, the testing is to be carried out in accordance with the approved specification, and the manufacturer's test certificate to that effect is to be submitted to the Surveyor.

11 Anchor Types

Anchors are, in general, to be of the stockless type. The weight of the head of a stockless anchor, including pins and fittings, is not to be less than three-fifths of the total weight of the anchor. Where specifically requested by the Owners, the Bureau is prepared to give consideration to the use of special types of anchors, and where these are of proven superior holding ability, consideration may also be given to some reduction in the weight, up to a maximum of 25%, from the weight specified in 3-5-1/Table 1. In such cases, an appropriate notation will be made in the *Record*.

13 Windlass or Winch Support Structure

13.1 General (2004)

The windlass is to be of good and substantial make, suitable for the size of intended anchor cable. The winch is to be well bolted down to a substantial bed, and deck beams below the windlass are to be of extra strength and additionally supported. Where wire ropes are used in lieu of chain cables, winches capable of controlling the wire rope at all times are to be fitted.

Construction and installation of all windlasses and winches used for anchoring are to be carried out in accordance with the following requirements, to the satisfaction of the Surveyor. In general, the design is to conform to an applicable standard or code of practice. As a minimum, standards or practices are to indicate strength, performance and testing criteria.

For vessels with length, L (as defined in 3-1-1/3.1), between 80 meters (263 feet) and 90 meters (295 feet), the windlass supporting structures located on the exposed fore deck within the forward $0.25L$ are to meet the requirements in 3-5-1/13.3. Where the mooring winch is integral with the windlass, it is to be considered as a part of the windlass for the purpose of said paragraph.

The manufacturer or builder is to submit, in accordance with 4-1-1/7, the following, as applicable:

13.1.1 Plans

- i) Arrangement and details of the windlass or winch, drums, brakes, shaft, gears, coupling bolts, wildcat, sheaves, pulleys and foundation.
- ii) Electric one line diagram
- iii) Piping system diagrams
- iv) Control arrangements

Plans or data are to show complete details including power ratings, working pressures, welding details, material specifications, pipe and electric cable specifications, etc.

13.1.2 Calculations

Detailed stress calculations for the applicable system components listed in 3-5-1/13.1.1 above. The calculations are to be based on the breaking strength of the chain or wire rope, are to indicate maximum torque or load to which the unit will be subjected and also show compliance with either applicable sections of the Rules, such as Section 4-3-1 and Appendix 4-3-1A1 of the *Steel Vessel Rules*, for the gears and shafts, or to other recognized standard or code of practice.

13.3 Support Structure (2004)

The windlass is to be bolted down to a substantial foundation which is to meet the following load cases and associated criteria.

13.3.1 Operating Loads

13.3.1(a) *Load on Windlass Support Structure (2006)*. The following load is to be applied in the direction of the chain.

With cable stopper **not attached to windlass**: 45% of B.S.

With cable stopper attached to windlass: 80% of B.S.

Without cable stopper: 80% of B.S.

B.S. = minimum breaking strength of the chain, as indicated in 2-2-2/Tables 2 and 3 of the *ABS Rules for Materials and Welding (Part 2)*.

13.3.1(b) *Load on Cable Stopper and Support Structure (2006)*. A load of 80% of B.S. is to be applied in the direction of the chain.

13.3.1(c) *Allowable Stress (2006)*. The stresses in the structures supporting the windlass and cable stopper are not to exceed the yield point.

13.3.2 Sea Loads

13.3.2(a) *Pressures*. The following pressures and associated areas are to be applied (see 3-5-1/Figure 2):

- 200 kN/m² (20.4 tf/m², 4178 lbf/ft²) normal to the shaft axis and away from the forward perpendicular, over the projected area in this direction,
- 150 kN/m² (15.3 tf/m², 3133 lbf/ft²) parallel to the shaft axis and acting both inboard and outboard separately, over the multiple of f times the projected area in this direction,

where f is defined as:

$$f = 1 + B/H, f \text{ need not be taken as greater than } 2.5$$

$$B = \text{width of windlass measured parallel to the shaft axis}$$

$$H = \text{overall height of windlass.}$$

13.3.2(b) *Forces*. Forces in the bolts, chocks and stoppers securing the windlass to the deck are to be calculated. The windlass is supported by N groups of bolts, each containing one or more bolts, see 3-5-1/Figure 2.

i) *Axial Forces*. The aggregate axial force, R_i in respective group of bolts (or bolt), i , positive in tension, may be calculated from the following equations:

$$R_{xi} = P_x h x_i A_i / I_x$$

$$R_{yi} = P_y h y_i A_i / I_y$$

and

$$R_i = R_{xi} + R_{yi} - R_{si}$$

where

- P_x = force, kN (tf, lbf), acting normal to the shaft axis
- P_y = force, kN (tf, lbf), acting parallel to the shaft axis, either inboard or outboard, whichever gives the greater force in bolt group i
- h = shaft height above the windlass mounting, cm (in.)
- x_i, y_i = x and y coordinates of bolt group i from the centroid of all N bolt groups, positive in the direction opposite to that of the applied force, cm (in.)
- A_i = cross sectional area of all bolts in group i , cm² (in²)
- I_x = $A_i x_i^2$ for N bolt groups
- I_y = $A_i y_i^2$ for N bolt groups
- R_{si} = static reaction at bolt group i , due to weight of windlass.

- ii) *Shear forces.* Aggregated shear forces, F_{xi} , F_{yi} , applied to the respective bolt group, i , of bolts, and the resultant combined force, F_i , may be calculated from:

$$F_{xi} = (P_x - \alpha gM)/N$$

$$F_{yi} = (P_y - \alpha gM)/N$$

and

$$F_i = (F_{xi}^2 + F_{yi}^2)^{0.5}$$

where

- α = coefficient of friction (0.5)
 M = mass of windlass, in tonnes (Ltons)
 g = gravity: 9.81 m/sec² (32.2 ft/sec²)
 N = number of groups of bolt.

The axial tensile/compressive and lateral forces from the above equations are also to be considered in the design of the supporting structure.

13.3.2(c) *Stresses in Bolts.* Tensile axial stresses in the individual bolts in each group of bolt i are to be calculated. The horizontal forces, F_{xi} and F_{yi} , are normally to be reacted by shear chocks. Where “fitted” bolts are designed to support these shear forces in one or both directions, the von Mises equivalent stresses in the individual “fitted” bolts are to be calculated and compared to the stress under proof load. Where pourable resins are incorporated in the holding down arrangements, due account is to be taken in the calculations.

13.3.2(d) *Allowable Stress*

- i) *Bolts.* The safety factor against bolt proof strength is to be not less than 2.0.
ii) *Supporting Structures.* The stresses in the above deck framing and the hull structure supporting the windlass are not to exceed the following values:

Bending Stress 85% of the yield strength of the material

Shearing Stress 60% of the yield strength of the material

13.5 Trial

See 3-7-2/1.

FIGURE 2
Direction of Forces and Weight (2004)

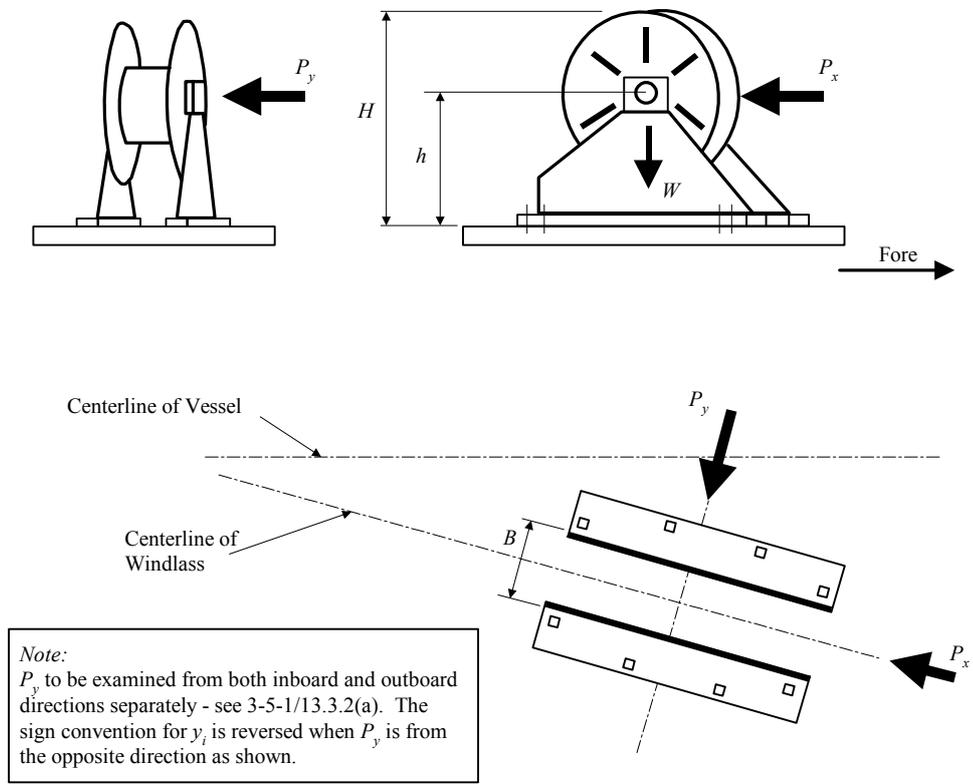
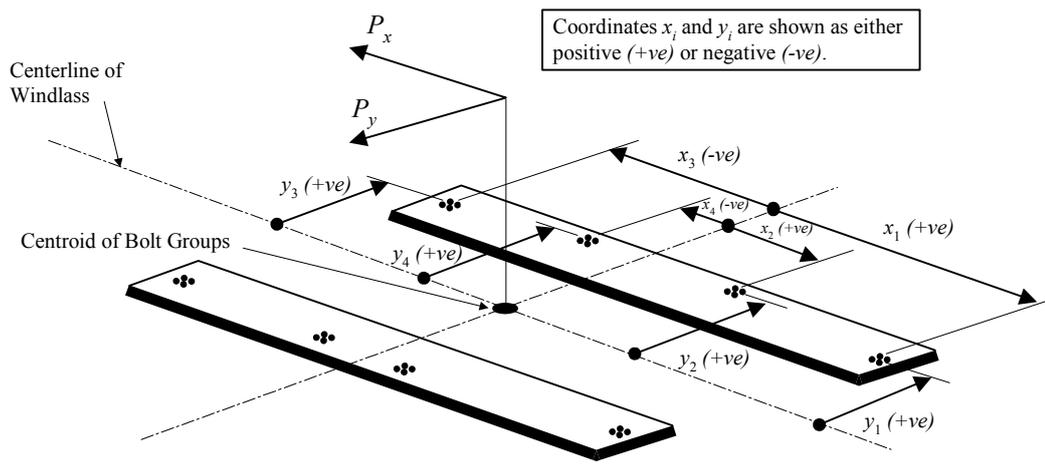


FIGURE 3
Sign Convention (2004)



15 Hawse Pipes

Hawse pipes are to be of ample size and strength. They are to have full rounded flanges and the least possible lead, in order to minimize the nip on the cables. They are to be securely attached to thick doubling or insert plates by continuous welds, the size of which are to be in accordance with Section 3-2-16 for the plating thickness and type of joint selected. When in position, they are to be thoroughly tested for watertightness by means of a hose in which the water pressure is not to be less than 2.06 bar (2.1 kgf/cm², 30 psi). Hawse pipes for stockless anchors are to provide ample clearances. The anchors are to be shipped and unshipped so that the Surveyor may be satisfied that there is no risk of the anchor jamming in the hawse pipe. Care is to be taken to ensure a fair lead for the chain from the windlass to the hawse pipes and to the chain pipes.

17 Hawsers and Towlines

Hawsers and towlines are not required as a condition of classification. The hawsers and towlines listed in 3-5-1/Table 2 are intended as a guide. Where the tabular breaking strength exceeds 490 kN (50,000 kgf, 110,200 lbf), the breaking strength and the number of individual hawsers given in the Table may be modified, provided their product is not less than that of the breaking strength and the number of hawsers given in the Table. For vessels having an *A/EN* ratio greater than 0.9 for metric units (9.7 for US units), the number of hawsers given in 3-5-1/Table 2 is to be increased by the number given below.

<i>A/EN Ratio</i>		<i>Increase number of hawsers by</i>
<i>SI Units, Metric Units</i>	<i>U.S. Units</i>	
above 0.9 up to 1.1	above 9.7 up to 11.8	1
above 1.1 up to 1.2	above 11.8 up to 12.9	2
above 1.2	above 12.9	3

where

A = defined in 3-5-1/3

EN = determined by the equation in 3-5-1/3

19 Bollard, Fairlead and Chocks (2007)

19.1 General

The arrangements and details of deck fittings used for mooring operations and/or towing operations at bow, sides and stern are to comply with the requirements of this section. The requirements for the supporting structures of these deck fittings are specified in 3-2-6/1.6.

19.3 Deck Fittings

The size of deck fittings is to be in accordance with recognized standards (e.g. ISO3913 Shipbuilding Welded Steel Bollards). The design load used to assess deck fittings and their attachments to the hull are to be in accordance with the requirements as specified in 3-2-6/1.6.

19.5 Safe Working Load (SWL)

The requirements on SWL apply for a single post basis (no more than one turn of one cable).

19.5.1 Mooring Operations

The SWL is not to exceed 80% of the design load per 3-2-6/1.6.2(a).

19.5.2 Towing Operations

The SWL used for normal towing operations (e.g., harbor/maneuvering) is not to exceed 80% of the design load per 3-2-6/1.6.2(b)i) and the SWL used for other towing operations (e.g., escort) is not to exceed the design load per 3-2-6/1.6.2(b)ii). For deck fittings used for both normal and other towing operations, the design load of 3-2-6/1.6.2(b) is to be used.

19.5.3 Marking and Plan

19.5.3(a) Marking. The SWL of each deck fitting is to be marked (by weld bead or equivalent) on the deck fittings used for towing/mooring.

19.5.3(b) Plan. The towing and mooring arrangements plan mentioned in 3-5-1/19.7 is to define the method of use of mooring lines and/or towing lines.

19.7 Towing and Mooring Arrangements Plan

The SWL for the intended use for each deck fitting is to be noted in the towing and mooring arrangements plan available on board for the guidance of the Master.

Information provided on the plan is to include in respect of each deck fitting:

- Location on the ship;
- Fitting type;
- SWL;
- Purpose (mooring/harbor towing/escort towing); and
- Manner of applying towing or mooring line load including limiting fleet angles.

Note: Where the arrangements and details of deck fittings and their supporting structures are designed based on the mooring arrangements as permitted in Note 2 of 3-2-6/1.6.2(a)i), the arrangement of mooring lines showing number of lines together with the breaking strength of each mooring line are to be clearly indicated on the plan.

This information is to be incorporated into the pilot card in order to provide the pilot proper information on harbor/escorting operations.

TABLE 1
Equipment for Self-propelled Ocean-going Vessels

SI, MKS Units							
The weight per anchor of bower anchors given in 3-5-1/Table 1 is for anchors of equal weight. The weight of individual anchors may vary 7% plus or minus from the tabular weight, provided that the combined weight of all anchors is not less than that required for anchors of equal weight. The total length of chain required to be carried onboard, as given in 3-5-1/Table 1, is to be reasonably divided between the two bower anchors. Where three anchors are given in 3-5-1/Table 1, the third anchor is intended as a spare bower anchor and is listed for guidance only. It is not required as a condition of classification.							
Equipment Numeral	Equipment Number*	Stockless Bower Anchors		Chain Cable Stud Link Bower Chain**			
		Number	Mass per Anchor, kg	Length, m	Diameter		
					Normal-Strength Steel (Grade 1), mm	High-Strength Steel (Grade 2), mm	Extra High-Strength Steel (Grade 3), mm
UA1	30	2	75	192.5	12.5	—	—
UA2	40	2	100	192.5	12.5	—	—
UA3	50	2	120	192.5	12.5	—	—
UA4	60	2	140	192.5	12.5	—	—
UA5	70	2	160	220	14	12.5	—
UA6	80	2	180	220	14	12.5	—
UA7	90	2	210	220	16	14	—
UA8	100	2	240	220	16	14	—
UA9	110	2	270	247.5	17.5	16	—
UA10	120	2	300	247.5	17.5	16	—
UA11	130	2	340	275	19	16	—
UA12	140	2	390	275	20.5	17.5	—
U6	150	2	480	275	22	19	—
U7	175	2	570	302.5	24	20.5	—
U8	205	3	660	302.5	26	22	20.5
U9	240	3	780	330	28	24	22
U10	280	3	900	357.5	30	26	24
U11	320	3	1020	357.5	32	28	24
U12	360	3	1140	385	34	30	26
U13	400	3	1290	385	36	32	28
U14	450	3	1440	412.5	38	34	30
U15	500	3	1590	412.5	40	34	30
U16	550	3	1740	440	42	36	32
U17	600	3	1920	440	44	38	34
U18	660	3	2100	440	46	40	36
U19	720	3	2280	467.5	48	42	36
U20	780	3	2460	467.5	50	44	38
U21	840	3	2640	467.5	52	46	40
U22	910	3	2850	495	54	48	42
U23	980	3	3060	495	56	50	44
U24	1060	3	3300	495	58	50	46
U25	1140	3	3540	522.5	60	52	46
U26	1220	3	3780	522.5	62	54	48
U27	1300	3	4050	522.5	64	56	50
U28	1390	3	4320	550	66	58	50
U29	1480	3	4590	550	68	60	52
U30	1570	3	4890	550	70	62	54

TABLE 1 (continued)

SI, MKS Units							
Equipment Numeral	Equipment Number*	Stockless Bower Anchors		Chain Cable Stud Link Bower Chain**			
		Number	Mass per Anchor, kg	Length, m	Diameter		
					Normal- Strength Steel (Grade 1), mm	High- Strength Steel (Grade 2), mm	Extra High- Strength Steel (Grade 3), mm
U31	1670	3	5250	577.5	73	64	56
U32	1790	3	5610	577.5	76	66	58
U33	1930	3	6000	577.5	78	68	60
U34	2080	3	6450	605	81	70	62
U35	2230	3	6900	605	84	73	64
U36	2380	3	7350	605	87	76	66
U37	2530	3	7800	632.5	90	78	68
U38	2700	3	8300	632.5	92	81	70
U39	2870	3	8700	632.5	95	84	73
U40	3040	3	9300	660	97	84	76
U41	3210	3	9900	660	100	87	78
U42	3400	3	10500	600	102	90	78
U43	3600	3	11100	687.5	105	92	81
U44	3800	3	11700	687.5	107	95	84
U45	4000	3	12300	687.5	111	97	87
U46	4200	3	12900	715	114	100	87
U47	4400	3	13500	715	117	102	90
U48	4600	3	14100	715	120	105	92
U49	4800	3	14700	742.5	122	107	95
U50	5000	3	15400	742.5	124	111	97
U51	5200	3	16100	742.5	127	111	97
U52	5500	3	16900	742.5	130	114	100
U53	5800	3	17800	742.5	132	117	102
U54	6100	3	18800	742.5	—	120	107
U55	6500	3	20000	770	—	124	111
U56	6900	3	21500	770	—	127	114
U57	7400	3	23000	770	—	132	117
U58	7900	3	24500	770	—	137	122
U59	8400	3	26000	770	—	142	127
U60	8900	3	27500	770	—	147	132
U61	9400	3	29000	770	—	152	132
U62	10000	3	31000	770	—	—	137
U63	10700	3	33000	770	—	—	142
U64	11500	3	35500	770	—	—	147
U65	12400	3	38500	770	—	—	152
U66	13400	3	42000	770	—	—	157
U67	14600	3	46000	770	—	—	162

* For intermediate values of equipment number, use equipment complement in sizes and weights given for the lower equipment number in the table.

** Wire ropes may be used in lieu of chain cables for both anchors on vessels less than 30 m (98.4 ft) in length. For vessels between 30 m (8.4 ft) and 40 m (131.2 ft) in length, wire rope may be used in lieu of chain cable for one anchor, provided normal chain cable is provided for the second anchor.

The wire is to have a breaking strength not less than the grade 1 chain of required size and a length of at least 1.5 times the chain it is replacing.

Between the wire rope and anchor, chain cable of the required size having a length of 12.5 m (41.0 ft), or the distance between anchor in stored position and winch, whichever is less, is to be fitted.

TABLE 1
Equipment for Self-propelled Ocean-going Vessels

US Units							
The weight per anchor of bower anchors given in 3-5-1/Table 1 is for anchors of equal weight. The weight of individual anchors may vary 7% plus or minus from the tabular weight, provided that the combined weight of all anchors is not less than that required for anchors of equal weight. The total length of chain required to be carried onboard, as given in 3-5-1/Table 1, is to be reasonably divided between the two bower anchors. Where three anchors are given in 3-5-1/Table 1, the third anchor is intended as a spare bower anchor and is listed for guidance only. It is not required as a condition of classification.							
Equipment Numeral	Equipment Number*	Stockless Bower Anchors		Chain Cable Stud Link Bower Chain**			
		Number	Mass per Anchor, pounds	Length, fathoms	Diameter		
					Normal-Strength Steel (Grade 1), inches	High-Strength Steel (Grade 2), inches	Extra High-Strength Steel (Grade 3), inches
UA1	30	2	165	105	1/2	—	—
UA2	40	2	220	105	1/2	—	—
UA3	50	2	265	105	1/2	—	—
UA4	60	2	310	105	1/2	—	—
UA5	70	2	350	120	9/16	1/2	—
UA6	80	2	400	120	9/16	1/2	—
UA7	90	2	460	120	5/8	9/16	—
UA8	100	2	530	120	5/8	9/16	—
UA9	110	2	595	135	11/16	5/8	—
UA10	120	2	670	135	11/16	5/8	—
UA11	130	2	750	150	3/4	11/16	—
UA12	140	2	860	150	13/16	11/16	—
U6	150	2	1060	150	7/8	3/4	—
U7	175	2	1255	165	15/16	13/16	—
U8	205	3	1455	165	1	7/8	13/16
U9	240	3	1720	180	1 1/8	15/16	7/8
U10	280	3	1985	195	1 3/16	1	15/16
U11	320	3	2250	195	1 1/4	1 1/8	15/16
U12	360	3	2510	210	1 5/16	1 3/16	1
U13	400	3	2840	210	1 7/16	1 1/4	1 1/8
U14	450	3	3170	225	1 1/2	1 5/16	1 3/16
U15	500	3	3500	225	1 9/16	1 5/16	1 3/16
U16	550	3	3830	240	1 5/8	1 7/16	1 1/4
U17	600	3	4230	240	1 3/4	1 1/2	1 5/16
U18	660	3	4630	240	1 13/16	1 9/16	1 7/16
U19	720	3	5020	255	1 7/8	1 5/8	1 7/16
U20	780	3	5420	255	2	1 3/4	1 1/2
U21	840	3	5820	255	2 1/16	1 13/16	1 9/16
U22	910	3	6280	270	2 1/8	1 7/8	1 5/8
U23	980	3	6740	270	2 3/16	1 15/16	1 3/4
U24	1060	3	7270	270	2 5/16	2	1 13/16
U25	1140	3	7800	285	2 3/8	2 1/16	1 13/16
U26	1220	3	8330	285	2 7/16	2 1/8	1 7/8
U27	1300	3	8930	285	2 1/2	2 3/16	2
U28	1390	3	9520	300	2 5/8	2 5/16	2
U29	1480	3	10120	300	2 11/16	2 3/8	2 1/16
U30	1570	3	10800	300	2 3/4	2 7/16	2 1/8

TABLE 1 (continued)

US Units							
Equipment Numeral	Equipment Number*	Stockless Bower Anchors		Chain Cable Stud Link Bower Chain**			
		Number	Mass per Anchor, pounds	Length, fathoms	Diameter		
					Normal- Strength Steel (Grade 1), inches	High- Strength Steel (Grade 2), inches	Extra High- Strength Steel (Grade 3), inches
U31	1670	3	11600	315	2 7/8	2 1/2	2 3/16
U32	1790	3	12400	315	3	2 5/8	2 5/16
U33	1930	3	13200	315	3 1/16	2 11/16	2 3/8
U34	2080	3	14200	330	3 3/16	2 3/4	2 7/16
U35	2230	3	15200	330	3 5/16	2 7/8	2 1/2
U36	2380	3	16200	330	3 7/16	3	2 5/8
U37	2530	3	17200	345	3 9/16	3 1/16	2 11/16
U38	2700	3	18300	345	3 5/8	3 3/16	2 3/4
U39	2870	3	19200	345	3 3/4	3 5/16	2 7/8
U40	3040	3	20500	360	3 7/8	3 5/16	3
U41	3210	3	21800	360	3 15/16	3 7/16	3 1/16
U42	3400	3	23100	360	4	3 9/16	3 1/16
U43	3600	3	24500	375	4 1/8	3 5/8	3 3/16
U44	3800	3	25800	375	4 1/4	3 3/4	3 5/16
U45	4000	3	27100	375	4 3/8	3 7/8	3 7/16
U46	4200	3	28400	390	4 1/2	3 15/16	3 7/16
U47	4400	3	29800	390	4 5/8	4	3 9/16
U48	4600	3	31100	390	4 3/4	4 1/8	3 5/8
U49	4800	3	32400	405	4 3/4	4 1/4	3 3/4
U50	5000	3	33900	405	4 7/8	4 3/8	3 7/8
U51	5200	3	35500	405	5	4 3/8	3 7/8
U52	5500	3	37200	405	5 1/8	4 1/2	3 15/16
U53	5800	3	39200	405	5 1/8	4 5/8	4
U54	6100	3	41400	405	—	4 3/4	4 1/4
U55	6500	3	44000	420	—	4 7/8	4 3/8
U56	6900	3	47400	420	—	5	4 1/2
U57	7400	3	50700	420	—	5 1/8	4 5/8
U58	7900	3	54000	420	—	5 3/8	4 3/4
U59	8400	3	57300	420	—	5 5/8	5
U60	8900	3	60600	420	—	5 3/4	5 1/8
U61	9400	3	63900	420	—	6	5 1/8
U62	10000	3	68000	420	—	—	5 3/8
U63	10700	3	72500	420	—	—	5 5/8
U64	11500	3	78000	420	—	—	5 3/4
U65	12400	3	85000	420	—	—	6
U66	13400	3	92500	420	—	—	6 1/8
U67	14600	3	101500	420	—	—	6 3/8

* For intermediate values of equipment number, use equipment complement in sizes and weights given for the lower equipment number in the table.

** Wire ropes may be used in lieu of chain cables for both anchors on vessels less than 30 m (98.4 ft) in length. For vessels between 30 m (8.4 ft) and 40 m (131.2 ft) in length, wire rope may be used in lieu of chain cable for one anchor, provided normal chain cable is provided for the second anchor.

The wire is to have a breaking strength not less than the grade 1 chain of required size and a length of at least 1.5 times the chain it is replacing.

Between the wire rope and anchor, chain cable of the required size having a length of 12.5 m (41.0 ft), or the distance between anchor in stored position and winch, whichever is less, is to be fitted.

TABLE 2
Towline and Hawsers for Self-propelled Ocean-going Vessels

SI & MKS Units

Equipment Numeral	Equipment Number*	Towline Wire or Rope			Hawsers			
		Length, m	Breaking Strength,		Number	Length of Each, m	Breaking Strength,	
			kN	kgf			kN	kgf
U6	150	180	98.0	10000	3	120	54.0	5500
U7	175	180	112.0	11400	3	120	59.0	6000
U8	205	180	129.0	13200	4	120	64.0	6500
U9	240	180	150.0	15300	4	120	69.0	7000
U10	280	180	174.0	17700	4	140	74.0	7500
U11	320	180	207.0	21100	4	140	78.0	8000
U12	360	180	224.0	22800	4	140	88.0	9000
U13	400	180	250.0	25500	4	140	98.0	10000
U14	450	180	277.0	28200	4	140	108.0	11000
U15	500	190	306.0	31200	4	160	123.0	12500
U16	550	190	338.0	34500	4	160	132.0	13500
U17	600	190	370.0	37800	4	160	147.0	15000
U18	660	190	406.0	41400	4	160	157.0	16000
U19	720	190	441.0	45000	4	170	172.0	17500
U20	780	190	479.0	48900	4	170	186.0	19000
U21	840	190	518.0	52800	4	170	201.0	20500
U22	910	190	559.0	57000	4	170	216.0	22000
U23	980	200	603.0	61500	4	180	230.0	23500
U24	1060	200	647.0	66000	4	180	250.0	25500
U25	1140	200	691.0	70500	4	180	270.0	27500
U26	1220	200	738.0	75300	4	180	284.0	29000
U27	1300	200	786.0	80100	4	180	309.0	31500
U28	1390	200	836.0	85200	4	180	324.0	33000
U29	1480	220	888.0	90600	5	190	324.0	33000
U30	1570	220	941.0	96000	5	190	333.0	34000
U31	1670	220	1024.0	104400	5	190	353.0	36000
U32	1790	220	1109.0	113100	5	190	378.0	38500
U33	1930	220	1168.0	119100	5	190	402.0	41000
U34	2080	240	1259.0	128400	5	200	422.0	43000
U35	2230	240	1356.0	138300	5	200	451.0	46000
U36	2380	240	1453.0	148200	5	200	480.0	49000
U37	2530	260	1471.0	150000	6	200	480.0	49000
U38	2700	260	1471.0	150000	6	200	490.0	50000
U39	2870	260	1471.0	150000	6	200	500.0	51000
U40	3040	280	1471.0	150000	6	200	520.0	53000

TABLE 2 (continued)
Towline and Hawsers for Self-propelled Ocean-going Vessels

SI & MKS Units

Equipment Numeral	Equipment Number*	Towline Wire or Rope			Hawsers			
		Length, m	Breaking Strength,		Number	Length of Each, m	Breaking Strength,	
			KN	kgf			kN	kgf
U41	3210	280	1471.0	150000	6	200	554.0	56500
U42	3400	280	1471.0	150000	6	200	588.0	60000
U43	3600	300	1471.0	150000	6	200	618.0	63000
U44	3800	300	1471.0	150000	6	200	647.0	66000
U45	4000	300	1471.0	150000	7	200	647.0	66000
U46	4200	300	1471.0	150000	7	200	657.0	67000
U47	4400	300	1471.0	150000	7	200	667.0	68000
U48	4600	300	1471.0	150000	7	200	677.0	69000
U49	4800	300	1471.0	150000	7	200	686.0	70000
U50	5000	300	1471.0	150000	8	200	686.0	70000
U51	5200	300	1471.0	150000	8	200	696.0	71000
U52	5500	300	1471.0	150000	8	200	706.0	72000
U53	5800	300	1471.0	150000	9	200	706.0	72000
U54	6100	300	1471.0	150000	9	200	716.0	73000
U55	6500	300	1471.0	150000	9	200	726.0	74000
U56	6900	300	1471.0	150000	10	200	726.0	74000
U57	7400	300	1471.0	150000	11	200	726.0	74000
U58	7900	—	—	—	12	200	736.0	75000
U59	8400	—	—	—	12	200	736.0	75000
U60	8900	—	—	—	13	200	736.0	75000
U61	9400	—	—	—	14	200	736.0	75000
U62	10000	—	—	—	15	200	736.0	75000
U63	10700	—	—	—	16	200	736.0	75000
U64	11500	—	—	—	17	200	736.0	75000
U65	12400	—	—	—	18	200	736.0	75000
U66	13400	—	—	—	19	200	736.0	75000
U67	14600	—	—	—	21	200	736.0	75000

* For intermediate values of equipment number, use equipment complement in sizes and weights given for the lower equipment number in the table.

TABLE 2
Towline and Hawsers for Self-propelled Ocean-going Vessels

US Units

Equipment Numeral	Equipment Number*	Towline Wire or Rope		Hawsers		
		Length, Fathoms	Breaking Strength, Pounds	Number	Length of Each Fathoms	Breaking Strength, Pounds
U6	150	98	22000	3	66	12100
U7	175	98	25100	3	66	13200
U8	205	98	29100	4	66	14300
U9	240	98	33700	4	66	15400
U10	280	98	39000	4	77	16500
U11	320	98	46500	4	77	17600
U12	360	98	50300	4	77	19800
U13	400	98	56200	4	77	22000
U14	450	98	62200	4	77	24200
U15	500	104	68800	4	88	27600
U16	550	104	76000	4	88	29800
U17	600	104	83300	4	88	33100
U18	660	104	91200	4	88	35300
U19	720	104	99200	4	93	38600
U20	780	104	107800	4	93	41900
U21	840	104	116400	4	93	45200
U22	910	104	125600	4	93	48500
U23	980	109	135500	4	98	51800
U24	1060	109	145500	4	98	56200
U25	1140	109	155400	4	98	60600
U26	1220	109	166000	4	98	63900
U27	1300	109	176500	4	98	69400
U28	1390	109	187800	4	98	72800
U29	1480	120	199700	5	104	72800
U30	1570	120	211500	5	104	75000
U31	1670	120	230000	5	104	79400
U32	1790	120	249500	5	104	84900
U33	1930	120	262500	5	104	90400
U34	2080	131	283000	5	109	94800
U35	2230	131	305000	5	109	101400
U36	2380	131	326500	5	109	108000
U37	2530	142	330500	6	109	108000
U38	2700	142	330500	6	109	110200
U39	2870	142	330500	6	109	112400
U40	3040	153	330500	6	109	116800

TABLE 2 (continued)
Towline and Hawsers for Self-propelled Ocean-going Vessels

US Units

Equipment Numeral	Equipment Number*	Towline Wire or Rope		Hawsers		
		Length, Fathoms	Breaking Strength, Pounds	Number	Length of Each Fathoms	Breaking Strength, Pounds
U41	3210	153	330500	6	109	124600
U42	3400	153	330500	6	109	132300
U43	3600	164	330500	6	109	138900
U44	3800	164	330500	6	109	145500
U45	4000	164	330500	7	109	145500
U46	4200	164	330500	7	109	147700
U47	4400	164	330500	7	109	149900
U48	4600	164	330500	7	109	152100
U49	4800	164	330500	7	109	154300
U50	5000	164	330500	8	109	154300
U51	5200	164	330500	8	109	156500
U52	5500	164	330500	8	109	158700
U53	5800	164	330500	9	109	158700
U54	6100	164	330500	9	109	160900
U55	6500	164	330500	9	109	163100
U56	6900	164	330500	10	109	163100
U57	7400	164	330500	11	109	163100
U58	7900	—	—	11	109	165300
U59	8400	—	—	12	109	165300
U60	8900	—	—	13	109	165300
U61	9400	—	—	14	109	165300
U62	10000	—	—	15	109	165300
U63	10700	—	—	16	109	165300
U64	11500	—	—	17	109	165300
U65	12400	—	—	18	109	165300
U66	13400	—	—	19	109	165300
U67	14600	—	—	21	109	165300

* For intermediate values of equipment number, use equipment complement in sizes and weights given for the lower equipment number in the table.

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CHAPTER

6 Navigation

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CHAPTER **6** **Navigation**

SECTION **1** **Visibility (1 July 1998)**

1 **Navigation Bridge Visibility**

Vessels of not less than 45 m (148 ft) in length having the keel laid or in similar stage of construction on or after 1 July 1998 are to meet the following requirements with regard to the visibility from the navigation bridge, unless they are navigating solely the Great Lakes of North America and their connecting and tributary waters as far east as the lower exit of the St. Lambert Lock at Montreal in the Province of Quebec, Canada. Special consideration will be given to vessels that operate only on domestic or on short, limited, international voyages.

1.1 **Field of Vision**

1.1.1 **Conning Position**

1.1.1(a) (1 July 2006) The view of the sea surface from the conning position is not to be obscured by more than $2L_{OA}$ (Length Overall) or 500 m (1640 ft), whichever is less, forward of the bow to 10° on either side for all conditions of draft, trim and deck cargo under which the particular vessel is expected to operate. See 3-6-1/Figure 1.

FIGURE 1 (1 July 2006)



1. A conning position is a place on the bridge with a commanding view and which is used by navigators when commanding, maneuvering and controlling a vessel.
2. Attention is drawn to flag Administrations requiring lengths of less than $2L_{OA}$.

1.1.1(b) No blind sector caused by cargo, cargo gear or other obstructions outside of the wheelhouse forward of the beam which obstructs the view of the sea surface as seen from the conning position is to exceed 10° . The total arc of blind sectors is not to exceed 20° . The clear sectors between blind sectors are to be at least 5° . However, in the view described in 3-6-1/1.1.1(a), each individual blind sector is not to exceed 5° .

1.1.1(c) The horizontal field of vision from the conning position is to extend over an arc of not less than 225° , that is, from right ahead to not less than 22.5° abaft the beam on either side of the vessel. See 3-6-1/Figure 2.

1.1.2 Bridge Wing

1.1.2(a) From each bridge wing, the horizontal field of vision is to extend over an arc of at least 225° , that is, from at least 45° on the opposite bow to right ahead and then from right ahead to right astern through 180° on the same side of the vessel. See 3-6-1/Figure 3.

1.1.2(b) The vessel's side is to be visible from the bridge wing.

1.1.3 Main Steering Position

From the main steering position, the horizontal field of vision is to extend over an arc from right ahead to at least 60° on each side of the vessel. See 3-6-1/Figure 4.

FIGURE 2

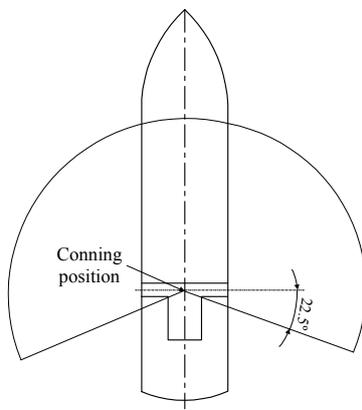


FIGURE 3

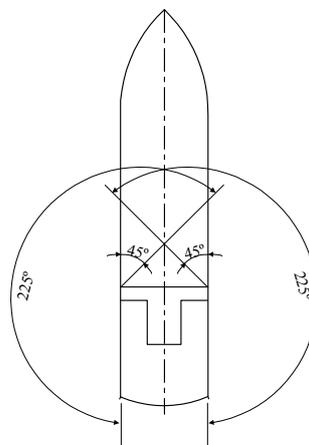
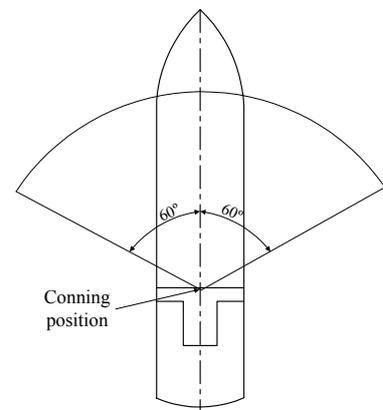


FIGURE 4



1.3 Windows and Their Arrangements

Windows and their arrangements are to meet the following requirements:

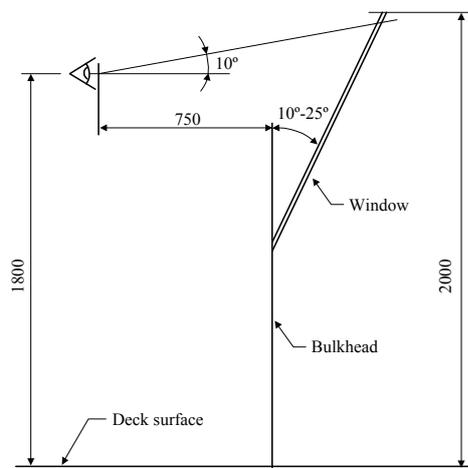
1.3.1 Framing

Framing between navigation bridge windows is to be kept to a minimum to meet the structural strength and stiffness requirements, and is not to be installed immediately in front of any workstations;

1.3.2 Inclination Angle

The bridge front windows are to be inclined from a vertical plane top out, at an angle of not less than 10° and not more than 25° , see 3-6-1/Figure 5;

FIGURE 5



1.3.3 Glass

Polarized and tinted windows are not to be fitted, and

1.3.4 Clear View

At all times, regardless of the weather conditions, at least two of the navigation bridge front windows are to provide a clear view, and in addition, depending on the bridge configuration, an additional number of windows are to provide a clear view. To this end, the following, or equivalent, is to be provided:

1.3.4(a) Sun Screens. Sunscreens with minimum color distortion. These sunscreens are to be readily removable and not permanently installed.

1.3.4(b) Wipers and Fresh Water Wash Systems. Heavy-duty wipers, preferably provided with an interval function, and fresh water wash systems. These wipers are to be capable of operating independently of each other.

1.3.4(c) De-icing and De-misting Systems. De-icing and de-misting systems to be provided.

1.3.4(d) Fixed Catwalk. A fixed catwalk with guardrails, fitted forward of the bridge windows, to enable manual cleaning of windows in the event of failure of the above systems.

1.3.5 Lower Edge

The height of the lower edge of the navigation bridge front windows above the bridge deck is to be kept as low as possible. In no case is the lower edge to present an obstruction to the forward view as described in this Section.

1.3.6 Upper Edge

The upper edge of the navigation bridge front windows is to allow a forward view of the horizon for a person with a height of eye of 1800 mm (5 ft-11 in.) above the bridge deck at the conning position when the vessel is pitching in heavy seas. The Bureau, if satisfied that an 1800 mm (5 ft-11 in.) height of eye is unreasonable and impractical, may allow reduction of the height of eye, but not to less than 1600 mm (5 ft-3 in.). See 3-6-1/Figure 5.

1.5 Unconventional Design

For vessels of unconventional design which cannot comply with the above requirements, arrangements are to be provided to the satisfaction of the Bureau to achieve a level of visibility that is as near as practical to those prescribed in this Section.

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CHAPTER **7 Testing, Trials and Surveys During Construction – Hull**

SECTION **1 Tank, Bulkhead and Rudder Tightness Testing**

1 General (1998)

1.1 Application

All tanks, including independent tanks of 5 m³ (176 ft³) or more in capacity, and other boundaries required to be watertight or weathertight are to be tested in accordance with this subsection and proven tight or structurally adequate, as follows:

1.1.1

Gravity Tanks for their structural adequacy and tightness,

1.1.2

Watertight Boundaries Other Than Tank Boundaries for their watertightness, and

1.1.3

Weathertight Boundaries for their weathertightness.

For the testing of cargo containment systems of liquefied gas carriers, the requirements in 5C-8-4/10 of the *Steel Vessel Rules* will apply.

1.3 Definitions

1.3.1

Structural Testing is a test to verify the structural adequacy of the design and the tightness of the tanks.

1.3.2

Air Testing is a test to verify the tightness of the structure by means of air pressure difference.

1.3.3

Hose Testing is a test to verify the tightness of the structure by a jet of water.

1.3.4

Hydropneumatic Testing is a combined hydrostatic and air testing wherein a tank is filled by water with air pressure applied on top.

1.3.5

Hydrostatic Testing is a test to verify the structural adequacy of the design and the tightness of the tanks structures by means of water pressure, by filling water to the level as specified in 3-7-1/Table 1. Hydrostatic testing is the normal means for structural testing with exception as per 3-7-1/3.3

3 Test Procedures (1998)

3.1 General

Tests are to be carried out in the presence of the Surveyor at a stage sufficiently close to completion, after all attachments, outfittings or penetrations which may affect the strength or tightness of the structure have been completed, and before any ceiling and cement work is applied over joints.

Specific test requirements are given in 3-7-1/5 and 3-7-1/Table 1. For vessels or tanks of special service, additional requirements are given in 3-7-1/Table 2.

For the timing of application of coating in relation to testing, see 3-7-1/7.

3.3 Structural Testing

Where structural testing is specified by 3-7-1/Table 1 or 3-7-1/Table 2, hydrostatic testing in accordance with 3-7-1/5.1 will be acceptable, except where practical limitations prevent it or where air testing is permitted by Note 1 to 3-7-1/Table 1. Hydropneumatic testing in accordance with 3-7-1/5.3 may be approved in lieu of hydrostatic testing. Structural testing may be carried out after the vessel is launched.

Tank boundaries are to be tested at least from one side. Tanks to be tested for structural adequacy (see Note 1 to 3-7-1/Table 1) are to be selected so that all representative structural members are tested for the expected tension and compression.

3.5 Air Testing

Air testing is to be in accordance with 3-7-1/5.5.

3.7 Hose Testing

Hose testing is applied to structures not subjected to structural or air testing, but are required to be watertight or weathertight, as specified in 3-7-1/Table 1. For the details of hose testing, see 3-7-1/5.7. Air testing or structural testing may be accepted in lieu of hose testing.

5 Details of Testing (1998)

5.1 Hydrostatic Testing

Hydrostatic testing is to consist of a head of water to the level specified in 3-7-1/Table 1.

5.3 Hydropneumatic Testing

When approved, the combined water level and air pressure used for hydropneumatic testing is to simulate the actual loading as far as practicable. The requirements and recommendations in 3-7-1/5.5 relative to air pressure will also apply.

5.5 Air Testing

All boundary welds, erection joints and penetrations, including pipe connections, are to be examined in accordance with the approved procedure and under a pressure differential not less than 0.15 bar (0.15 kgf/cm², 2.2 psi) with a leak indicating solution.

It is recommended that the air pressure in the tank be raised to and maintained at 0.20 bar (0.20 kgf/cm², 2.9 psi) for approximately one hour, with a minimum number of personnel around the tank, before being lowered to the test pressure.

A U-tube with a height sufficient to hold a head of water corresponding to the required test pressure is to be arranged. The cross sectional area of the U-tube is to be not less than that of the pipe supplying air. In addition to the U-tube, a master gauge or other approved means is to be provided to verify the pressure.

Other effective methods of air testing, including compressed air fillet weld testing or vacuum testing, may be considered in accordance with 3-7-1/5.9.

5.7 Hose Testing

Hose testing is to be carried out with the pressure in the hose of at least 2 bar (2 kgf/cm², 30 psi) during test. The nozzle is to have a minimum inside diameter of 12 mm (0.5 in.) and is to be located at a distance to the joint not exceeding 1.5 m (5 ft).

5.9 Other Methods of Testing

Other methods of testing may be considered upon submission of full particulars.

7 Application of Coating (1998)

7.1 Final Coating

7.1.1 Structural Testing

Final coating may be applied prior to the hydrostatic testing, provided an air test is carried out before the application of final coating.

7.1.2 Air Testing

For all manual or semi-automatic erection welds and all fillet weld tank boundary connections including penetrations, final coating is to be applied after air testing. For other welds, final coating may be applied prior to air testing, provided the Surveyor, after examination prior to the application of coating, is satisfied with the weld. The Surveyor reserves the right to require air testing prior to the final coating of automatic erection welds and manual or automatic pre-erection welds.

7.1.3 Hose Testing

Final coating is to be applied after all required hose testing is completed.

7.3 Temporary Coating

Any temporary coating which may conceal defects or leaks is to be applied as specified for final coating. This requirement does not apply to shop primer applied before fabrication.

TABLE 1
Testing Requirements for Tanks and Boundaries (1998)

	<i>Structures to be Tested</i>	<i>Type of Testing</i>	<i>Hydrostatic Testing Head or Pressure</i>	<i>Remarks</i>
1	Double Bottom Tanks	Structural ^(1,2)	The greater of - to the top of overflow, or - to the bulkhead deck	
2	Double Side Tanks	Structural ^(1,2)	The greater of - to the top of overflow, or - to 2.4 m (8 ft) above top of tank ⁽³⁾	
3	Deep Tanks or Cargo Oil Tanks	Structural ^(1,2)	The greatest of - to the top of overflow, - to 2.4 m (8 ft) above top of tank ⁽³⁾ , or - to the top of tank ⁽³⁾ plus setting of any pressure relief valve	
	Fuel Oil Bunkers	Structural		
4	Ballast Holds of Bulk Carriers	Structural ⁽¹⁾	<i>(2006)</i> The greater of - to the top of overflow, or - to 0.9m (3 ft) above top of hatch coaming ⁽³⁾	See items 12 and 13 for hatch covers.
5a	Peak Tanks	Structural	The greater of - to the top of overflow, or - to 2.4 m (8 ft) above top of tank ⁽³⁾	After peak tank test to be carried out after installation of stern tube.
5b	Fore Peak Voids (collision bhd.)	See Note 4	See Note 4	
5c	Aft Peak Voids	Air		
6	Cofferdams	Structural ⁽⁵⁾	The greater of - to the top of overflow, or - to 2.4 m (8 ft) above top of cofferdam	
7	Watertight Bulkheads	Hose ⁽⁶⁾		
8	Watertight Doors Below Freeboard or Bulkhead Deck	Hose		See 3-2-9/9.11 of the <i>Steel Vessel Rules</i> for additional test at the manufacturer.
9	Double Plate Rudder	Air		
10	Shaft Tunnel Clear of Deep Tanks	Hose		
11	Shell Doors	Hose		
12	Watertight Hatch Covers of Tanks on Combination Carriers.	<i>(2006)</i> Structural ^(1,2)	The greater of: - to 2.4 m (8 ft) above the top of hatch cover, or - setting pressure of the pressure relief valve.	At least every 2 nd hatch cover is to be tested.
13	Weather-tight Hatch Covers, Doors and other Closing Appliances	Hose ⁽⁶⁾		
14a	<i>(2006)</i> Chain Locker and Chain Pipe (aft of collision bulkhead)	Structural	<i>(1 July 2003)</i> To the top of chain pipe	Applies to vessels with freeboard length L_f (as defined in 3-1-1/3.3) greater than 24 meters (79 feet)
14b	<i>(2006)</i> Chain Locker and Chain Pipe (fwd of collision bulkhead)	Structural ⁽⁷⁾	To the top of chain pipe	

TABLE 1 (continued)
Testing Requirements for Tanks and Boundaries (1998)

	<i>Structures to be Tested</i>	<i>Type of Testing</i>	<i>Hydrostatic Testing Head or Pressure</i>	<i>Remarks</i>
15	Independent Tanks	Structural	The greater of - to the top of overflow, or - to 0.9 m (3 ft) above top of tank	
16	Ballast Ducts	Structural	Ballast pump maximum pressure or setting of any relief valve for the ballast duct if that is less.	
17	Hawse Pipes	Hose		

Notes:

- 1 (2006) Provided the structural similarity of a group of tanks is recognized by the Bureau and a representative tank as selected by the Bureau is structurally tested based on the design approval, air testing in accordance with 3-7-1/5.5 may be accepted. **In general, structural testing need not be repeated for subsequent vessels of a series of identical newbuildings.** Subsequent tanks may require hydrostatic testing, if found necessary, after the structural testing.
- 2 All cargo segregation boundaries in oil carriers and combination carriers and tanks for segregated cargoes of pollutants are to be hydrostatically tested.
- 3 (2006) Top of tank is the deck forming the top of the tank, excluding hatchways. **In holds for liquid cargo or ballast with large hatch covers, the top of tank is to be taken at the top of the hatch coaming.**
- 4 Hydrostatic testing to the damaged waterline, but not less than the distance to the bulkhead deck.
- 5 Air testing in accordance with 3-7-1/5.5 may be accepted, except that hydropneumatic testing may be required in consideration of the construction techniques and welding procedures employed.
- 6 Where hose testing is impractical due to the stage of outfitting, air test or other alternate method of testing such as close visual examination and, where necessary, nondestructive test of all joints may be considered.
- 7 (2006) **In general, structural testing need not be repeated for subsequent vessels of a series of identical newbuildings provided air testing in accordance with 3-7-1/5.5 is carried out.** Subsequent chain lockers may require hydrostatic testing if found necessary after the structural testing.

TABLE 2
Additional Testing Requirements for Vessels
or Tanks of Special Service (1998)

	<i>Type of Vessels or Tanks</i>	<i>Structures to be Tested</i>	<i>Type of Testing</i>	<i>Hydrostatic Testing Head</i>	<i>Remarks</i>
1	Liquefied Gas Carriers	Ballast or Fuel Oil Tanks adjacent to or between Cargo Tank Hold Spaces	Structural	The greater of water head - to the top of overflow, or - to 2.4 m (8 ft) above top of tank	
2	Edible Liquid Tanks	Independent Tanks	Structural	The greater of water head - to the top of overflow, or - to 0.9 m (3 ft) above top of tank	
3	Chemical Carriers	Integral or Independent Tanks	Structural	The greater of water head - to 2.4 m (8 ft) above top of tank, or - to top of tank plus setting of any pressure relief valve	

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CHAPTER **7 Testing, Trials and Surveys During Construction – Hull**

SECTION **2 Trials**

1 Anchor Windlass Trials *(1 July 2008)*

Each anchor windlass is to be tested under normal working conditions to demonstrate satisfactory operation. Each required anchor handling unit is to be independently tested for braking, clutch functioning, power lowering, hoisting and proper riding of the chain through the hawsepipe, over the wildcat (chain wheel), through the chain pipe and stowing in the chain locker. **It is to be confirmed that anchors properly seat in the stored position and that chain stoppers function as designed if fitted.** Also, it is to be demonstrated that the windlass is capable of lifting each anchor with 82.5 m (45 fathoms) length of chain submerged and hanging free. Where the available water depth is insufficient, the proposed test method will be specially considered.

3 Bilge System Trials

All elements of the bilge system are to be tested to demonstrate satisfactory pumping operation, including emergency suctions and all controls. Upon completion of the trials, the bilge strainers are to be opened, cleaned and closed up in good order.

5 Steering Trials

Refer to 4-3-3/15.3 for the technical details of the steering trials.

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CHAPTER **7 Testing, Trials and Surveys During Construction – Hull**

SECTION **3 Surveys**

1 Construction Welding and Fabrication

For surveys of hull construction welding and fabrication, refer to Chapter 4 of the *ABS Rules for Materials and Welding (Part 2)* and the *ABS Guide for Nondestructive Inspection of Hull Welds*.

3 Hull Castings and Forgings

For surveys in connection with the manufacture and testing of hull castings and forgings, refer to Chapter 1 of the *ABS Rules for Materials and Welding (Part 2)*.

5 Piping

For surveys in connection with the manufacture and testing of piping, refer to Part 4, Chapter 4.

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